



# Premier Gateway West Scoped Area Transportation Study Interim Report (Secondary Plan)

## Technical Appendices

Paradigm Transportation Solutions Limited

April 2018

# Appendix A

## VISSIM Model Development and Calibration Report

### Synchro and SimTraffic Parameter Adjustments and Methodology Memorandum





# Premier Gateway West Scoped Area Traffic Study Model Development and Calibration

Paradigm Transportation Solutions Limited

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## Project Number

170050

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# 1 Introduction

This technical report documents the development and calibration of the micro-simulation models to be used for the Premier Gateway West Scoped Area Traffic Study. Weekday morning (AM), weekday afternoon (PM) and Saturday models have been developed using the VISSIM software package (Version 9.00) to analyze existing conditions. These models will subsequently be used to assess future conditions on the adjacent freeway and arterial road segments, weaving sections, and ramp terminal operations within the study area limits.

## 2 Model Development

The United States Federal Highway Administration (FHWA) document entitled “Traffic Analysis Toolbox Volume III: Guidelines for Applying Traffic Microsimulation Modeling Software – July 2004” was referenced as a guideline for developing the micro-simulation models. The document recommends completing the following four steps before calibrating and validating a model:

- ▶ Project Scope;
- ▶ Data Collection;
- ▶ Base Model Development; and
- ▶ Error Checking.

### 2.1 Project Scope

**Figure 1** illustrates the study area for the micro-simulation modelling. The limits extend one interchange on Highway 401 on either side of the primary study area for the traffic study, which is bounded by Fifth Line North to the west and Ninth Line South to the east.

### 2.2 Data Collection

Reliable traffic data is imperative to accurately capturing the traffic conditions within a micro-simulation model. An extensive data collection effort was undertaken in May 2017 to provide input parameters and output measures of performance for model calibration and validation. The data included:

- ▶ Highway 401 mainline traffic volumes;
- ▶ Highway 401 travel times and speeds;
- ▶ Intersection turning movement counts;
- ▶ Posted maximum speed limits; and
- ▶ Intersection traffic control signal timings.



The Ministry of Transportation (MTO) also provided historical data for the Highway 401 mainline (link volumes and travel times) to validate the information collected for this modelling effort. The collected data for the AM and Saturday peak periods closely matched the MTO data. For the PM peak period, collected volumes were found to be about 15% lower than the MTO data. Accordingly, the Highway 401 mainline volumes used in the models were increased by this percentage in both travel directions.

**Appendix A** documents the comparison of the MTO and collected traffic data.

## 2.3 Base Model Development

Development of the AM, PM and Saturday peak period base models involved creating and coding the geometric (roadway) network over a high-quality aerial image of the study area. Highway 401 was coded with the default VISSIM “freeway” link behaviour, while the municipal road network was coded with the default “urban” link behaviour.

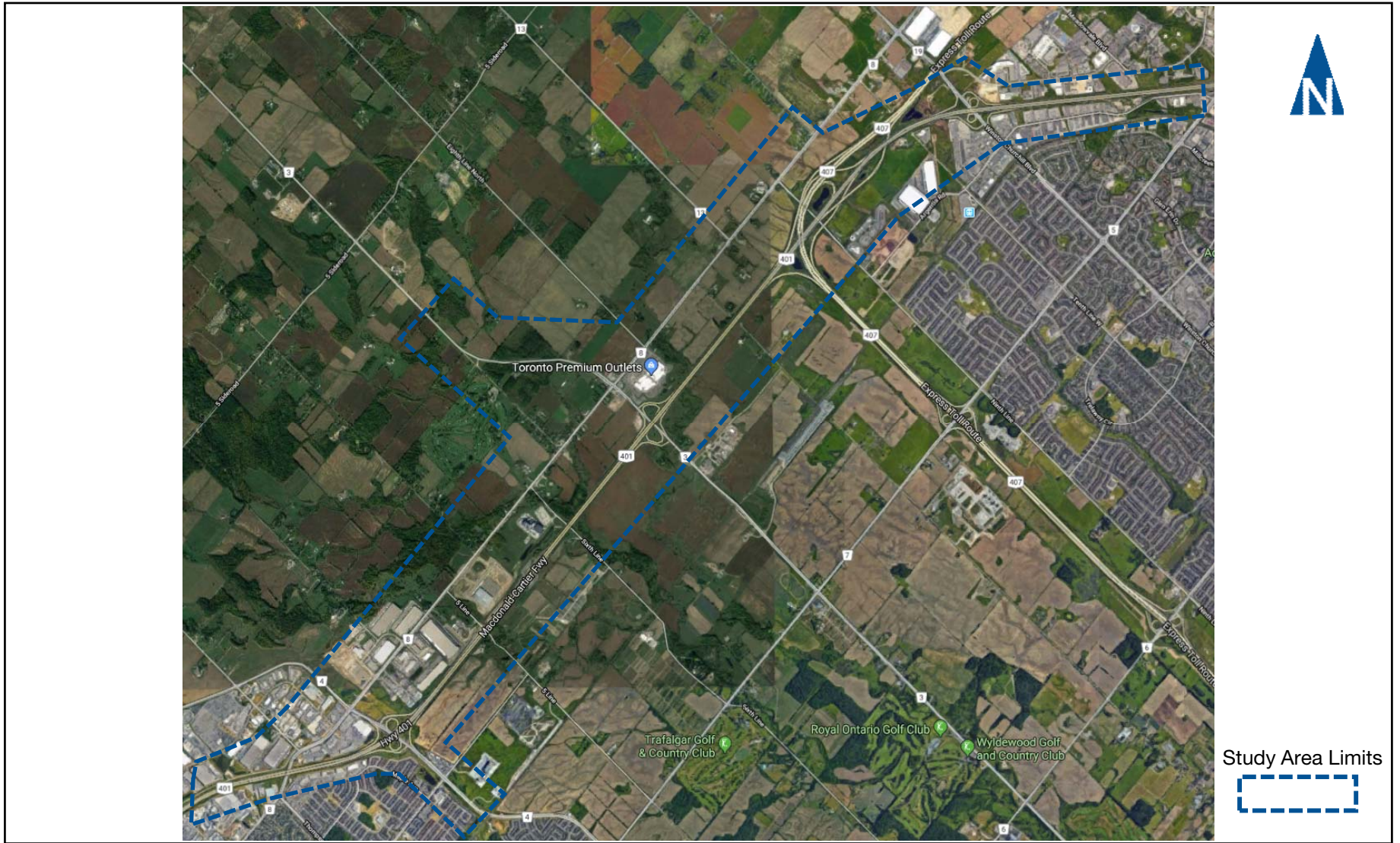
Once the geometric network was created, desired and reduced speeds were coded into the models. The desired speeds were based on the maximum posted speed limits. Reduced speeds for turns were based on industry accepted values. **Table 1** and **Table 2** summarize the speeds used. It is noted that on-ramp and off-ramp reduced speed area zones varied according to the posted maximum recommended advisory speeds. These reduced speed zones were coded accordingly.

Intersection control was coded within the models per signal timing plans provided by the responsible road authority (primarily Halton Region). Right-turns on red were coded where allowed in the field using the VISSIM software feature. Stop signs were coded at stop controlled intersections using the default stop parameters.

Conflict area points and priority rules were also coded within the models. Conflict areas were denoted in locations where links/connectors intersect and have the potential for vehicles to cross paths. The default parameters were used. Priority rules were incorporated into the models at certain locations where a conflict area would not accurately replicate the behaviour of conflicting vehicles. Default priority rule parameter values were utilized.

From the traffic data, volumes between all intersections were balanced and used to determine the vehicle inputs and outputs for the models. Static vehicle routing was utilized and based upon the existing traffic distribution. Vehicle inputs were coded into the models by an hourly flow rate and their corresponding vehicle composition.





# Micro-Simulation Model Study Area Limits

**TABLE 1: DESIRED SPEEDS**

Freeway/Road	Desired Speed (Maximum Posted)
Highway 401	100 km/h
James Snow Parkway	60 km/h
Trafalgar Road	60 km/h
Winston Churchill Boulevard	60 km/h
Steeles Avenue (between Fifth Line N – Fifth Line S)	70 km/h
Steeles Avenue (between Fifth Line S – Eighth Line S)	60 km/h
Steeles Avenue (between Eighth Line S – Ninth Line S)	80 km/h
Fifth Line N/Brownridge Road	60 km/h
Fifth Line S	60 km/h
Sixth Line N	60 km/h
Sixth Line S	60 km/h
Hornby Road	60 km/h
Eighth Line N	60 km/h
Eighth Line S	60 km/h
Ninth Line N	60 km/h
Ninth Line S	60 km/h

**TABLE 2: TURN SPEEDS**

Turn Direction	Reduced Speed Area Zone
Left-Turn (Passenger Vehicle)	25 km/h
Left-Turn (Truck/Heavy Vehicle)	20 km/h
Right-Turn (Passenger Vehicle)	15 km/h
Right-Turn (Truck/Heavy Vehicle)	12 km/h

## 2.4 Error Checking

The error checking portion of the model development process is focussed on identifying and resolving significant coding errors before commencing the time-consuming calibration and validation process. This involves a detailed review of the coded data and the simulation animation.

All coded data, including geometry, speeds, signal timing plans and traffic volumes, were examined. A detailed review of the simulation animation was also conducted to determine: if any locations within the models were missing conflict points or priority rules; whether signal timings were not functioning correctly; or if any other general coding parameters were missed or coded incorrectly. Based on the review, some parameters in the models were revised to more accurately capture the interactions of the vehicles on the network. **Appendix B** details the list of parameter adjustments.





## 3 Calibration and Validation

### 3.1 Parameters and Process

The calibration process involves the adjustment of model parameters until the simulation closely replicates actual traffic operations. The process is done iteratively until an accurate representation of the network operation is achieved, thereby providing a level of confidence in the model results.

Calibration parameters can be separated into two categories: system and operational. System calibration involves the investigation of model input assumptions (i.e., vehicle inputs, vehicle routing). Operational calibration focuses on detailed driver behaviour characteristics that affect the overall traffic operations in the model.

During the system calibration stage, input assumptions are verified and checked. The objectives are to identify where uncertainties were introduced into the base model and determine their effect on the overall system operations. System calibration parameters checked include the assumptions made on vehicle route choice, traffic demand inputs, traffic composition, study area boundaries, and the temporal distribution of demand and routing. Additionally, input data such as ramp terminal signal timings, roadway speed distributions, and roadway geometry characteristics are also verified for consistency with other model inputs.

Within the operational calibration stage, the model parameters that affect the overall traffic operations of the network are checked and modified if needed. This process consists of modifying the detailed driver behaviour parameters that affect the overall capacity of the transportation facilities, aggressiveness of drivers, and locations for lane changing. Adjustments of these parameters is essential for modelling freeway bottlenecks and local driving behaviour that can affect the overall traffic flow, speeds, capacity, and congestion. The parameters include car following characteristics (headway, standstill distance, safety distance, etc.), lane changing accepted deceleration rates, route lane change distances, and lane selection. The operational calibration stage requires the most time (and resources) to complete.

The validation process compares simulated model results with field observed data to determine the accuracy of the model. The goal is to identify parameter settings in the simulation model which produce outputs that match field data. Parameters are adjusted until an acceptable correlation is achieved between the observed field data and the modelled existing conditions. It is the final step in the iterative calibration process.

The validation check determines how closely the model is replicating actual study area conditions based on the targets set. Visual inspection of the simulation model is undertaken during this stage again. If the calibration/validation targets are not met, the system and operational calibration



process is revisited. Data results are analyzed and evaluated to determine the most appropriate parameters to modify during the next iteration.

Once calibrated and validated, the model can be used with confidence to analyze future scenarios which may include modifications to travel demands, roadway geometry, and/or the transportation network.

### 3.2 Targets

The accuracy of the calibration and validation process is based on an assessment and comparison between modelled and observed values. Targets are used to assess whether the process has yielded a simulation model that reasonably corresponds to field measures of effectiveness (MOEs).

For the freeway portion of the models, the MOEs examined included mainline freeway throughput (volumes), travel time and speed, per MTO requirements. **Table 3** summarizes typical calibration targets for these MOEs, as set out in the FHWA document.

**TABLE 3: FREEWAY MODEL CALIBRATION TARGETS**

Criteria and Measures	Calibration Acceptance Targets
<b><i>Hourly Flows, Model Versus Observed</i></b>	
Individual Link Flows	
Within 15%, for 700 veh/h < Flow < 2,700 veh/h	> 85% of cases
Within 100 veh/h, for Flow < 700 veh/h	> 85% of cases
Within 400 veh/h, for Flow > 2,700 veh/h	> 85% of cases
Sum of All Link Flows	Within 5% of sum of all link counts
GEH < 5 for Individual Link Flows	> 85% of cases
GEH for Sum of All Link Flows	GEH < 4 for sum of all link counts
<b><i>Travel Times, Model Versus Observed</i></b>	
Journey Times, Network	
Within 15% (or 1.0 minute, if higher)	> 85% of cases
<b><i>Visual Audits</i></b>	
Individual Link Speeds	
Visually Acceptable Speed-Flow Relationship	To analyst's satisfaction
Bottlenecks	
Visually Acceptable Queuing	To analyst's satisfaction

Source: FHWA, Traffic Analysis Toolbox Volume III: Guidelines for Applying Traffic Microsimulation Modelling Software – July 2004, Table 4, p. 64

As noted in the table, the GEH statistic is one criterion used to determine how well modelled and observed results match. The GEH statistic is a modified Chi-Squared statistic that incorporates both relative and absolute differences, and is computed as follows:





$$GEH = \sqrt{\frac{(M - O)^2}{0.5 * (M + O)}}$$

Where:

M: Simulated/Modelled Flows  
O: Observed Flows

GEH values provided an indication of the goodness of fit as outlined below:

GEH < 5	Flows can be considered a good fit
5 < GEH < 10	Flows may require further investigation
GEH > 5	Flows cannot be considered to be a good fit

For intersection turning movement volumes, the following calibration targets were set:

- ▶ Achieve 80% for most of the compared turning movement count (TMC) data sets, with an overall GEH value  $5 < GEH < 10$ ;
- ▶ Verify that no significant link, intersection approach or turning movement flows have a GEH value of greater than 10; and
- ▶ Verify that the absolute variation between the modelled and observed data is not more than 25%.

### 3.3 Results

The base models were calibrated based on the process and targets defined above. A three-hour simulation period was undertaken per MTO requirements. A total of five computer simulation runs were completed with the average results for the single peak hour reported and tabulated. **Table 4** summarizes the simulation time intervals.

**TABLE 4: SIMULATION TIME INTERVALS**

Period	Time Interval (seconds)
Seeding	1,800
Shoulder of peak hour	1,800 – 5,400
Peak Hour	5,400 – 9,000
Shoulder of peak hour	9,000 – 12,600

#### 3.3.1 Entering/Exiting Volume Comparisons

Vehicle inputs and outputs were checked to ensure all vehicle demands were loaded onto the network and that the correct vehicle volumes were exiting. This initial check provides an indication whether there are any significant issues with the model.



**Table 5** summarizes the total entering and exiting volumes for the AM, PM, and Saturday peak hour models. **Appendix C** contains the detailed output.

**TABLE 5: VEHICLE INPUTS AND OUTPUTS**

Time Period	Inbound			Outbound		
	Observed	Modelled	% Diff	Observed	Modelled	% Diff
AM Peak Hour	20,687	20,589	-0.5%	20,696	20,097	-2.9%
PM Peak Hour	23,152	23,069	-0.4%	23,152	22,598	-2.4%
Saturday Peak Hour	18,036	17,959	-0.4%	17,986	17,378	-3.4%

### 3.3.2 Highway 401 Mainline Volume Comparisons

A comparison of modelled and observed flows was conducted for the Highway 401 mainline freeway sections. The modelled traffic volumes as well as the simulation outputs were calculated as a mean of multiple computer model runs. Averaged results provided a reasonable representation, as this type of micro-simulation analysis is a stochastic process in which every computer run represents a single observation. A complete experiment therefore consisted of multiple computer runs, whose outputs were averaged to obtain the final modelled results that were then compared with the observed data.

**Table 6, Table 7, and Table 8** compare observed and modelled traffic flow on Highway 401 for the AM, PM, and Saturday peak hours. The tables show that the flows compare favourably. GEH values of less than 5 were observed for all individual link flows in the models, and a GEH less than 4 for the sum of all link counts. On all individual freeway links, flows are within the 400 vehicles per hour permissible variation. As a result, the absolute variation between the modelled and observed data is well within the 5% variation limit. The GEH values for all link flows meet the specified calibration targets.

### 3.3.3 Highway 401 On-Off Ramp Volume Comparisons

**Table 9, Table 10, and Table 11** compare the observed and modelled traffic volume for the Highway 401 on and off ramps for the AM, PM, and Saturday peak hours. Calibration targets are met for all link flows.

### 3.3.4 Intersection Volume Comparisons

**Table 12, Table 13, and Table 14** compare the observed and modelled total intersection traffic volumes for the AM, PM, and Saturday peak hours. Similar to the freeway mainline volumes, the overall intersection flows compare favourably. GEH values for all intersections are below the target of 5. Additionally, all intersection approaches and turning movements register GEH values within the specified targets. **Appendix D** provides the detailed results.



**TABLE 6: COMPARISON OF HIGHWAY 401 MAINLINE VOLUMES – AM PEAK HOUR**

Freeway Section	Direction	Observed	Modelled	% Diff	GEH
West of James Snow Parkway	EB	4,569	4,591	0%	0.3
James Snow Parkway – Trafalgar Road		5,266	5,238	-1%	0.4
Trafalgar Road – Highway 407		5,791	5,749	-1%	0.6
Highway 407 – Winston Churchill Boulevard		3,743	3,722	-1%	0.3
East of Winston Churchill Boulevard		3,434	3,367	-2%	1.1
East of Winston Churchill Boulevard	WB	3,446	3,466	1%	0.3
Winston Churchill Boulevard – Highway 407		3,451	3,461	0%	0.2
Highway 407 – Trafalgar Road		4,174	4,154	0%	0.3
Trafalgar Road – James Snow Parkway		4,231	4,195	-1%	0.6
West of James Snow Parkway		3,876	3,844	-1%	0.5
<b>Eastbound Total</b>		<b>22,803</b>	<b>22,667</b>	<b>-0.6%</b>	<b>0.9</b>
<b>Westbound Total</b>		<b>19,178</b>	<b>19,120</b>	<b>-0.3%</b>	<b>0.4</b>
<b>Sum of All Link Flows</b>		<b>41,981</b>	<b>41,787</b>	<b>-0.5%</b>	<b>0.9</b>

**TABLE 7: COMPARISON OF HIGHWAY 401 MAINLINE VOLUMES – PM PEAK HOUR**

Freeway Section	Direction	Observed	Modelled	% Diff	GEH
West of James Snow Parkway	EB	4,523	4,544	0%	0.3
James Snow Parkway – Trafalgar Road		4,815	4,805	0%	0.1
Trafalgar Road – Highway 407		4,823	4,812	0%	0.2
Highway 407 – Winston Churchill Boulevard		3,439	3,433	0%	0.1
East of Winston Churchill Boulevard		3,254	3,221	-1%	0.6
East of Winston Churchill Boulevard	WB	4,178	4,193	0%	0.2
Winston Churchill Boulevard – Highway 407		4,200	4,216	0%	0.2
Highway 407 – Trafalgar Road		5,946	5,785	-3%	2.1
Trafalgar Road – James Snow Parkway		5,584	5,423	-3%	2.2
West of James Snow Parkway		4,957	4,849	-2%	1.5
<b>Eastbound Total</b>		<b>20,854</b>	<b>20,815</b>	<b>0%</b>	<b>0.3</b>
<b>Westbound Total</b>		<b>24,865</b>	<b>24,466</b>	<b>-2%</b>	<b>2.5</b>
<b>Sum of All Link Flows</b>		<b>45,719</b>	<b>45,281</b>	<b>-1%</b>	<b>2.1</b>



**TABLE 8: COMPARISON OF HIGHWAY 401 MAINLINE VOLUMES – SATURDAY PEAK HOUR**

Freeway Section	Direction	Observed	Modelled	% Diff	GEH
West of James Snow Parkway	EB	4,338	4,367	1%	0.4
James Snow Parkway – Trafalgar Road		4,477	4,445	-1%	0.5
Trafalgar Road – Highway 407		4,916	4,789	-3%	1.8
Highway 407 – Winston Churchill Boulevard		3,980	3,853	-3%	2.0
East of Winston Churchill Boulevard		4,124	3,976	-4%	2.3
East of Winston Churchill Boulevard	WB	4,661	4,669	0%	0.1
Winston Churchill Boulevard – Highway 407		4,284	4,278	0%	0.1
Highway 407 – Trafalgar Road		4,932	4,889	-1%	0.6
Trafalgar Road – James Snow Parkway		4,623	4,543	-2%	1.2
West of James Snow Parkway		4,077	3,998	-2%	1.2
<b>Eastbound Total</b>		<b>21,835</b>	<b>21,430</b>	<b>-2%</b>	<b>2.8</b>
<b>Westbound Total</b>		<b>22,577</b>	<b>22,377</b>	<b>-1%</b>	<b>1.3</b>
<b>Sum of All Link Flows</b>		<b>44,412</b>	<b>43,807</b>	<b>-1%</b>	<b>2.9</b>



**TABLE 9: COMPARISON OF HIGHWAY 401 RAMP VOLUMES – AM PEAK HOUR**

Ramp Terminal	Direction		Observed	Modelled	% Diff	GEH
James Snow Parkway North Terminal	SB JSP to WB 401	On	105	103	-2%	0.2
	WB 401 to JSP	Off	702	695	-1%	0.3
	NB JSP to WB 401	On	242	241	0%	0.1
James Snow Parkway South Terminal	NB JSP to EB 401	On	922	913	-1%	0.3
	EB 401 to JSP	Off	409	390	-5%	1.0
	SB JSP to EB 401	On	184	190	3%	0.4
Trafalgar Road North Terminal	SB Trafalgar to WB 401	On	104	85	-18%	2.0
	WB 401 to Trafalgar	Off	401	399	0%	0.1
	NB Trafalgar to WB 401	On	347	349	1%	0.1
Trafalgar Road South Terminal	NB Trafalgar to EB 401	On	269	254	-6%	0.9
	EB 401 to Trafalgar	Off	230	230	0%	0.0
	SB Trafalgar to EB 401	On	486	467	-4%	0.9
Highway 407	EB 401 to WB 407	Off	173	171	-1%	0.2
	EB 401 to EB 407	Off	2131	2104	-1%	0.6
	EB 407 to EB 401	On	256	249	-3%	0.4
	WB 401 to WB 407	Off	86	97	13%	1.1
	EB 407 to WB 401	On	133	123	-5%	0.9
	WB 407 to WB 401	On	676	683	1%	0.3
Winston Churchill Blvd North Terminal	SB WCB to WB 401	On	370	350	-5%	1.1
	WB 401 to WCB	Off	618	618	0%	0.0
	NB WCB to WB 401	On	253	253	0%	0.0
Winston Churchill Blvd South Terminal	NB WCB to EB 401	On	243	241	-1%	0.1
	EB 401 to WCB	Off	801	782	-2%	0.7
	SB WCB to EB 401	On	249	241	-3%	0.5
<b>Sum of All Link Flows</b>			<b>10,390</b>	<b>10,228</b>	<b>-1.6%</b>	<b>1.6</b>



**TABLE 10: COMPARISON OF HIGHWAY 401 RAMP VOLUMES – PM PEAK HOUR**

Ramp Terminal	Direction		Observed	Modelled	% Diff	GEH
James Snow Parkway North Terminal	SB JSP to WB 401	On	110	75	-32%	3.6
	WB 401 to JSP	Off	944	915	-3%	1.0
	NB JSP to WB 401	On	207	204	-1%	0.2
James Snow Parkway South Terminal	NB JSP to EB 401	On	300	304	1%	0.2
	EB 401 to JSP	Off	274	271	-1%	0.2
	SB JSP to EB 401	On	266	265	0%	0.1
Trafalgar Road North Terminal	SB Trafalgar to WB 401	On	467	453	-3%	0.7
	WB 401 to Trafalgar	Off	887	868	-2%	0.6
	NB Trafalgar to WB 401	On	58	60	3%	0.3
Trafalgar Road South Terminal	NB Trafalgar to EB 401	On	328	322	-2%	0.3
	EB 401 to Trafalgar	Off	547	526	-4%	0.9
	SB Trafalgar to EB 401	On	227	227	0%	0.0
Highway 407	EB 401 to WB 407	Off	207	207	0%	0.0
	EB 401 to EB 407	Off	1,329	1,318	-1%	0.3
	EB 407 to EB 401	On	152	156	3%	0.3
	WB 401 to WB 407	Off	363	364	0%	0.1
	EB 407 to WB 401	On	207	208	0%	0.1
	WB 407 to WB 401	On	1,902	1,887	-1%	0.3
Winston Churchill Blvd North Terminal	SB WCB to WB 401	On	200	186	-7%	1.0
	WB 401 to WCB	Off	895	885	-1%	0.3
	NB WCB to WB 401	On	717	728	2%	0.4
Winston Churchill Blvd South Terminal	NB WCB to EB 401	On	250	236	-6%	0.9
	EB 401 to WCB	Off	611	599	-2%	0.5
	SB WCB to EB 401	On	176	170	-3%	0.5
<b>Sum of All Link Flows</b>			<b>11,624</b>	<b>11,434</b>	<b>-1.6%</b>	<b>1.8</b>





**TABLE 11: COMPARISON OF HIGHWAY 401 RAMP VOLUMES – SAT PEAK HOUR**

Ramp Terminal	Direction		Observed	Modelled	% Diff	GEH
James Snow Parkway North Terminal	SB JSP to WB 401	On	200	192	-4%	0.6
	WB 401 to JSP	Off	868	854	-2%	0.5
	NB JSP to WB 401	On	122	122	0%	0.0
James Snow Parkway South Terminal	NB JSP to EB 401	On	110	118	7%	0.7
	EB 401 to JSP	Off	179	184	3%	0.4
	SB JSP to EB 401	On	208	201	-3%	0.5
Trafalgar Road North Terminal	SB Trafalgar to WB 401	On	17	19	12%	0.5
	WB 401 to Trafalgar	Off	725	669	-8%	2.1
	NB Trafalgar to WB 401	On	399	441	11%	2.0
Trafalgar Road South Terminal	NB Trafalgar to EB 401	On	200	196	-2%	0.3
	EB 401 to Trafalgar	Off	468	469	0%	0.0
	SB Trafalgar to EB 401	On	707	646	-9%	2.3
Highway 407	EB 401 to WB 407	Off	155	158	2%	0.2
	EB 401 to EB 407	Off	865	833	-4%	1.1
	EB 407 to EB 401	On	84	91	8%	0.7
	WB 401 to WB 407	Off	72	72	0%	0.0
	EB 407 to WB 401	On	135	124	-8%	1.0
	WB 407 to WB 401	On	585	595	2%	0.4
Winston Churchill Blvd North Terminal	SB WCB to WB 401	On	210	211	0%	0.1
	WB 401 to WCB	Off	903	889	-2%	0.5
	NB WCB to WB 401	On	316	305	-3%	0.6
Winston Churchill Blvd South Terminal	NB WCB to EB 401	On	250	259	4%	0.6
	EB 401 to WCB	Off	477	463	-3%	0.6
	SB WCB to EB 401	On	371	367	-1%	0.2
<b>Sum of All Link Flows</b>			<b>8,626</b>	<b>8,478</b>	<b>-1.7%</b>	<b>1.6</b>



**TABLE 12: COMPARISON OF TOTAL INTERSECTION VOLUMES – AM PEAK HOUR**

Intersection	Observed	Modelled	% Diff	GEH
James Snow Parkway & Hwy 401 North Terminal	1,731	1708	-1%	0.6
James Snow Parkway & Hwy 401 South Terminal	1,678	1640	-2%	0.5
Trafalgar Road & Hwy 401 North Terminal	2,594	2,549	-2%	0.9
Trafalgar Road & Hwy 401 South Terminal	2,516	2,465	-2%	1.0
Winston Churchill Boulevard & Hwy 401 North Terminal	2,802	2,785	-1%	0.3
Winston Churchill Boulevard & Hwy 401 South Terminal	2,778	2,757	-1%	0.4
Fifth Line N & Steeles Avenue	1,554	1,526	-2%	0.7
Fifth Line S & Steeles Avenue	1,299	1,272	-2%	0.8
Sixth Line N & Steeles Avenue	1,282	1,238	-3%	1.2
Sixth Line S & Steeles Avenue	1,217	1,173	-4%	1.3
Hornby Road & Steeles Avenue	1,230	1,185	-4%	1.3
Hornby Road & Trafalgar Road	1,598	1,526	-5%	1.8
Trafalgar Road & Steeles Avenue	3,527	3,321	-6%	3.5
TPO Center Mall Access & Steeles Avenue	1,852	1,724	-7%	3.0
Eighth Line N/TPO Access & Steeles Avenue	2,020	1,888	-7%	3.0
Eighth Line S & Steeles Avenue	1,571	1,503	-4%	1.7
Ninth Line N & Steeles Avenue	2,350	2,260	-4%	1.9
Ninth Line S & Steeles Avenue	2,796	2,717	-3%	1.5
James Snow Parkway & Main Street	3,480	3,435	-1%	0.8
<b>Total Intersections</b>		<b>19</b>		
<b># of Intersections GEH &lt; 5</b>		<b>19</b>		
<b>% of Intersections GEH &lt; 5</b>		<b>100%</b>		



**TABLE 13: COMPARISON OF TOTAL INTERSECTION VOLUMES – PM PEAK HOUR**

Intersection	Observed	Modelled	% Diff	GEH
James Snow Parkway & Hwy 401 North Terminal	2,252	2,228	-1%	0.5
James Snow Parkway & Hwy 401 South Terminal	2,134	2,112	-1%	0.5
Trafalgar Road & Hwy 401 North Terminal	2,924	2,892	-1%	0.6
Trafalgar Road & Hwy 401 South Terminal	2,394	2,344	-2%	1.0
Winston Churchill Boulevard & Hwy 401 North Terminal	3,190	3,134	-2%	1.0
Winston Churchill Boulevard & Hwy 401 South Terminal	3,531	3,504	-1%	0.5
Fifth Line N & Steeles Avenue	2,008	2,014	0%	0.1
Fifth Line S & Steeles Avenue	1,815	1,827	1%	0.3
Sixth Line N & Steeles Avenue	1,823	1,835	1%	0.2
Sixth Line S & Steeles Avenue	1,750	1,771	1%	0.5
Hornby Road & Steeles Avenue	1,764	1,775	1%	0.3
Hornby Road & Trafalgar Road	1,411	1,318	-7%	2.5
Trafalgar Road & Steeles Avenue	4,136	3,979	-4%	2.5
TPO Center Mall Access & Steeles Avenue	2,736	2,602	-5%	2.6
Eighth Line N/TPO Access & Steeles Avenue	2,551	2,456	-4%	1.9
Eighth Line S & Steeles Avenue	2,168	2,117	-2%	1.1
Ninth Line N & Steeles Avenue	3,137	3,108	-1%	0.5
Ninth Line S & Steeles Avenue	3,573	3,534	-1%	0.7
James Snow Parkway & Main Street	3,362	3,344	-1%	0.3
<b>Total Intersections</b>		<b>19</b>		
<b># of Intersections GEH &lt; 5</b>		<b>19</b>		
<b>% of Intersections GEH &lt; 5</b>		<b>100%</b>		



**TABLE 14: COMPARISON OF TOTAL INTERSECTION VOLUMES – SATURDAY PEAK HOUR**

Intersection	Observed	Modelled	% Diff	GEH
James Snow Parkway & Hwy 401 North Terminal	1,968	1,918	-3%	1.1
James Snow Parkway & Hwy 401 South Terminal	1,738	1,698	-2%	1.0
Trafalgar Road & Hwy 401 North Terminal	2,777	2,593	-7%	3.6
Trafalgar Road & Hwy 401 South Terminal	2,286	2,251	-2%	0.7
Winston Churchill Boulevard & Hwy 401 North Terminal	2,341	2,293	-2%	1.0
Winston Churchill Boulevard & Hwy 401 South Terminal	2,300	2,251	-2%	1.0
Fifth Line N & Steeles Avenue	1,004	964	-4%	1.3
Fifth Line S & Steeles Avenue	926	891	-4%	1.2
Sixth Line N & Steeles Avenue	936	892	-5%	1.5
Sixth Line S & Steeles Avenue	888	850	-4%	1.3
Hornby Road & Steeles Avenue	904	866	-4%	1.3
Hornby Road & Trafalgar Road	1,222	1,146	-6%	2.2
Trafalgar Road & Steeles Avenue	3,387	3,115	-8%	4.8
TPO Center Mall Access & Steeles Avenue	2,315	2,153	-7%	3.4
Eighth Line N/TPO Access & Steeles Avenue	1,742	1,685	-3%	1.4
Eighth Line S & Steeles Avenue	1,285	1,246	-3%	1.1
Ninth Line N & Steeles Avenue	1,838	1,823	-1%	0.4
Ninth Line S & Steeles Avenue	1,892	1,888	0%	0.1
James Snow Parkway & Main Street	-	-	-	-
<b>Total Intersections</b>		<b>18</b>		
<b># of Intersections GEH &lt; 5</b>		<b>18</b>		
<b>% of Intersections GEH &lt; 5</b>		<b>100%</b>		



### 3.3.4 Highway 401 Travel Time and Speed Comparisons

Travel time survey data was collected along Highway 401 via the test vehicle method, in which the analyst collects measurements from a moving vehicle within the traffic stream. Automatic data collection equipment was utilized to aid with collection, with a GPS unit used to track the position and speed of the test vehicles as they travelled along the corridor.

Weekday and Saturday travel time studies were conducted along Highway 401 in both directions on May 4 (Thursday) and 6, 2017, respectively.

**Table 15** summarizes the number of test runs conducted.

**TABLE 15: TRAVEL TIME TEST RUNS**

Peak Period	Eastbound	Westbound
AM (6-10AM)	19	18
PM (3-7PM)	13	12
Saturday (11AM-3PM)	19	19

**Table 16** provides an excerpt from the Institute of Transportation Engineers (ITE) Manual of Transportation Engineering Studies (2<sup>nd</sup> Edition), summarizing the minimum number of test runs recommended for travel time and delay studies to achieve specified confidence levels in the collected data. The R values along the left side of the table indicate the difference in maximum and minimum running speeds, while the R values along the top indicate the desired permitted error.

The confidence level of the travel time studies was determined based on the data for the PM peak period, as the fewest number of runs were completed during this survey in the westbound direction. With a difference in maximum and minimum running speeds of approximately 40 km/h and 12 runs completed, a confidence level of 95% is achieved at a permitted error of  $\pm 8.0$  km/h. The calibration targets for travel times to be achieved are that journey times through the network should be within 15% (or 1.0 minute, if higher) for greater than 85% of cases. As such, the surveys provide a reasonable level of confidence for the true travel times and speeds for the corridor.

**Table 17**, **Table 18**, and **Table 19** compare the observed and modelled travel time and speed results along Highway 401 for the AM, PM, and Saturday peak hours. The tables show that the journey travel times are within the specified target calibration criteria. The observed and modelled eastbound and westbound journey times through the network are within 15%.

**Appendix E** provides lane by lane speed contour graphs, further comparing the results. The graphs have also been cross-referenced to speed plot data from obtained from the MTO iCorridor website.



**TABLE 16: MINIMUM NUMBER OF TEST RUNS FOR TRAVEL TIME AND DELAY STUDIES**

R	Minimum Sample Size $n$ for Specified Permitted Error $\epsilon$									
	Confidence Level: 99.73%					Confidence Level: 95%				
MPH (KPH)	1 (1.6)	2 (3.2)	3 (4.8)	4 (6.4)	5 (8.0)	1 (1.6)	2 (3.2)	3 (4.8)	4 (6.4)	5 (8.0)
1 (1.6)	6	5	4	4	4	4	3	3	3	3
2 (3.2)	9	6	5	5	4	6	4	3	3	3
3 (4.8)	13	8	6	5	5	8	5	4	4	3
4 (6.4)	17	9	7	6	6	10	6	5	4	4
5 (8.0)	21	11	8	7	6	12	7	5	4	4
6 (9.7)	26	13	9	8	7	15	8	6	5	4
7 (11.3)	32	15	10	8	7	18	9	6	5	5
8 (12.9)	37	17	12	9	8	21	10	7	6	5
9 (14.5)	43	19	13	10	9	21	11	8	6	5
10 (16.1)	50	21	14	11	9	27	12	8	7	6
11 (17.7)	57	24	15	12	10	31	13	9	7	6
12 (19.3)	64	26	17	13	11	34	15	10	8	6
13 (20.9)	72	29	18	14	11	38	16	11	8	7
14 (22.5)	80	32	20	15	12	43	18	11	9	7
15 (24.1)	89	34	21	16	13	47	19	12	9	8
20 (32.2)	-	50	30	21	17	71	27	17	12	10
25 (40.2)	-	68	39	27	21	99	36	22	15	12
30 (48.3)	-	89	50	34	26	-	47	27	19	15

Source: Institute of Transportation Engineers, Manual of Transportation Engineering Studies (2<sup>nd</sup> Edition), 1998, Exhibit 9.1, p. 163





**TABLE 17: COMPARISON OF HIGHWAY 401 TRAVEL TIMES AND SPEEDS – AM PEAK HOUR**

Dir.	Section	Travel Times (seconds)			Speeds (km/h)		
		Obs.	Mod.	Diff	Obs.	Mod.	%Diff
EB	James Snow Pkwy. to Trafalgar Rd.	182	152.6	-29.5	84	100	19%
	Trafalgar Rd. to Hwy. 407	105	108.4	3.4	99	97	-2%
	Hwy. 407 to Winston Churchill Blvd.	63	64.2	1.2	107	105	-2%
WB	Winston Churchill Blvd. to Hwy. 407	57	64.3	7.3	118	105	-11%
	Hwy. 407 to Trafalgar Rd.	88	100.2	12.2	118	105	-11%
	Trafalgar Rd. to James Snow Pkwy.	137	147.0	9.9	111	103	-7%
<b>Total EB Travel Time</b>		<b>350.0</b>	<b>325.2</b>	<b>-24.8</b> <b>(-7%)</b>	-	-	-
<b>Total WB Travel Time</b>		<b>282.0</b>	<b>311.4</b>	<b>29.4</b> <b>(10%)</b>	-	-	-

**TABLE 18: COMPARISON OF HIGHWAY 401 TRAVEL TIMES AND SPEEDS – PM PEAK HOUR**

Dir.	Section	Travel Times (seconds)			Speeds (km/h)		
		Obs.	Mod.	Diff	Obs.	Mod.	%Diff
EB	James Snow Pkwy. to Trafalgar Rd.	181	193.0	12.0	84	79	-6%
	Trafalgar Rd. to Hwy. 407	113	120.7	8.7	93	87	-7%
	Hwy. 407 to Winston Churchill Blvd.	69	70.1	1.1	98	96	-2%
WB	Winston Churchill Blvd. to Hwy. 407	187	131.5	-55.5	36	51	42%
	Hwy. 407 to Trafalgar Rd.	233	228.3	-4.5	45	46	2%
	Trafalgar Rd. to James Snow Pkwy.	183	186.9	3.8	83	81	-2%
<b>Total EB Travel Time</b>		<b>362.0</b>	<b>383.9</b>	<b>21.9</b> <b>(6%)</b>	-	-	-
<b>Total WB Travel Time</b>		<b>603.0</b>	<b>546.7</b>	<b>56.1</b> <b>(-9%)</b>	-	-	-

**TABLE 19: COMPARISON OF HIGHWAY 401 TRAVEL TIMES AND SPEEDS – SATURDAY PEAK HOUR**

Dir.	Section	Travel Times (seconds)			Speeds (km/h)		
		Obs.	Mod.	Diff	Obs.	Mod.	%Diff
EB	James Snow Pkwy. to Trafalgar Rd.	157	152.0	-5.0	97	100	4%
	Trafalgar Rd. to Hwy. 407	104	103.9	-0.1	101	101	0%
	Hwy. 407 to Winston Churchill Blvd.	59	64.3	5.3	115	105	-9%
WB	Winston Churchill Blvd. to Hwy. 407	60	64.9	4.9	112	104	-7%
	Hwy. 407 to Trafalgar Rd.	91	101.8	10.8	114	103	-9%
	Trafalgar Rd. to James Snow Pkwy.	149	147.7	-1.3	102	103	1%
<b>Total EB Travel Time</b>		<b>320.0</b>	<b>320.2</b>	<b>0.2</b> <b>(0%)</b>	-	-	-
<b>Total WB Travel Time</b>		<b>300.0</b>	<b>314.4</b>	<b>14.4</b> <b>(5%)</b>	-	-	-



## 4 Conclusion

The analyses presented in this report indicate that the micro-simulation models developed from the data collected calibrate favourably to existing conditions based on the established calibration targets and visual observations in the field. Modelled volume throughput and travel times and speeds correspond to observed data, confirming the models fairly represent the base year. With confidence, it can be concluded that the base models capture the observed traffic patterns and vehicle behaviours.

On this basis, the base models will be utilized as the foundation for other VISSIM models developed for this assignment.



# Appendix A

## Comparison of MTO and Collected Traffic Data



21 December 2017

**File**  
170050

**To**

**From**  
Gene Chartier, P.Eng., FITE  
Vice-President  
Paradigm Transportation Solutions Limited

## Highway 401 Traffic Volume and Travel Time Comparison

The following memorandum compares traffic volumes and travel time collected by the Ministry of Transportation, Ontario (MTO) and Paradigm Transportation Solutions Limited (Paradigm) for Highway 401 in support of the Town of Halton Hills Premier Gateway Traffic Study. The MTO has since suggested that Paradigm compare their traffic data to theirs in determining the appropriateness of a micro-simulation model in development.

The study limits on Highway 401 for the Premier Gateway Traffic Study is from the James Snow Parkway interchange in the west to Winston Churchill Boulevard interchange in the east.

### Traffic Volumes

#### Location and Date

Paradigm conducted traffic counts and travel time surveys on Highway 401 in May 2017. **Table 1** shows the location and dates of the traffic counts compared with the traffic volumes provided by the MTO. MTO provide traffic volumes from 2014 to 2016 for all sections of Highway 401 except for between James Snow Parkway and Trafalgar Road. They also provided some July 2014 and October 2014 traffic counts, but those were not considered as Paradigm counts were done in the spring. Only the October 2016 traffic volumes between Highway 407 and Winston Churchill Boulevard were looked at as they are the most recent traffic volumes provided.

**Table 1: Traffic Count Locations & Dates**

Highway 401 Location	Date	
	Paradigm	MTO
West of James Snow Parkway		May 13, 2014 to May 19, 2014
James Snow Parkway & Trafalgar Road	May 4, 2017 & May 6, 2017	May 13, 2014 to May 19, 2014
Trafalgar Road & Highway 407	May 4, 2017 & May 6, 2017	
Highway 407 & Winston Churchill Boulevard	May 4, 2017 & May 6, 2017	May 13, 2014 to May 19, 2014
		Oct 14, 2016 to Oct 20, 2016
East of Winston Churchill Boulevard		May 25, 2014 to May 31, 2014

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## Peak Hour Traffic Volumes

The weekday AM, PM, and Saturday traffic volumes for Highway 401 were compared. The traffic volumes from MTO contained seven consecutive days of traffic collection. The Tuesday, Wednesday, and Thursday traffic volumes were averaged to get the weekday AM and PM peak hour traffic volumes. The Saturday peak hour traffic volumes were obtained from the Saturday traffic counts. Paradigm only counted one weekday and one Saturday for their traffic counts. **Table 2** compares the AM, PM, and Saturday peak hour traffic volumes of Highway 401 and the locations where both Paradigm and MTO had counts available.

During the AM peak hour, the section of Highway 401 between Trafalgar Road and Highway 407 are within 1% for the eastbound direction and 5% for the westbound direction. The section between Highway 407 and Winston Churchill Boulevard, Paradigm's counts are within 3% for the eastbound direction and 2% for the westbound direction compared to the 2014 MTO traffic volumes. Paradigm's counts are within 5% for the eastbound and 2% for the westbound directions with the 2016 MTO traffic volumes.

During the PM peak hour, the Paradigm traffic volumes range from -19% to 2% difference with the MTO traffic volumes. The section of Highway 401 between Trafalgar Road and Highway 407, the eastbound direction is -8% and the westbound direction is -14%. The section between Highway 407 and Winston Churchill Boulevard, the eastbound direction is -19% compared with the 2014 traffic volumes and -15% with the 2016 volumes. The westbound direction is -13% against the 2014 volumes and 2% with the 2016 volumes.

The Saturday peak hour shows that Paradigm's traffic volumes are within 3% in both directions for the section of Highway 401 between Trafalgar Road and Highway 407. Between Highway 407 and Winston Churchill Boulevard, Paradigm's traffic volumes are within -5% for the eastbound direction and 4% for the westbound direction compared with the 2014 traffic volumes. Compared with the 2016 traffic volumes, the difference is -5% for the eastbound direction and -7% in the westbound direction.

**Table 2: Peak Hour Traffic Volumes**

Highway 401 Location	AM Peak Hour		PM Peak Hour		Saturday Peak Hour	
	EB	WB	EB	WB	EB	WB
<i>Paradigm (2017)</i>						
Trafalgar Road & Highway 407	5791	4174	3970	4967	4716	4932
Highway 407 & Winston Church Boulevard	3743	3451	2849	3655	3980	4284
<i>MTO (2014 - 2016)</i>						
Trafalgar Road & Highway 407 (2014)	5742	3978	4300	5794	4838	5109
Highway 407 & Winston Church Boulevard (2014)	3651	3393	3505	4181	4204	4119
Highway 407 & Winston Church Boulevard (2016)	3569	3393	3345	3593	4178	4595
<i>Difference (Paradigm - MTO)</i>						
Trafalgar Road & Highway 407	49	196	-330	-827	-122	-177
Highway 407 & Winston Church Boulevard (2014)	92	58	-656	-526	-224	165
Highway 407 & Winston Church Boulevard (2016)	174	58	-496	62	-198	-311
<i>Difference %</i>						
Trafalgar Road & Highway 407	1%	5%	-8%	-14%	-3%	-3%
Highway 407 & Winston Church Boulevard (2014)	3%	2%	-19%	-13%	-5%	4%
Highway 407 & Winston Church Boulevard (2016)	5%	2%	-15%	2%	-5%	-7%

Paradigm’s traffic volumes are close to MTO’s traffic volumes for the AM and Saturday peak hours and should be considered acceptable. During the PM peak hour, the MTO traffic volumes could be used, or increase Paradigm’s traffic volumes by 15% for both directions.

## Travel Time

### Location and Date

Paradigm conducted travel time surveys on Highway 401 on May 4, 2017 (Weekday) and May 6, 2017 (Saturday). The travel times were run in accordance to FHWA guidelines where multiple runs were conducted in each direction for each period (AM peak period, PM peak period, and the Saturday peak period). MTO provided their travel time data for the AM and PM peak periods. The date and year of the data was not documented in the provided information. It is noted that there is a large difference in the segment length in the between the Paradigm and MTO data for the section between Highway 407 and Winston Churchill Boulevard. Segment discrepancies are likely starting and end points of the segment lengths (i.e., start of an off-ramp, or a point where the Highway 407 mainline crosses the Highway 401).

### Travel Time & Speed

**Table 3** compares the AM and PM peak period and speeds for Highway 401 between James Snow Parkway and Winston Churchill Boulevard. Under the AM peak period, eastbound direction, Paradigm’s travel time and speed are significantly faster from

James Snow Parkway to Winston Churchill Boulevard with the overall travel time almost 3 minutes less compared to MTO. In the westbound direction, the overall travel time is 44 seconds different. MTO iCorridor 2016 speed plot data along Highway 401 was referenced, and provides indication that the average speeds collected by Paradigm are inline with the iCorridor website data.

Under the PM peak period, travel times and speeds are relatively close.

**Table 3: Travel Time and Speeds**

Highway 401 Sections	Segment Distance (km)		AM					
			Average Travel Time (min)			Average Speed (km/h)		
	Paradigm	MTO	Paradigm	MTO	Diff	Paradigm	MTO	Diff
<i>Eastbound</i>								
James Snow Parkway to Trafalgar Rd	4.22	4.37	3.03	5.48	2.45	84	53	30
Trafalgar Rd to Highway 407	2.91	2.49	1.88	2.01	0.13	99	75	24
Highway 407 to Winston Churchill Blvd	1.87	2.56	1.05	1.47	0.42	107	105	2
<b>Total / Average</b>	<b>9.00</b>	<b>9.43</b>	<b>5.95</b>	<b>8.95</b>	<b>3.00</b>	<b>97</b>	<b>78</b>	<b>19</b>
<i>Westbound</i>								
Winston Churchill Blvd to Highway 407	1.87	2.45	0.95	1.69	0.74	118	92	26
Highway 407 to Trafalgar Rd	2.90	2.51	1.47	1.40	0.07	118	108	11
Trafalgar Rd to James Snow Parkway	4.22	4.24	2.28	2.35	0.06	111	109	2
<b>Total / Average</b>	<b>8.99</b>	<b>9.21</b>	<b>4.70</b>	<b>5.44</b>	<b>0.74</b>	<b>116</b>	<b>103</b>	<b>13</b>

Highway 401 Sections	Segment Distance (km)		PM					
			Average Travel Time (min)			Average Speed (km/h)		
	Paradigm	MTO	Paradigm	MTO	Diff	Paradigm	MTO	Diff
<i>Eastbound</i>								
James Snow Parkway to Trafalgar Rd	4.22	4.37	3.02	2.73	-0.30	84	96	-13
Trafalgar Rd to Highway 407	2.91	2.49	1.88	1.53	-0.35	93	98	-5
Highway 407 to Winston Churchill Blvd	1.87	2.56	1.15	1.40	0.25	98	110	-12
<b>Total / Average</b>	<b>9.00</b>	<b>9.43</b>	<b>6.05</b>	<b>5.66</b>	<b>-0.39</b>	<b>92</b>	<b>101</b>	<b>-10</b>
<i>Westbound</i>								
Winston Churchill Blvd to Highway 407	1.87	2.45	3.12	4.44	1.32	36	35	1
Highway 407 to Trafalgar Rd	2.90	2.51	3.88	3.64	-0.25	45	42	3
Trafalgar Rd to James Snow Parkway	4.22	4.24	3.05	3.27	0.22	83	78	5
<b>Total / Average</b>	<b>8.99</b>	<b>9.21</b>	<b>10.05</b>	<b>11.35</b>	<b>-1.30</b>	<b>54</b>	<b>52</b>	<b>3</b>

There could have been several factors (weather, sun, collisions, debris, etc.) that could have caused the difference between the eastbound travel times. For the study, the AM, PM and Saturday peak period travel times by Paradigm should be sufficient.

We trust that the foregoing information will meet your requirements. Please do not hesitate to contact us if we can be of further assistance.

Yours very truly,

**PARADIGM TRANSPORTATION SOLUTIONS LIMITED**



**Gene Chartier**  
M.A.Sc., P.Eng., FITE  
Vice-President

# Appendix B

## VISSIM Parameter Adjustments





**Project:** Toronto Premium Outlet - Scoped Traffic Study - Calibration Report

**Project #:** 170050

**Task:** Coding Assumptions in VISSIM

### Vehicle Inputs

Volumes (Truck %) As calculated  
Length of auto (default)  
Length of heavy truck (default)

### Functions

Maximum acceleration (default)  
Desired acceleration (default)  
Maximum deceleration (default)  
Desired deceleration (default)

### Routings

Static Routing - Turning movements  
Distribution based on existing turning movements

### Desired Speed and Reduced Speed

Highway 401	Posted speed limit (100 km/h)	
Arterials/Roads	James Snow Parkway	Posted speed limit (80 km/h)
	Trafalgar Road	Posted speed limit (60 km/h)
	Winston Churchill Boulevard	Posted speed limit (60 km/h)
	Steeles Ave. (Fifth Line N - S)	Posted speed limit (70 km/h)
	Steeles Ave. (Fifth Line S - Eighth Line S)	Posted speed limit (60 km/h)
	Steeles Ave. (Eighth Line S - Ninth Line S)	Posted speed limit (80 km/h)
	Fifth Line N/Fifth Line S	Posted speed limit (60 km/h)
	Sixth Line N/Sixth Line S	Posted speed limit (60 km/h)
	Eighth Line N/Eighth Line S	Posted speed limit (60 km/h)
	Ninth Line N/Ninth Line S	Posted speed limit (60 km/h)
	Hornby Road	Posted speed limit (60 km/h)

Reduced Speed Right Turns	Auto:	15 km/h
	Truck:	12 km/hr
Reduced Speed Left Turns	Auto:	30 km/h
	Truck:	20 km/h

### Conflict Area/Priority Rules

Priority Rule	Right Turning	
	Min. Gap Time: 3.0 s; Min. Headway: 5.0 m; Max. Speed: 180 km/h	default
Conflict Areas		

### Driving Behaviour Parameter Sets

#### Urban (motorized)

*Following* Wiedemann 74

*Lane Change (defaults)*

*Lateral (defaults)*

*Signal Control (defaults)*

## Freeway (free lane selection)

### Following

Wiedemann 99

Look ahead distance Min: 20 m default = 0.0 m

CC0 (Standstill Distance)	1.5 m	default
<b>CC1 (Headway Time)</b>	<b>0.5</b>	default = 0.9s
CC2 (Following Variation)	4.0 m	default
CC3 (Threshold for Entering 'Following')	-8	default
CC4 (Negative 'Following' Threshold)	-0.35	default
CC5 (Positive 'Following' Threshold)	0.35	default
CC6 (Speed Dependency of Oscillation)	11.44	default
<b>CC7 (Oscillation Acceleration)</b>	<b>0.75 m/s<sup>2</sup></b>	default = 0.25 m/s <sup>2</sup>
<b>CC8 (Standstill Acceleration)</b>	<b>5.00 m/s<sup>2</sup></b>	default = 3.50 m/s <sup>2</sup>
CC9 (Acceleration at 80 km/h)	1.5 m/s <sup>2</sup>	default

### Lane Change

General Behaviour: Free Lane Selection

Necessary lane change (route)	Own	Trailing Vehicle	
Maximum deceleration:	-4.00 m/s <sup>2</sup>	-3.00 m/s <sup>2</sup>	default = -3
- 1 m/s <sup>2</sup> per distance:	200.0 m	200.0 m	default
Accepted deceleration:	<b>-1.00 m/s<sup>2</sup></b>	<b>-1.00 m/s<sup>2</sup></b>	default = 0.
		default = -0.50 m/s <sup>2</sup>	
<b>Waiting time before diffusion: 10.00 s</b>		default=60 s	
Min. headway (front/rear):		default=0.50 s	
<b>Safety distance reduction factor: 0.50</b>			default=0.60
<b>Maximum deceleration for cooperative braking: -7.00 m/s<sup>2</sup></b>			default = -3.00 m/s <sup>2</sup>
Overtake reduced speed areas: checked (active)			
Advanced merging: checked (active)			
Cooperative lane change: checked (active)			
Maximum speed difference: 10.80 km/h		default	
Maximum collision time: 10.00 s		default	

### Lateral (defaults)

### Signal Control (defaults)

## Lane Changing Distance

At least 250 m for each lane change  
 For a 8-lane highway, the maximum number of lane changes is 3, the lane change distance will be 750 m

**Freeway (free lane selection)**

**PM ONLY**

*Following*

Wiedemann 99  
 Look ahead distance Min: 0.50 m default = 0.0 m

CC0 (Standstill Distance) 1.5 m default  
**CC1 (Headway Time) 0.9** default = 0.9s  
 CC2 (Following Variation) 4.0 m default  
 CC3 (Threshold for Entering 'Following') -8 default  
 CC4 (Negative 'Following' Threshold) -0.35 default  
 CC5 (Positive 'Following' Threshold) 0.35 default  
 CC6 (Speed Dependency of Oscillation) 11.44 default  
**CC7 (Oscillation Acceleration) 0.25 m/s<sup>2</sup>** default = 0.25 m/s<sup>2</sup>  
**CC8 (Standstill Acceleration) 3.50 m/s<sup>2</sup>** default = 3.50 m/s<sup>2</sup>  
 CC9 (Acceleration at 80 km/h) 1.5 m/s<sup>2</sup> default

*Lane Change*

General Behaviour: Free Lane Selection

Necessary lane change (route)	Own	Trailing Vehicle	
Maximum deceleration:	-3.50 m/s <sup>2</sup>	-3.50 m/s <sup>2</sup>	default = -3.50 m/s <sup>2</sup>
- 1 m/s <sup>2</sup> per distance:	200.0 m	200.0 m	default
Accepted deceleration:	<b>-0.50 m/s<sup>2</sup></b>	<b>-0.50 m/s<sup>2</sup></b>	default = 0.50 m/s <sup>2</sup>
	default = -0.50 m/s <sup>2</sup>		
<b>Waiting time before diffusion: 50.00 s</b>	default=60 s		
Min. headway (front/rear):	default=0.50 s		
<b>Safety distance reduction factor: 0.60</b>			default=0.60
<b>Maximum deceleration for cooperative braking: -3.50 m/s<sup>2</sup></b>			default = -3.00 m/s <sup>2</sup>
Overtake reduced speed areas: unchecked (unactive)			
Advanced merging: checked (active)			
Cooperative lane change: checked (active)			
Maximum speed difference: 10.80 km/h	default		
Maximum collision time: 10.00 s	default		

*Lateral (defaults)*

*Signal Control (defaults)*

**Lane Changing Distance**

At least 250 m for each lane change  
 For a 8-lane highway, the maximum number of lane changes is 3, the lane change distance will be 750 m

# Appendix C

## Detailed Input and Output Volumes

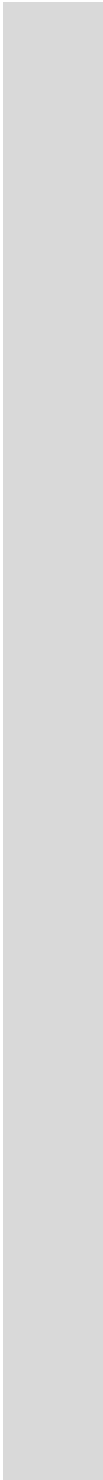


**Project:** Toronto Premium Outlets - Scoped Traffic Study

**Project #** 170050

**Task:** Node Evaluation Results: Volume Check Entering/Exiting Model

Node	Description	AM Peak Hour		
		Volumes	AVG	% Diff
1	Hwy 401 EB - In	4569	4565	0%
2	Hwy 401 WB - Out	3876	3763	-3%
3	Hwy 401 WB - In	3446	3474	1%
4	Hwy 401 EB - Out	3434	3320	-3%
5	Steeles EB - In	941	951	1%
6	Steeles WB - Out	526	475	-10%
7	Steeles WB - In	812	823	1%
8	Steeles EB - Out	1400	1338	-4%
9	James Snow Pkwy NB - In	1038	1038	0%
10	James Snow Pkwy SB - Out	1030	980	-5%
11	James Snow Pkwy SB - In	600	581	-3%
12	James Snow Pkwy NB - Out	888	873	-2%
13	Fifth Line N SB - In	89	92	3%
14	Fifth Line N NB - Out	91	83	-9%
15	Brownridge Rd NB - In	14	14	0%
16	Brownridge Rd SB - Out	157	165	5%
17	Fifth Line S NB - In	9	10	11%
18	Fifth Line S SB - Out	27	28	4%
19	Sixth Line N SB - In	41	33	-20%
20	Sixth Line N NB - Out	36	40	11%
21	Sixth Line S NB - In	2	2	0%
22	Sixth Line S SB - Out	5	4	-20%
23	Trafalgar Rd SB - In	1229	1193	-3%
24	Trafalgar Rd NB - Out	361	325	-10%
25	Trafalgar Rd NB - In	1025	1018	-1%
26	Trafalgar Rd SB - Out	1446	1389	-4%
27	Central Mall NB - In	47	48	2%
28	Central Mall SB - Out	40	21	-48%
29	Eighth Line N SB - In	536	489	-9%
30	Eighth Line N NB - Out	98	90	-8%
31	East Mall NB - In	12	12	0%
32	East Mall SB - Out	49	49	0%
33	Eighth Line S NB - In	1	1	0%
34	Eighth Line S SB - Out	2	2	0%
35	Ninth Line N SB - In	641	639	0%
36	Ninth Line N NB - Out	288	285	-1%
37	Ninth Line S NB - In	440	443	1%
38	Ninth Line S SB - Out	738	714	-3%
39	Hwy 401 EB to Hwy 407 WB - Out	173	171	-1%
40	Hwy 407 EB to Hwy 401 EB - In	256	249	-3%
41	Hwy 401 EB to Hwy 407 EB - Out	2131	2104	-1%
42	Hwy 407 EB to Hwy 401 WB - In	133	123	-8%
43	Hwy 407 WB to Hwy 401 WB - In	676	683	1%
44	Hwy 401 WB to Hwy 407 WB - Out	86	97	13%
45	Winston Churchill SB - In	1366	1328	-3%
46	Winston Churchill NB - Out	1558	1551	0%
47	Winston Churchill NB - In	1225	1231	0%
48	Winston Churchill SB - Out	1337	1310	-2%
67	Main St EB - In	1374	1381	1%
68	Main St WB - Out	475	481	1%
69	Main St WB - In	165	168	2%
70	Main St EB - Out	444	439	-1%
		41383	40686	-2%



4  
113  
-28  
114  
-10  
51  
-11  
62  
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50  
19  
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-3  
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-8  
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8  
-4  
0  
1  
36  
36  
7  
57  
-1  
19  
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-3  
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7  
27  
10  
-7  
-11  
38  
7  
-6  
27  
-7  
-6  
-3  
5  
697

In	20687	20589	-0.5%
Out	20696	20097	-2.9%

**Project:** Toronto Premium Outlets - Scoped Traffic Study  
**Project #** 170050  
**Task:** Node Evaluation Results: Volume Check Entering/Exiting Model

Node	Description	PM Peak Hour		
		Volumes	AVG	% Diff
1	Hwy 401 EB - In	4523	4527	0%
2	Hwy 401 WB - Out	4957	4850	-2%
3	Hwy 401 WB - In	4178	4178	0%
4	Hwy 401 EB - Out	3254	3219	-1%
5	Steeles EB - In	739	746	1%
6	Steeles WB - Out	1145	1098	-4%
7	Steeles WB - In	1838	1836	0%
8	Steeles EB - Out	1016	970	-5%
9	James Snow Pkwy NB - In	960	960	0%
10	James Snow Pkwy SB - Out	1037	1017	-2%
11	James Snow Pkwy SB - In	998	955	-4%
12	James Snow Pkwy NB - Out	688	701	2%
13	Fifth Line N SB - In	169	170	1%
14	Fifth Line N NB - Out	66	61	-8%
15	Brownridge Rd NB - In	85	91	7%
16	Brownridge Rd SB - Out	8	8	0%
17	Fifth Line S NB - In	23	23	0%
18	Fifth Line S SB - Out	3	2	-33%
19	Sixth Line N SB - In	36	29	-19%
20	Sixth Line N NB - Out	62	66	6%
21	Sixth Line S NB - In	9	8	-11%
22	Sixth Line S SB - Out	0	0	#DIV/0!
23	Trafalgar Rd SB - In	500	482	-4%
24	Trafalgar Rd NB - Out	902	825	-9%
25	Trafalgar Rd NB - In	1091	1084	-1%
26	Trafalgar Rd SB - Out	1122	1079	-4%
27	Central Mall NB - In	272	279	3%
28	Central Mall SB - Out	388	335	-14%
29	Eighth Line N SB - In	110	115	5%
30	Eighth Line N NB - Out	357	328	-8%
31	East Mall NB - In	178	179	1%
32	East Mall SB - Out	148	152	3%
33	Eighth Line S NB - In	6	7	17%
34	Eighth Line S SB - Out	2	2	0%
35	Ninth Line N SB - In	333	336	1%
36	Ninth Line N NB - Out	809	796	-2%
37	Ninth Line S NB - In	805	809	0%
38	Ninth Line S SB - Out	520	518	0%
39	Hwy 401 EB to Hwy 407 WB - Out	207	207	0%
40	Hwy 407 EB to Hwy 401 EB - In	152	156	3%
41	Hwy 401 EB to Hwy 407 EB - Out	1329	1318	-1%
42	Hwy 407 EB to Hwy 401 WB - In	207	208	0%
43	Hwy 407 WB to Hwy 401 WB - In	1902	1887	-1%
44	Hwy 401 WB to Hwy 407 WB - Out	363	364	0%
45	Winston Churchill SB - In	1287	1249	-3%
46	Winston Churchill NB - Out	1700	1656	-3%
47	Winston Churchill NB - In	1856	1841	-1%
48	Winston Churchill SB - Out	1606	1584	-1%
67	Main St EB - In	663	670	1%
68	Main St WB - Out	1186	1176	-1%
69	Main St WB - In	232	244	5%
70	Main St EB - Out	277	266	-4%
		46304	45667	-1%

-4
107
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43
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-6
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0
1
7
-4
1
0
18
77
7
43
-7
53
-5
29
-1
-4
-1
0
-3
13
-4
2
0
-4
11
-1
15
-1
38
44
15
22
-7
10
-12
11
637

In	23152	23069	-0.4%
Out	23152	22598	-2.4%



**Project:** Toronto Premium Outlets - Scoped Traffic Study  
**Project #** 170050  
**Task:** Node Evaluation Results: Volume Check Entering/Exiting Model

Node	Description	SAT Peak Hour		
		Volumes	AVG	% Diff
1	Hwy 401 EB - In	4388	4347	-1%
2	Hwy 401 WB - Out	4077	3996	-2%
3	Hwy 401 WB - In	4661	4670	0%
4	Hwy 401 EB - Out	4124	3975	-4%
5	Steeles EB - In	433	433	0%
6	Steeles WB - Out	543	496	-9%
7	Steeles WB - In	687	699	2%
8	Steeles EB - Out	837	809	-3%
9	James Snow Pkwy NB - In	585	585	0%
10	James Snow Pkwy SB - Out	1199	1158	-3%
11	James Snow Pkwy SB - In	883	847	-4%
12	James Snow Pkwy NB - Out	676	666	-1%
13	Fifth Line N SB - In	49	49	0%
14	Fifth Line N NB - Out	31	28	-10%
15	Brownridge Rd NB - In	17	17	0%
16	Brownridge Rd SB - Out	9	11	22%
17	Fifth Line S NB - In	2	2	0%
18	Fifth Line S SB - Out	3	3	0%
19	Sixth Line N SB - In	23	18	-22%
20	Sixth Line N NB - Out	40	39	-3%
21	Sixth Line S NB - In	0	0	#DIV/0!
22	Sixth Line S SB - Out	0	0	#DIV/0!
23	Trafalgar Rd SB - In	596	570	-4%
24	Trafalgar Rd NB - Out	623	575	-8%
25	Trafalgar Rd NB - In	1247	1248	0%
26	Trafalgar Rd SB - Out	795	771	-3%
27	Central Mall NB - In	444	457	3%
28	Central Mall SB - Out	649	573	-12%
29	Eighth Line N SB - In	132	135	2%
30	Eighth Line N NB - Out	176	155	-12%
31	East Mall NB - In	373	376	1%
32	East Mall SB - Out	213	211	-1%
33	Eighth Line S NB - In	8	10	25%
34	Eighth Line S SB - Out	3	2	-33%
35	Ninth Line N SB - In	366	365	0%
36	Ninth Line N NB - Out	364	362	-1%
37	Ninth Line S NB - In	317	327	3%
38	Ninth Line S SB - Out	278	283	2%
39	Hwy 401 EB to Hwy 407 WB - Out	155	158	2%
40	Hwy 407 EB to Hwy 401 EB - In	84	91	8%
41	Hwy 401 EB to Hwy 407 EB - Out	865	833	-4%
42	Hwy 407 EB to Hwy 401 WB - In	135	124	-8%
43	Hwy 407 WB to Hwy 401 WB - In	585	595	2%
44	Hwy 401 WB to Hwy 407 WB - Out	72	72	0%
45	Winston Churchill SB - In	1000	974	-3%
46	Winston Churchill NB - Out	918	896	-2%
47	Winston Churchill NB - In	1021	1020	0%
48	Winston Churchill SB - Out	1336	1306	-2%
67	Main St EB - In		0	#DIV/0!
68	Main St WB - Out		0	#DIV/0!
69	Main St WB - In		0	#DIV/0!
70	Main St EB - Out		0	#DIV/0!
		36022	35337	-2%

41  
81  
-9  
149  
0  
47  
-12  
28  
0  
41  
36  
10  
0  
3  
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-2  
0  
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5  
1  
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0  
26  
48  
-1  
24  
-13  
76  
-3  
21  
-3  
2  
-2  
1  
1  
2  
-10  
-5  
-3  
-7  
32  
11  
-10  
0  
26  
22  
1  
30  
0  
0  
0  
0  
685

In	18036	17959	-0.4%
Out	17986	17378	-3.4%

# Appendix D

## Detailed Intersection Turning Movement Volumes































# Appendix E

## Lane by Lane Speed Plots













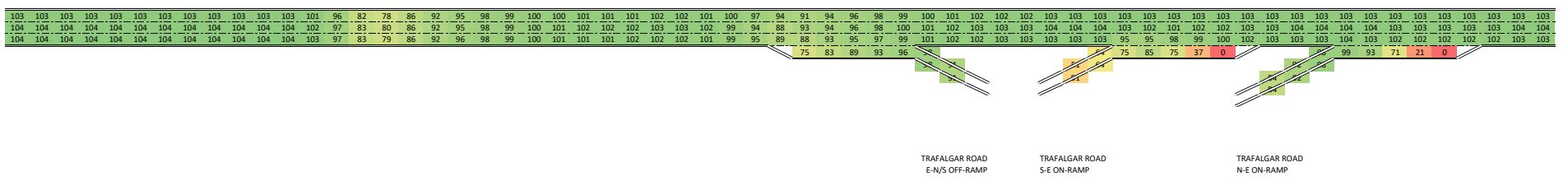
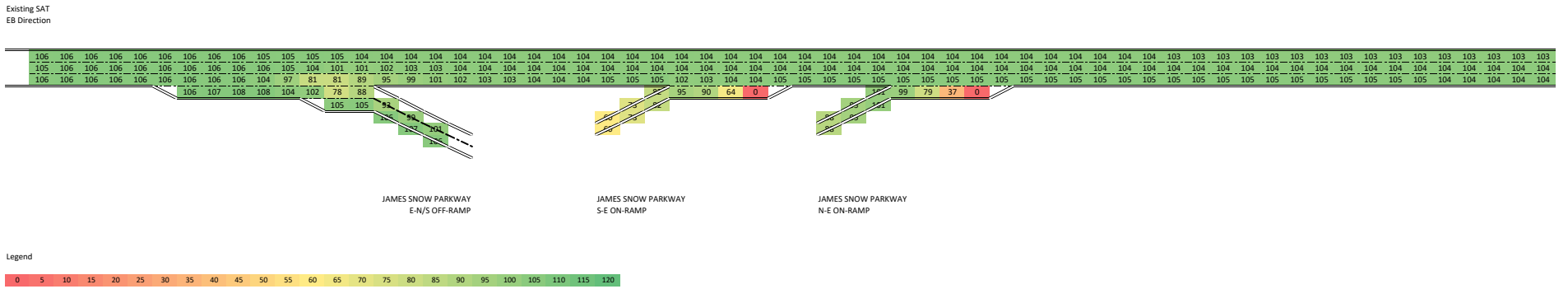








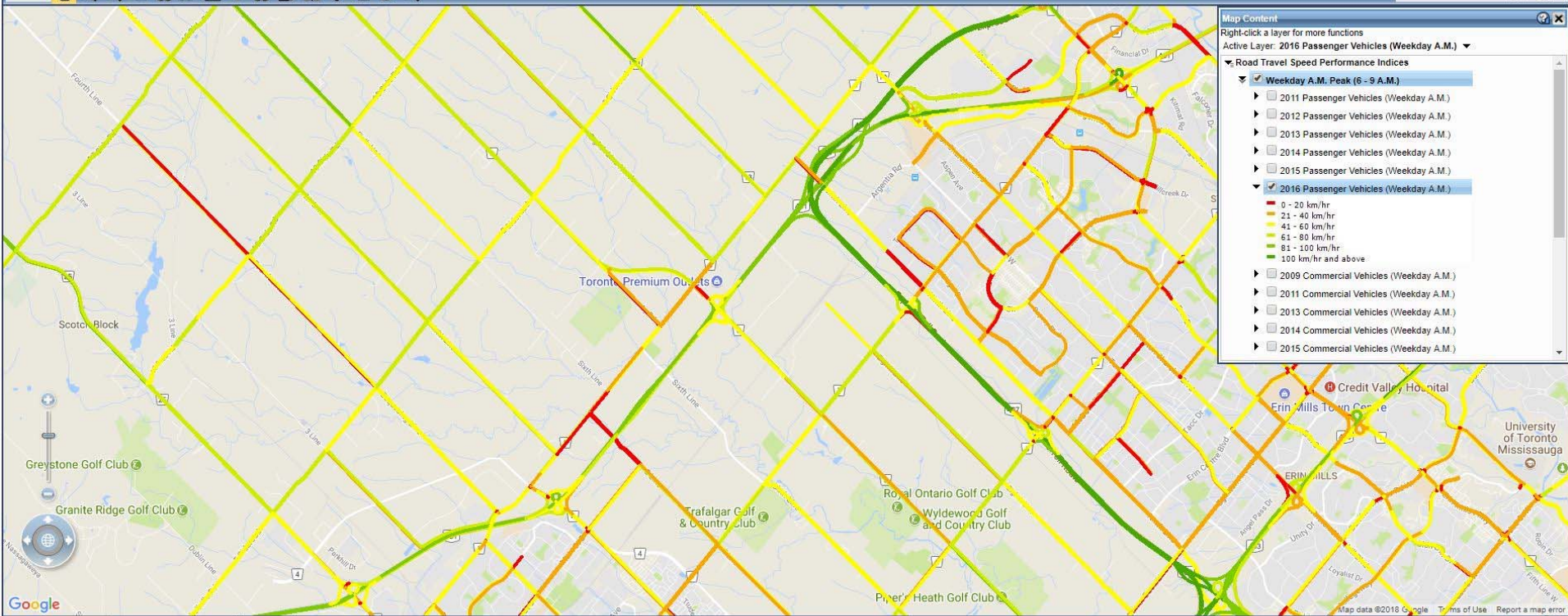
Existing SAT  
EB Direction











**Map Content**

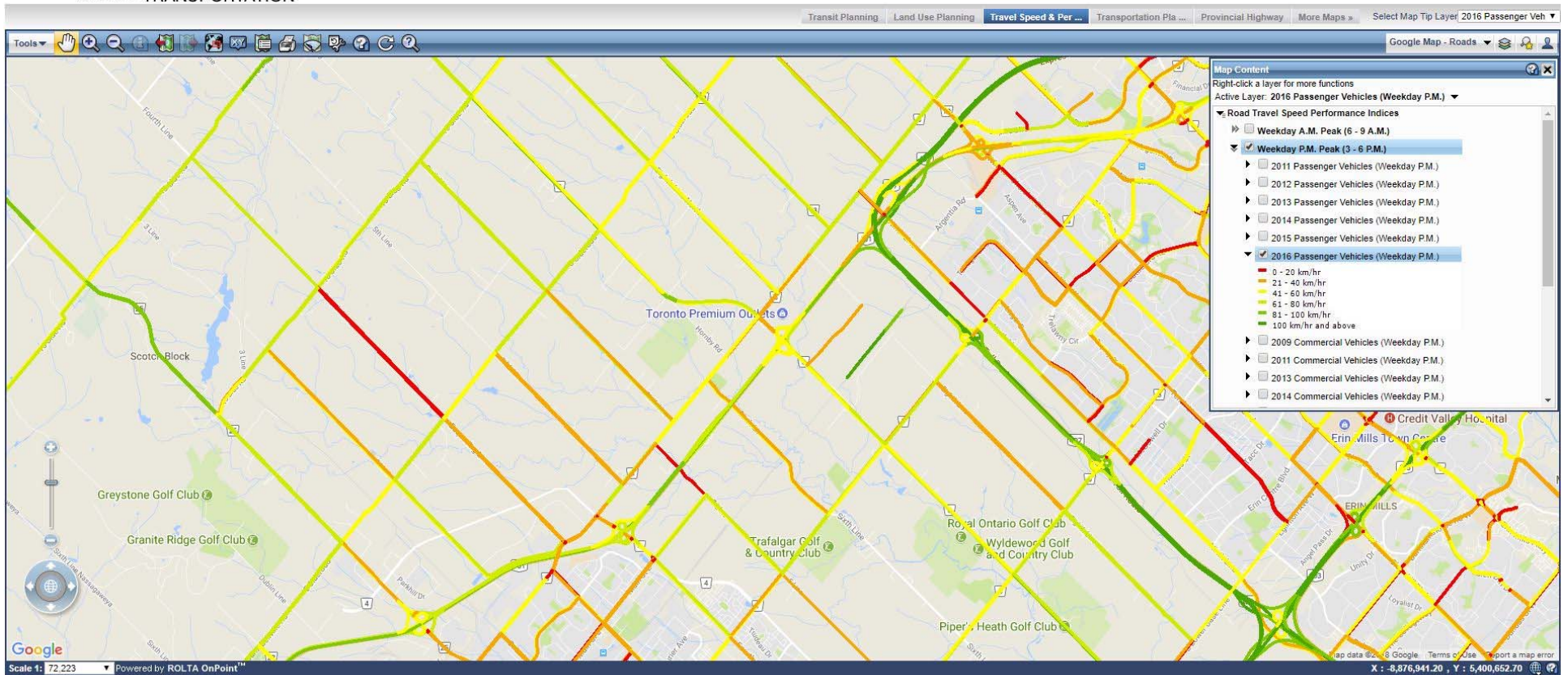
Right-click a layer for more functions

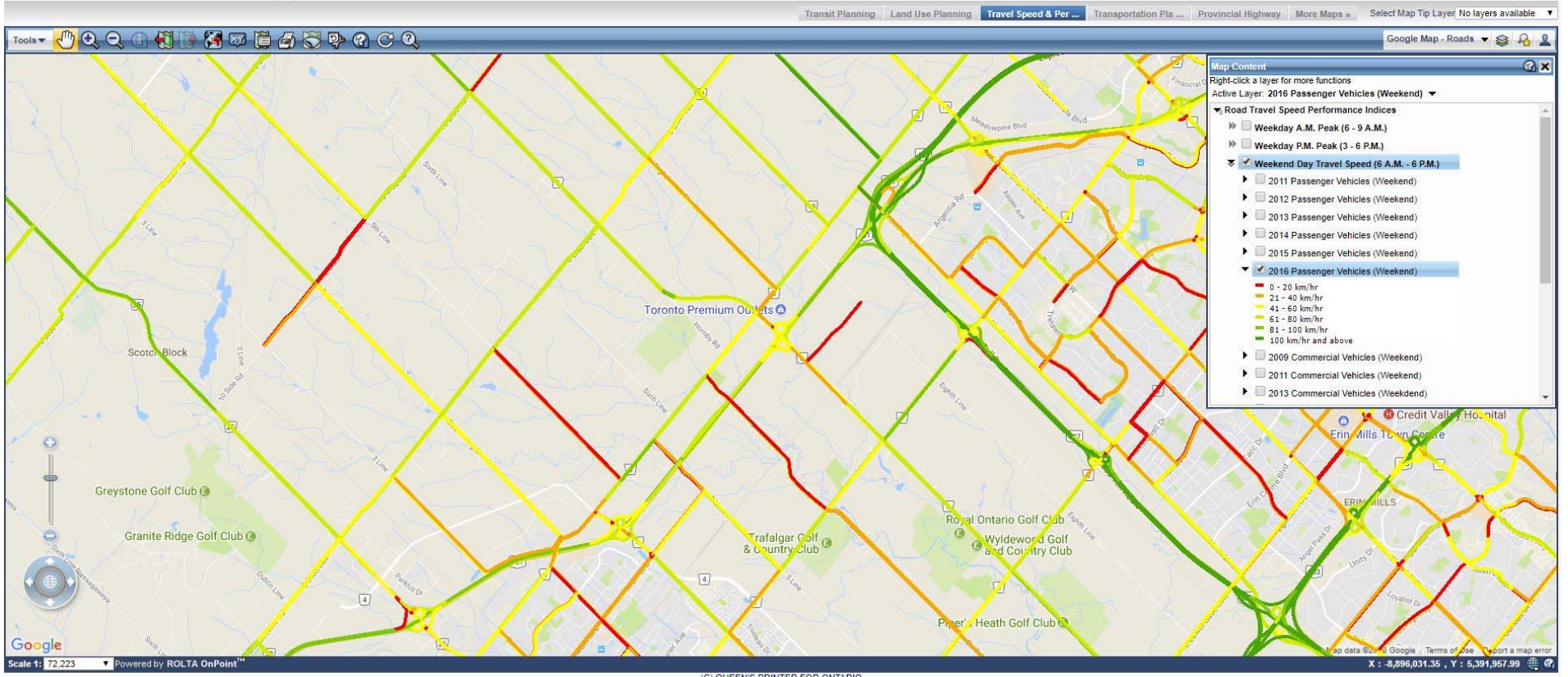
Active Layer: 2016 Passenger Vehicles (Weekday A.M.)

▼ Road Travel Speed Performance Indices

- ▼ Weekday A.M. Peak (6 - 9 A.M.)
  - 2011 Passenger Vehicles (Weekday A.M.)
  - 2012 Passenger Vehicles (Weekday A.M.)
  - 2013 Passenger Vehicles (Weekday A.M.)
  - 2014 Passenger Vehicles (Weekday A.M.)
  - 2015 Passenger Vehicles (Weekday A.M.)
  - 2016 Passenger Vehicles (Weekday A.M.)
- 2009 Commercial Vehicles (Weekday A.M.)
- 2011 Commercial Vehicles (Weekday A.M.)
- 2013 Commercial Vehicles (Weekday A.M.)
- 2014 Commercial Vehicles (Weekday A.M.)
- 2015 Commercial Vehicles (Weekday A.M.)









26 February 2018

**File**

170050

**To**

File

**From**

Gene Chartier, P.Eng., FITE  
Vice-President  
Paradigm Transportation Solutions Limited

## Premier Gateway West Scoped Area Traffic Study Synchro and SimTraffic Model Analysis Parameters and Methodology

This memorandum documents the parameter adjustments and methodology applied for the Synchro and SimTraffic models developed and utilized for analysis in the Premier Gateway West Scoped Area Traffic Study.

### Synchro Parameter Adjustments and Methodology

Synchro will be used to assess traffic operational performance at intersections and optimize traffic control signal timings. The following parameters adjustments were made in calibrating the models to base year conditions, or will be applied in horizon year analyses:

- ▶ Existing lane arrangements and auxiliary turn lane storage and taper lengths were obtained from field conditions.
- ▶ Heavy vehicle percentages were derived from existing traffic count data.
- ▶ Conflicting pedestrian volumes were derived from existing traffic count data.
- ▶ Existing signal timing parameters provided by responsible road authority were used (includes minimum green times, clearance intervals, pedestrian walk and flashing do not walk (FDW) times, vehicle extensions).
- ▶ For future horizon year assessments, signal timing splits and intersection offsets will be optimized (only applicable if signal timing plans are currently coordinated and if warranted by the traffic growth flow patterns due to proposed development trips and distribution).
- ▶ Calculated overall intersection peak hour factors were applied to all study area intersections.
- ▶ For any traffic movements with a volume-to-capacity (V/C) ratio greater than 1.00 under existing conditions, Halton Region indicated that no adjustments to default Synchro parameters will be accepted. The Region will allow for signal phasing changes under existing conditions. If capacity issues remain following signal phasing adjustments, Halton

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Region will review and advise if saturation flow rate adjustments are warranted.

- ▶ Synchro default values were applied for all other inputs, including left and right turning speeds, lost time adjustment values, etc.

### **SimTraffic Parameter Adjustments and Methodology**

Given the proximity of several intersections within the study area, SimTraffic will be used to assess vehicle queuing, including the interaction of adjacent intersection queues and potential queue impacts. The following parameters adjustments were made in calibrating the models to base year conditions, or will be applied in horizon year analyses:

- ▶ Seeding interval time of 15-minutes.
- ▶ Recording/Analysis time of 60-minutes.
- ▶ Five (5) simulation runs, beginning with random seed zero (0).
- ▶ 95<sup>th</sup> percentile queue results will be taken from the five (5) simulation runs report generated using the “Multiple Runs” option.
- ▶ SimTraffic default values for all other inputs, including vehicle, driver and interval parameters.

### **PARADIGM TRANSPORTATION SOLUTIONS LIMITED**



**Gene Chartier**  
M.A.Sc., P.Eng., FITE  
Vice-President

# Appendix B

## Traffic Count Data



# Ontario Traffic Inc

## Morning Peak Diagram

### Specified Period

**From:** 6:00:00  
**To:** 10:00:00

### One Hour Peak

**From:** 7:30:00  
**To:** 8:30:00

**Municipality:** Milton  
**Site #:** 1711400001  
**Intersection:** Trafalgar Rd & Hwy 401 North Rarr  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

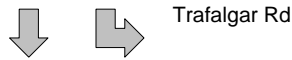
North Leg Total: 2255  
North Entering: 1519  
North Peds: 0  
Peds Cross:  $\times$

Heavys	0	0	0
Trucks	119	0	119
Cars	1400	0	1400
<b>Totals</b>	<b>1519</b>	<b>0</b>	

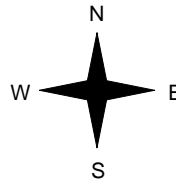


Heavys	0
Trucks	125
Cars	611
<b>Totals</b>	<b>736</b>

East Leg Total: 394  
East Entering: 394  
East Peds: 0  
Peds Cross:  $\times$



Trafalgar Rd



	Cars	Trucks	Heavys	Totals
	177	105	0	282
	69	43	0	112
	<b>246</b>	<b>148</b>	<b>0</b>	

Hwy 401 North Ramp Terminal



Cars	Trucks	Heavys	Totals
0	0	0	0

Cars	1469
Trucks	162
Heavys	0
<b>Totals</b>	<b>1631</b>



Trafalgar Rd

Cars	434	0	434
Trucks	20	0	20
Heavys	0	0	0
<b>Totals</b>	<b>454</b>	<b>0</b>	

Peds Cross:  $\times$   
South Peds: 0  
South Entering: 454  
South Leg Total: 2085

## Comments

# Ontario Traffic Inc

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 19:00:00

### One Hour Peak

**From:** 17:00:00

**To:** 18:00:00

**Municipality:** Milton  
**Site #:** 1711400001  
**Intersection:** Trafalgar Rd & Hwy 401 North Rarr  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

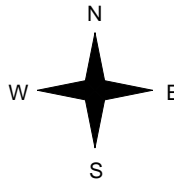
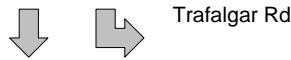
**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 2072  
 North Entering: 684  
 North Peds: 0  
 Peds Cross:  $\times$

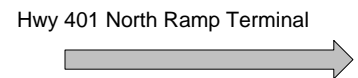
Heavys	0	0	0
Trucks	37	0	37
Cars	647	0	647
<b>Totals</b>	<b>684</b>	<b>0</b>	

Heavys	0
Trucks	50
Cars	1338
<b>Totals</b>	<b>1388</b>

East Leg Total: 887  
 East Entering: 887  
 East Peds: 0  
 Peds Cross:  $\times$



	Cars	Trucks	Heavys	Totals
	564	24	0	588
	288	11	0	299
	852	35	0	



	Cars	Trucks	Heavys	Totals
	0	0	0	0
	0	0	0	0

Cars	935	Cars	774	0	774
Trucks	48	Trucks	26	0	26
Heavys	0	Heavys	0	0	0
<b>Totals</b>	<b>983</b>	<b>Totals</b>	<b>800</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 800  
 South Leg Total: 1783

## Comments

# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400001  
**Intersection:** Trafalgar Rd & Hwy 401 North Rarr  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

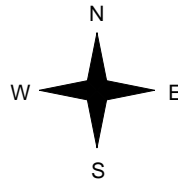
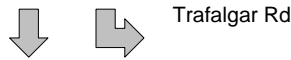
**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 14604  
 North Entering: 6823  
 North Peds: 0  
 Peds Cross:  $\times$

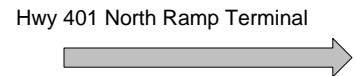
Heavys	0	0	0
Trucks	623	0	623
Cars	6200	0	6200
<b>Totals</b>	<b>6823</b>	<b>0</b>	

Heavys	0
Trucks	747
Cars	7034
<b>Totals</b>	<b>7781</b>

East Leg Total: 4562  
 East Entering: 4562  
 East Peds: 0  
 Peds Cross:  $\times$



	Cars	Trucks	Heavys	Totals
	2629	521	0	3150
	1209	203	0	1412
	<b>3838</b>	<b>724</b>	<b>0</b>	



Cars	Trucks	Heavys	Totals
0	0	0	0

Cars	7409
Trucks	826
Heavys	0
<b>Totals</b>	<b>8235</b>

Cars	4405	0	4405
Trucks	226	0	226
Heavys	0	0	0
<b>Totals</b>	<b>4631</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 4631  
 South Leg Total: 12866

### Comments



# Ontario Traffic Inc Traffic Count Summary

Intersection: Trafalgar Rd & Hwy 401 North Rd    Count Date: 4-May-17    Municipality: Milton

North Approach Totals						South Approach Totals						
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds	North/South Total Approaches	Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	913	0	913	0	1272	7:00:00	0	359	0	359	0
8:00:00	0	1364	0	1364	0	1815	8:00:00	0	451	0	451	0
9:00:00	0	1268	0	1268	0	1730	9:00:00	0	462	0	462	0
10:00:00	0	785	0	785	0	1206	10:00:00	0	421	0	421	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	0	626	0	626	0	1305	16:00:00	0	679	0	679	0
17:00:00	0	657	0	657	0	1436	17:00:00	0	779	0	779	0
18:00:00	0	684	0	684	0	1484	18:00:00	0	800	0	800	0
19:00:00	0	526	0	526	0	1206	19:00:00	0	680	0	680	0
Totals:	0	6823	0	6823	0	11454		0	4631	0	4631	0
East Approach Totals						West Approach Totals						
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds	East/West Total Approaches	Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	108	0	207	315	0	315	7:00:00	0	0	0	0	0
8:00:00	99	0	257	356	0	356	8:00:00	0	0	0	0	0
9:00:00	103	0	268	371	0	371	9:00:00	0	0	0	0	0
10:00:00	91	0	301	392	0	392	10:00:00	0	0	0	0	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	212	0	525	737	0	737	16:00:00	0	0	0	0	0
17:00:00	195	0	447	642	0	642	17:00:00	0	0	0	0	0
18:00:00	299	0	588	887	0	887	18:00:00	0	0	0	0	0
19:00:00	305	0	557	862	0	862	19:00:00	0	0	0	0	0
Totals:	1412	0	3150	4562	0	4562		0	0	0	0	0
Calculated Values for Traffic Crossing Major Street												
Hours Ending:	7:00	8:00	9:00	10:00			16:00	17:00	18:00	19:00		
Crossing Values:	108	99	103	91			212	195	299	305		









# Ontario Traffic Inc

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00  
**To:** 15:00:00

### One Hour Peak

**From:** 13:30:00  
**To:** 14:30:00

**Municipality:** Milton  
**Site #:** 1711400010  
**Intersection:** James Snow Pkwy & Hwy 401 Sou  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 1623  
North Entering: 1084  
North Peds: 0  
Peds Cross:  $\times$

Heavys	0	0	0
Trucks	0	9	9
Cars	0	1075	1075
<b>Totals</b>	<b>0</b>	<b>1084</b>	



Heavys	0
Trucks	25
Cars	514
<b>Totals</b>	<b>539</b>

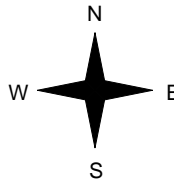
Heavys	Trucks	Cars	Totals
0	0	0	0



James Snow Pkwy



Hwy 401 South Ramp Terminal



Heavys	Trucks	Cars	Totals
0	15	49	64
0	0	115	115
0	15	164	



James Snow Pkwy



Peds Cross:  $\times$   
West Peds: 0  
West Entering: 179  
West Leg Total: 179

Cars	1190
Trucks	9
Heavys	0
<b>Totals</b>	<b>1199</b>



Cars	0	465	465
Trucks	0	10	10
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>475</b>	

Peds Cross:  $\times$   
South Peds: 0  
South Entering: 475  
South Leg Total: 1674

## Comments

# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400010  
**Intersection:** James Snow Pkwy & Hwy 401 Sou  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 5982  
 North Entering: 3999  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	35	35
Cars	0	3964	3964
<b>Totals</b>	<b>0</b>	<b>3999</b>	



Heavys	0
Trucks	91
Cars	1892
<b>Totals</b>	<b>1983</b>

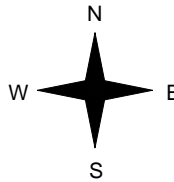
Heavys	Trucks	Cars	Totals
0	0	0	0



James Snow Pkwy



Hwy 401 South Ramp Terminal



Heavys	Trucks	Cars	Totals
0	63	185	248
0	12	431	443
0	75	616	



James Snow Pkwy



Peds Cross:  $\nabla$   
 West Peds: 0  
 West Entering: 691  
 West Leg Total: 691

Cars	4395
Trucks	47
Heavys	0
<b>Totals</b>	<b>4442</b>



Cars	0	1707	1707
Trucks	0	28	28
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>1735</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 1735  
 South Leg Total: 6177

### Comments

# Ontario Traffic Inc

## Traffic Count Summary

Intersection: James Snow Pkwy & Hwy 401 So | Count Date: 6-May-17 | Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	0	909	0	909	0	1351	12:00:00	0	442	0	442	0
13:00:00	0	961	0	961	0	1369	13:00:00	0	408	0	408	0
14:00:00	0	1058	0	1058	0	1503	14:00:00	0	445	0	445	0
15:00:00	0	1071	0	1071	0	1511	15:00:00	0	440	0	440	0
<b>Totals:</b>						5734	<b>Totals:</b>					
0	3999	0	3999	0								
<b>East Approach Totals</b> <th rowspan="3" style="text-align: center;">East/West Total Approaches</th> <th colspan="6" style="text-align: center;"><b>West Approach Totals</b></th>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	0	0	0	0	0	148	12:00:00	48	0	100	148	0
13:00:00	0	0	0	0	0	183	13:00:00	65	0	118	183	0
14:00:00	0	0	0	0	0	171	14:00:00	69	0	102	171	0
15:00:00	0	0	0	0	0	189	15:00:00	66	0	123	189	0
<b>Totals:</b>						691	<b>Totals:</b>					
0	0	0	0	0								
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	0:00	0:00	0:00	11:00					12:00	13:00	14:00	15:00
Crossing Values:	0	0	0	0					48	65	69	66











# James Snow Pkwy @ Main St E

## Morning Peak Diagram

### Specified Period

**From:** 5:30:00  
**To:** 10:00:00

### One Hour Peak

**From:** 7:30:00  
**To:** 8:30:00

**Municipality:** Halton Region  
**Site #:** 0000002734  
**Intersection:** James Snow Pkwy & Main St E  
**TFR File #:** 23  
**Count date:** 8-Nov-2016

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 2484  
North Entering: 737  
North Peds: 6  
Peds Cross:  $\times$

Heavys	6	29	6	41
Trucks	8	8	2	18
Cars	176	412	90	678
Totals	190	449	98	



Heavys	31
Trucks	9
Cars	1707
Totals	1747

East Leg Total: 594  
East Entering: 172  
East Peds: 3  
Peds Cross:  $\times$

Heavys	13
Trucks	8
Cars	411
Totals	432

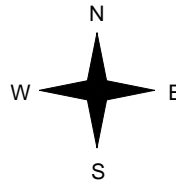


James Snow Pkwy

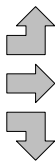
Cars	45	Trucks	0	Heavys	10	Totals	55
Cars	42	Trucks	0	Heavys	1	Totals	43
Cars	72	Trucks	1	Heavys	1	Totals	74
Cars	159	Trucks	1	Heavys	12	Totals	



Main St E



Heavys	8
Trucks	4
Cars	847
Totals	859
Heavys	0
Trucks	0
Cars	215
Totals	215
Heavys	6
Trucks	0
Cars	400
Totals	406
Heavys	14
Trucks	4
Cars	1462
Totals	



Main St E



Cars	412	Trucks	3	Heavys	7	Totals	422
------	-----	--------	---	--------	---	--------	-----

Peds Cross:  $\times$   
West Peds: 2  
West Entering: 1480  
West Leg Total: 1912

Cars	884	Cars	193	815	107	1115
Trucks	9	Trucks	0	5	1	6
Heavys	36	Heavys	6	13	1	20
Totals	929	Totals	199	833	109	



Peds Cross:  $\times$   
South Peds: 2  
South Entering: 1141  
South Leg Total: 2070

## Comments

# James Snow Pkwy @ Main St E

## Mid-day Peak Diagram

### Specified Period

**From:** 12:00:00

**To:** 14:00:00

### One Hour Peak

**From:** 12:15:00

**To:** 13:15:00

**Municipality:** Halton Region  
**Site #:** 0000002734  
**Intersection:** James Snow Pkwy & Main St E  
**TFR File #:** 23  
**Count date:** 8-Nov-2016

**Weather conditions:**  
 Clear/Dry  
**Person(s) who counted:**  
 Cam

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 1482  
 North Entering: 793  
 North Peds: 7  
 Peds Cross:  $\bowtie$

Heavys	8	27	5	40
Trucks	11	7	4	22
Cars	324	283	124	731
<b>Totals</b>	<b>343</b>	<b>317</b>	<b>133</b>	



Heavys	40
Trucks	12
Cars	637
<b>Totals</b>	<b>689</b>

East Leg Total: 505  
 East Entering: 204  
 East Peds: 4  
 Peds Cross:  $\bowtie$

Heavys	Trucks	Cars	Totals
8	13	547	568

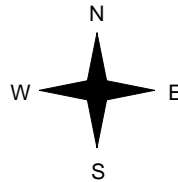


James Snow Pkwy

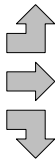
Cars	Trucks	Heavys	Totals
25	3	13	41
89	1	0	90
67	1	5	73
<b>181</b>	<b>5</b>	<b>18</b>	



Main St E



Heavys	Trucks	Cars	Totals
4	3	292	299
2	0	95	97
0	1	139	140
<b>6</b>	<b>4</b>	<b>526</b>	



Main St E



Cars	Trucks	Heavys	Totals
285	4	12	301

Peds Cross:  $\bowtie$   
 West Peds: 1  
 West Entering: 536  
 West Leg Total: 1104

Cars	489	Cars	134	320	66	520
Trucks	9	Trucks	1	6	0	7
Heavys	32	Heavys	0	23	5	28
<b>Totals</b>	<b>530</b>	<b>Totals</b>	<b>135</b>	<b>349</b>	<b>71</b>	



Peds Cross:  $\bowtie$   
 South Peds: 4  
 South Entering: 555  
 South Leg Total: 1085

## Comments

# James Snow Pkwy @ Main St E

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 22:00:00

### One Hour Peak

**From:** 17:15:00

**To:** 18:15:00

**Municipality:** Halton Region  
**Site #:** 0000002734  
**Intersection:** James Snow Pkwy & Main St E  
**TFR File #:** 23  
**Count date:** 8-Nov-2016

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 2346  
 North Entering: 1554  
 North Peds: 3  
 Peds Cross:  $\bowtie$

Heavys	5	1	1	7
Trucks	2	1	1	4
Cars	727	729	87	1543
Totals	734	731	89	



Heavys	6
Trucks	9
Cars	777
Totals	792

East Leg Total: 509  
 East Entering: 229  
 East Peds: 0  
 Peds Cross:  $\bowtie$

Heavys	Trucks	Cars	Totals
5	2	1201	1208

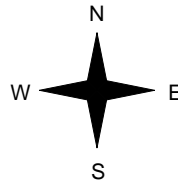


James Snow Pkwy

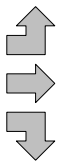
Cars	Trucks	Heavys	Totals
30	0	1	31
119	0	0	119
79	0	0	79
228	0	1	



Main St E



Heavys	Trucks	Cars	Totals
0	0	316	316
0	0	70	70
1	0	248	249
1	0	634	



Main St E



James Snow Pkwy

Cars	Trucks	Heavys	Totals
276	2	2	280

Peds Cross:  $\bowtie$   
 West Peds: 0  
 West Entering: 635  
 West Leg Total: 1843

Cars	1056	Cars	355	431	119	905
Trucks	1	Trucks	0	9	1	10
Heavys	2	Heavys	0	5	1	6
Totals	1059	Totals	355	445	121	



Peds Cross:  $\bowtie$   
 South Peds: 1  
 South Entering: 921  
 South Leg Total: 1980

## Comments

# James Snow Pkwy @ Main St E

## Total Count Diagram

**Municipality:** Halton Region  
**Site #:** 0000002734  
**Intersection:** James Snow Pkwy & Main St E  
**TFR File #:** 23  
**Count date:** 8-Nov-2016

**Weather conditions:**  
 Clear/Dry  
**Person(s) who counted:**  
 Cam

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 23144  
 North Entering: 11963  
 North Peds: 29  
 Peds Cross:  $\times$

Heavys	55	187	54	296
Trucks	51	61	22	134
Cars	4851	5626	1056	11533
<b>Totals</b>	<b>4957</b>	<b>5874</b>	<b>1132</b>	



Heavys	295
Trucks	106
Cars	10780
<b>Totals</b>	<b>11181</b>

East Leg Total: 5573  
 East Entering: 2127  
 East Peds: 22  
 Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
82	67	8256	8405

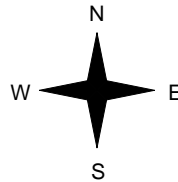


James Snow Pkwy

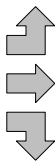
Cars	Trucks	Heavys	Totals
318	8	50	376
888	9	3	900
801	9	41	851
<b>2007</b>	<b>26</b>	<b>94</b>	



Main St E



Heavys	Trucks	Cars	Totals
52	45	4968	5065
10	3	1261	1274
22	10	2770	2802
<b>84</b>	<b>58</b>	<b>8999</b>	



James Snow Pkwy

Main St E



Cars	Trucks	Heavys	Totals
3314	30	102	3446

Peds Cross:  $\times$   
 West Peds: 6  
 West Entering: 9141  
 West Leg Total: 17546

Cars	9197	Cars	2517	5494	997	9008
Trucks	80	Trucks	7	53	5	65
Heavys	250	Heavys	24	193	38	255
<b>Totals</b>	<b>9527</b>	<b>Totals</b>	<b>2548</b>	<b>5740</b>	<b>1040</b>	



Peds Cross:  $\times$   
 South Peds: 24  
 South Entering: 9328  
 South Leg Total: 18855

### Comments



# Trafalgar Rd @ 5 Side Rd

## Morning Peak Diagram

### Specified Period

**From:** 7:00:00

**To:** 9:00:00

### One Hour Peak

**From:** 7:15:00

**To:** 8:15:00

**Municipality:** Halton Region  
**Site #:** 0000002630  
**Intersection:** Trafalgar Rd & 5 Side Rd  
**TFR File #:** 4  
**Count date:** 27-May-2015

**Weather conditions:**  
 Cloudy/Dry  
**Person(s) who counted:**  
 Les

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 1317  
 North Entering: 947  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	7	36	2	45
Trucks	1	26	4	31
Cars	24	826	21	871
<b>Totals</b>	<b>32</b>	<b>888</b>	<b>27</b>	



Heavys	67
Trucks	16
Cars	287
<b>Totals</b>	<b>370</b>

East Leg Total: 475  
 East Entering: 158  
 East Peds: 0  
 Peds Cross:  $\nabla$

Heavys	Trucks	Cars	Totals
11	2	129	142



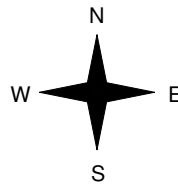
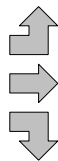
Trafalgar Rd

Cars	Trucks	Heavys	Totals
3	0	5	8
93	0	1	94
52	3	1	56
<b>148</b>	<b>3</b>	<b>7</b>	



5 Side Rd

Heavys	Trucks	Cars	Totals
1	2	31	34
4	4	241	249
4	2	232	238
<b>9</b>	<b>8</b>	<b>504</b>	



Trafalgar Rd



Cars	Trucks	Heavys	Totals
297	8	12	317

Peds Cross:  $\nabla$   
 West Peds: 0  
 West Entering: 521  
 West Leg Total: 663

Cars	1110
Trucks	31
Heavys	41
<b>Totals</b>	<b>1182</b>



Cars	12	253	35	300
Trucks	1	14	0	15
Heavys	3	61	6	70
<b>Totals</b>	<b>16</b>	<b>328</b>	<b>41</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 385  
 South Leg Total: 1567

## Comments

# Trafalgar Rd @ 5 Side Rd

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00  
**To:** 14:00:00

### One Hour Peak

**From:** 13:00:00  
**To:** 14:00:00

**Municipality:** Halton Region  
**Site #:** 0000002630  
**Intersection:** Trafalgar Rd & 5 Side Rd  
**TFR File #:** 4  
**Count date:** 27-May-2015

**Weather conditions:**  
Cloudy/Dry  
**Person(s) who counted:**  
Les

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 781  
North Entering: 401  
North Peds: 0  
Peds Cross:  $\times$

Heavys	6	57	0	63
Trucks	3	20	0	23
Cars	19	290	6	315
Totals	28	367	6	



Heavys	53
Trucks	26
Cars	301
Totals	380

East Leg Total: 137  
East Entering: 75  
East Peds: 0  
Peds Cross:  $\times$

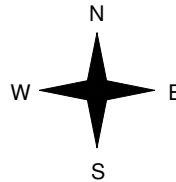
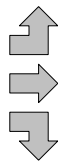
Heavys	Trucks	Cars	Totals
11	3	93	107



Trafalgar Rd

Cars	Trucks	Heavys	Totals
7	0	0	7
53	0	1	54
13	1	0	14
73	1	1	

Heavys	Trucks	Cars	Totals
1	2	11	14
0	3	35	38
5	4	18	27
6	9	64	



5 Side Rd



Peds Cross:  $\times$   
West Peds: 0  
West Entering: 79  
West Leg Total: 186

Cars	321	Cars	21	283	17	321
Trucks	25	Trucks	0	24	1	25
Heavys	62	Heavys	4	52	0	56
Totals	408	Totals	25	359	18	



Trafalgar Rd

Cars	Trucks	Heavys	Totals
58	4	0	62

Peds Cross:  $\times$   
South Peds: 1  
South Entering: 402  
South Leg Total: 810

## Comments

# Trafalgar Rd @ 5 Side Rd

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00  
**To:** 18:00:00

### One Hour Peak

**From:** 16:30:00  
**To:** 17:30:00

**Municipality:** Halton Region  
**Site #:** 0000002630  
**Intersection:** Trafalgar Rd & 5 Side Rd  
**TFR File #:** 4  
**Count date:** 27-May-2015

### Weather conditions:

Cloudy/Dry

### Person(s) who counted:

Les

### \*\* Signalized Intersection \*\*

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 1423  
North Entering: 520  
North Peds: 0  
Peds Cross:  $\times$

Heavys	2	17	0	19
Trucks	2	7	1	10
Cars	54	431	6	491
<b>Totals</b>	<b>58</b>	<b>455</b>	<b>7</b>	



Heavys	17
Trucks	23
Cars	863
<b>Totals</b>	<b>903</b>

East Leg Total: 474  
East Entering: 272  
East Peds: 0  
Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
2	5	335	342

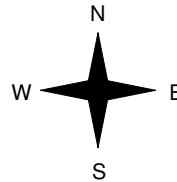


Trafalgar Rd

Cars	Trucks	Heavys	Totals
35	0	0	35
197	1	0	198
39	0	0	39
<b>271</b>	<b>1</b>	<b>0</b>	



5 Side Rd



Heavys	Trucks	Cars	Totals
1	3	43	47
1	2	141	144
3	0	32	35
<b>5</b>	<b>5</b>	<b>216</b>	



5 Side Rd



Peds Cross:  $\times$   
West Peds: 0  
West Entering: 226  
West Leg Total: 568

Cars	502	Cars	84	785	51	920
Trucks	7	Trucks	2	20	0	22
Heavys	20	Heavys	0	16	0	16
<b>Totals</b>	<b>529</b>	<b>Totals</b>	<b>86</b>	<b>821</b>	<b>51</b>	



Trafalgar Rd



Cars	Trucks	Heavys	Totals
198	3	1	202

Peds Cross:  $\times$   
South Peds: 0  
South Entering: 958  
South Leg Total: 1487

## Comments

# Trafalgar Rd @ 5 Side Rd

## Total Count Diagram

**Municipality:** Halton Region  
**Site #:** 0000002630  
**Intersection:** Trafalgar Rd & 5 Side Rd  
**TFR File #:** 4  
**Count date:** 27-May-2015

**Weather conditions:**  
 Cloudy/Dry  
**Person(s) who counted:**  
 Les

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 8562  
 North Entering: 4370  
 North Peds: 4  
 Peds Cross:  $\bowtie$

Heavys	35	330	8	373
Trucks	15	131	7	153
Cars	230	3542	72	3844
<b>Totals</b>	<b>280</b>	<b>4003</b>	<b>87</b>	



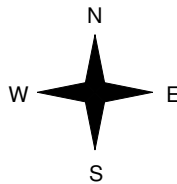
Heavys	359
Trucks	162
Cars	3671
<b>Totals</b>	<b>4192</b>

East Leg Total: 2418  
 East Entering: 1175  
 East Peds: 2  
 Peds Cross:  $\bowtie$

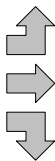
Heavys	Trucks	Cars	Totals
66	34	1286	1386



5 Side Rd



Heavys	Trucks	Cars	Totals
9	14	211	234
10	20	887	917
39	8	526	573
58	42	1624	



Cars	Trucks	Heavys	Totals
99	0	9	108
775	11	7	793
265	4	5	274
1139	15	21	



5 Side Rd



Cars	Trucks	Heavys	Totals
1184	32	27	1243

Peds Cross:  $\bowtie$   
 West Peds: 0  
 West Entering: 1724  
 West Leg Total: 3110

Cars	4333
Trucks	143
Heavys	374
<b>Totals</b>	<b>4850</b>



Trafalgar Rd

Cars	281	3361	225	3867
Trucks	8	148	5	161
Heavys	24	341	9	374
<b>Totals</b>	<b>313</b>	<b>3850</b>	<b>239</b>	

Peds Cross:  $\bowtie$   
 South Peds: 2  
 South Entering: 4402  
 South Leg Total: 9252

### Comments

# Ninth Line @ 5 Side Rd

## Morning Peak Diagram

### Specified Period

**From:** 7:00:00

**To:** 9:00:00

### One Hour Peak

**From:** 7:15:00

**To:** 8:15:00

**Municipality:** Halton Region  
**Site #:** 0000002592  
**Intersection:** Ninth Line & 5 Side Rd  
**TFR File #:** 3  
**Count date:** 26-May-2015

### Weather conditions:

Cloudy/Dry

### Person(s) who counted:

Les

### \*\* Signalized Intersection \*\*

**Major Road:** Ninth Line runs N/S

North Leg Total: 885

North Entering: 671

North Peds: 4

Peds Cross:  $\times$

Heavys	1	15	0	16
Trucks	0	8	0	8
Cars	25	363	259	647
<b>Totals</b>	<b>26</b>	<b>386</b>	<b>259</b>	



Heavys 13

Trucks 10

Cars 191

Totals 214

East Leg Total: 736

East Entering: 123

East Peds: 0

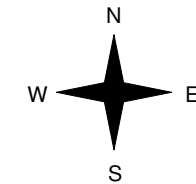
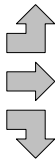
Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
3	2	136	141



5 Side Rd

Heavys	Trucks	Cars	Totals
1	1	19	21
5	8	329	342
1	0	15	16
<b>7</b>	<b>9</b>	<b>363</b>	



Ninth Line

Cars	Trucks	Heavys	Totals
12	0	0	12
108	1	1	110
1	0	0	1
<b>121</b>	<b>1</b>	<b>1</b>	

5 Side Rd



Cars	Trucks	Heavys	Totals
600	8	5	613

Peds Cross:  $\times$

West Peds: 0

West Entering: 379

West Leg Total: 520

Cars	379
Trucks	8
Heavys	16
<b>Totals</b>	<b>403</b>



Cars	3	160	12	175
Trucks	1	9	0	10
Heavys	1	12	0	13
<b>Totals</b>	<b>5</b>	<b>181</b>	<b>12</b>	

Peds Cross:  $\times$

South Peds: 0

South Entering: 198

South Leg Total: 601

## Comments

# Ninth Line @ 5 Side Rd

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00

**To:** 14:00:00

### One Hour Peak

**From:** 12:45:00

**To:** 13:45:00

**Municipality:** Halton Region  
**Site #:** 0000002592  
**Intersection:** Ninth Line & 5 Side Rd  
**TFR File #:** 3  
**Count date:** 26-May-2015

### Weather conditions:

Cloudy/Dry

### Person(s) who counted:

Les

### \*\* Signalized Intersection \*\*

**Major Road:** Ninth Line runs N/S

North Leg Total: 425

North Entering: 210

North Peds: 0

Peds Cross:  $\times$

Heavys	0	13	0	13
Trucks	1	9	0	10
Cars	8	146	33	187
<b>Totals</b>	<b>9</b>	<b>168</b>	<b>33</b>	



Heavys	9
Trucks	9
Cars	197
<b>Totals</b>	<b>215</b>

East Leg Total: 274

East Entering: 96

East Peds: 1

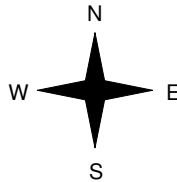
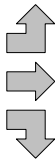
Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
0	3	81	84



5 Side Rd

Heavys	Trucks	Cars	Totals
0	1	29	30
4	3	136	143
0	1	12	13
4	5	177	



Ninth Line

Cars	Trucks	Heavys	Totals
31	0	0	31
63	1	0	64
0	1	0	1
94	2	0	

5 Side Rd



Cars	Trucks	Heavys	Totals
170	4	4	178

Peds Cross:  $\times$

West Peds: 0

West Entering: 186

West Leg Total: 270

Cars	158	Cars	10	137	1	148
Trucks	11	Trucks	1	8	1	10
Heavys	13	Heavys	0	9	0	9
<b>Totals</b>	<b>182</b>	<b>Totals</b>	<b>11</b>	<b>154</b>	<b>2</b>	



Peds Cross:  $\times$

South Peds: 0

South Entering: 167

South Leg Total: 349

## Comments

# Ninth Line @ 5 Side Rd

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 18:00:00

### One Hour Peak

**From:** 16:45:00

**To:** 17:45:00

**Municipality:** Halton Region  
**Site #:** 0000002592  
**Intersection:** Ninth Line & 5 Side Rd  
**TFR File #:** 3  
**Count date:** 26-May-2015

### Weather conditions:

Cloudy/Dry

### Person(s) who counted:

Les

### \*\* Signalized Intersection \*\*

**Major Road:** Ninth Line runs N/S

North Leg Total: 929

North Entering: 282

North Peds: 0

Peds Cross:  $\times$

Heavys	0	3	0	3
Trucks	1	8	0	9
Cars	19	224	27	270
<b>Totals</b>	<b>20</b>	<b>235</b>	<b>27</b>	



Heavys	5
Trucks	5
Cars	637
<b>Totals</b>	<b>647</b>

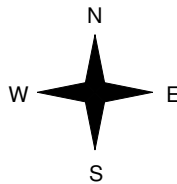
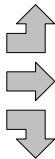
East Leg Total: 806  
 East Entering: 573  
 East Peds: 0  
 Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
0	2	374	376



5 Side Rd

Heavys	Trucks	Cars	Totals
1	0	24	25
0	1	196	197
0	0	14	14
<b>1</b>	<b>1</b>	<b>234</b>	



Ninth Line

Cars	Trucks	Heavys	Totals
222	0	1	223
343	1	0	344
6	0	0	6
<b>571</b>	<b>1</b>	<b>1</b>	

5 Side Rd



Cars	Trucks	Heavys	Totals
231	2	0	233

Peds Cross:  $\times$

West Peds: 0

West Entering: 236

West Leg Total: 612

Cars	244	Cars	12	391	8	411
Trucks	8	Trucks	0	5	1	6
Heavys	3	Heavys	0	3	0	3
<b>Totals</b>	<b>255</b>	<b>Totals</b>	<b>12</b>	<b>399</b>	<b>9</b>	



Peds Cross:  $\times$

South Peds: 0

South Entering: 420

South Leg Total: 675

## Comments

# Ninth Line @ 5 Side Rd

## Total Count Diagram

**Municipality:** Halton Region  
**Site #:** 0000002592  
**Intersection:** Ninth Line & 5 Side Rd  
**TFR File #:** 3  
**Count date:** 26-May-2015

**Weather conditions:**  
 Cloudy/Dry  
**Person(s) who counted:**  
 Les

**\*\* Signalized Intersection \*\***

**Major Road:** Ninth Line runs N/S

North Leg Total: 5334  
 North Entering: 2660  
 North Peds: 5  
 Peds Cross:  $\bowtie$

Heavys	4	93	1	98
Trucks	4	57	1	62
Cars	128	1717	655	2500
Totals	136	1867	657	



Heavys	81
Trucks	67
Cars	2526
Totals	2674

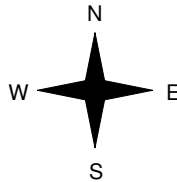
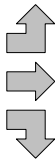
East Leg Total: 3786  
 East Entering: 1760  
 East Peds: 2  
 Peds Cross:  $\bowtie$

Heavys	Trucks	Cars	Totals
12	20	1305	1337



5 Side Rd

Heavys	Trucks	Cars	Totals
4	7	133	144
17	19	1290	1326
4	2	67	73
25	28	1490	



Ninth Line



Cars	Trucks	Heavys	Totals
593	5	3	601
1119	10	5	1134
22	2	1	25
1734	17	9	

5 Side Rd



Cars	Trucks	Heavys	Totals
1986	22	18	2026

Peds Cross:  $\bowtie$   
 West Peds: 0  
 West Entering: 1543  
 West Leg Total: 2880

Cars	1806	Cars	58	1800	41	1899
Trucks	61	Trucks	6	55	2	63
Heavys	98	Heavys	3	74	0	77
Totals	1965	Totals	67	1929	43	



Peds Cross:  $\bowtie$   
 South Peds: 3  
 South Entering: 2039  
 South Leg Total: 4004

### Comments



# Ontario Traffic Inc

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00

**To:** 15:00:00

### One Hour Peak

**From:** 12:00:00

**To:** 13:00:00

**Municipality:** Milton  
**Site #:** 1711400011  
**Intersection:** Winston Churchill Blvd & Hwy 401 I  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Winston Churchill Blvd runs N/S

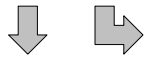
North Leg Total: 1708  
 North Entering: 790  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	0	0	0
Trucks	23	0	23
Cars	767	0	767
<b>Totals</b>	<b>790</b>	<b>0</b>	

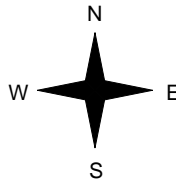


Heavys	0
Trucks	46
Cars	872
<b>Totals</b>	<b>918</b>

East Leg Total: 903  
 East Entering: 903  
 East Peds: 2  
 Peds Cross:  $\times$



Winston Churchill Blvd



Cars	Trucks	Heavys	Totals
240	30	0	270
617	16	0	633
<b>857</b>	<b>46</b>	<b>0</b>	

Hwy 401 North Ramp Termin



Cars	Trucks	Heavys	Totals
0	0	0	0

Cars	1384
Trucks	39
Heavys	0
<b>Totals</b>	<b>1423</b>



Cars	632	0	632
Trucks	16	0	16
Heavys	0	0	0
<b>Totals</b>	<b>648</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 648  
 South Leg Total: 2071

## Comments

# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400011  
**Intersection:** Winston Churchill Blvd & Hwy 401 I  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

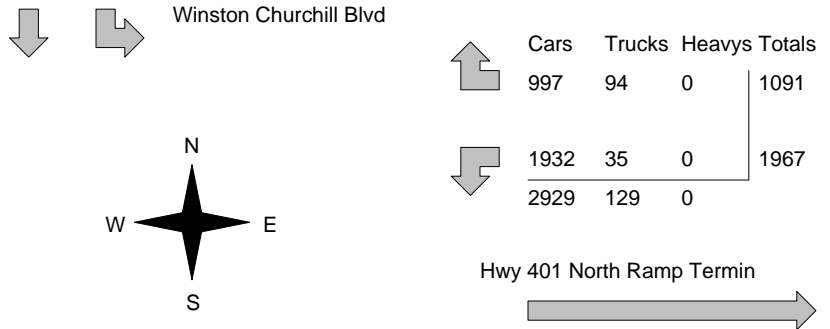
**Major Road:** Winston Churchill Blvd runs N/S

North Leg Total: 6657  
 North Entering: 3028  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	0	0	0
Trucks	87	0	87
Cars	2941	0	2941
<b>Totals</b>	<b>3028</b>	<b>0</b>	

Heavys	0
Trucks	153
Cars	3476
<b>Totals</b>	<b>3629</b>

East Leg Total: 3058  
 East Entering: 3058  
 East Peds: 2  
 Peds Cross:  $\times$



Cars	4873		Cars	2479	0	2479
Trucks	122		Trucks	59	0	59
Heavys	0		Heavys	0	0	0
<b>Totals</b>	<b>4995</b>		<b>Totals</b>	<b>2538</b>	<b>0</b>	

Cars	Trucks	Heavys	Totals
0	0	0	0

Cars	Trucks	Heavys	Totals
997	94	0	1091
1932	35	0	1967
<b>2929</b>	<b>129</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 2538  
 South Leg Total: 7533

### Comments

# Ontario Traffic Inc Traffic Count Summary

Intersection: Winston Churchill Blvd & Hwy 401    Count Date: 6-May-17    Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	0	757	0	757	0	1339	12:00:00	0	582	0	582	0
13:00:00	0	790	0	790	0	1438	13:00:00	0	648	0	648	0
14:00:00	0	669	0	669	0	1324	14:00:00	0	655	0	655	0
15:00:00	0	812	0	812	0	1465	15:00:00	0	653	0	653	0
<b>Totals:</b>	0	3028	0	3028	0	5566		0	2538	0	2538	0
<b>East Approach Totals</b>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	385	0	232	617	0	617	12:00:00	0	0	0	0	0
13:00:00	633	0	270	903	2	903	13:00:00	0	0	0	0	0
14:00:00	508	0	331	839	0	839	14:00:00	0	0	0	0	0
15:00:00	441	0	258	699	0	699	15:00:00	0	0	0	0	0
<b>Totals:</b>	1967	0	1091	3058	2	3058		0	0	0	0	0
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	11:00	12:00	13:00	13:00		14:00	14:00	15:00	15:00			
Crossing Values:	0	385	633	633		508	508	441	441			









# Ontario Traffic Inc

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00

**To:** 15:00:00

### One Hour Peak

**From:** 13:15:00

**To:** 14:15:00

**Municipality:** Milton  
**Site #:** 1711400012  
**Intersection:** Winston Churchill Blvd & Hwy 401 S  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Winston Churchill Blvd runs N/S

North Leg Total: 2016  
 North Entering: 1052  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	0	0	0
Trucks	0	15	15
Cars	0	1037	1037
<b>Totals</b>	<b>0</b>	<b>1052</b>	



Heavys	0
Trucks	24
Cars	940
<b>Totals</b>	<b>964</b>

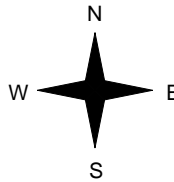
Heavys	Trucks	Cars	Totals
0	0	0	0



Winston Churchill Blvd



Hwy 401 South Ramp Termin



Heavys	Trucks	Cars	Totals
0	14	179	193
0	3	281	284
0	17	460	



Winston Churchill Blvd

Peds Cross:  $\times$   
 West Peds: 0  
 West Entering: 477  
 West Leg Total: 477

Cars	1318
Trucks	18
Heavys	0
<b>Totals</b>	<b>1336</b>



Cars	0	761	761
Trucks	0	10	10
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>771</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 771  
 South Leg Total: 2107

## Comments



# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400012  
**Intersection:** Winston Churchill Blvd & Hwy 401 S  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Winston Churchill Blvd runs N/S

North Leg Total: 7676  
 North Entering: 4031  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	52	52
Cars	0	3979	3979
<b>Totals</b>	<b>0</b>	<b>4031</b>	

Heavys	0
Trucks	76
Cars	3569
<b>Totals</b>	<b>3645</b>



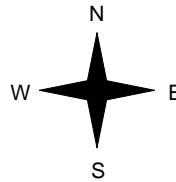
Heavys	Trucks	Cars	Totals
0	0	0	0



Winston Churchill Blvd



Hwy 401 South Ramp Termin



Heavys	Trucks	Cars	Totals
0	44	691	735
0	13	1047	1060
0	57	1738	



Winston Churchill Blvd



Peds Cross:  $\nabla$   
 West Peds: 0  
 West Entering: 1795  
 West Leg Total: 1795

Cars	5026
Trucks	65
Heavys	0
<b>Totals</b>	<b>5091</b>



Cars	0	2878	2878
Trucks	0	32	32
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>2910</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 2910  
 South Leg Total: 8001

### Comments

# Ontario Traffic Inc

## Traffic Count Summary

Intersection: Winston Churchill Blvd & Hwy 401      Count Date: 6-May-17      Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	0	924	0	924	0	1562	12:00:00	0	638	0	638	0
13:00:00	0	1057	0	1057	0	1795	13:00:00	0	738	0	738	0
14:00:00	0	1053	0	1053	0	1800	14:00:00	0	747	0	747	0
15:00:00	0	997	0	997	0	1784	15:00:00	0	787	0	787	0
Totals:	0	4031	0	4031	0	6941	0	2910	0	2910	0	
<b>East Approach Totals</b>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	0	0	0	0	0	435	12:00:00	173	0	262	435	0
13:00:00	0	0	0	0	0	476	13:00:00	200	0	276	476	0
14:00:00	0	0	0	0	0	478	14:00:00	190	0	288	478	0
15:00:00	0	0	0	0	0	406	15:00:00	172	0	234	406	0
Totals:	0	0	0	0	0	1795	735	0	1060	1795	0	
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	0:00	0:00	0:00	11:00		12:00	13:00	14:00	15:00			
Crossing Values:	0	0	0	0		173	200	190	172			









# Ontario Traffic Inc

## Morning Peak Diagram

### Specified Period

**From:** 6:00:00  
**To:** 10:00:00

### One Hour Peak

**From:** 7:30:00  
**To:** 8:30:00

**Municipality:** Milton  
**Site #:** 1711400002  
**Intersection:** Trafalgar Rd & Hwy 401 South Ran  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 2138  
North Entering: 834  
North Peds: 0  
Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	85	85
Cars	0	749	749
<b>Totals</b>	<b>0</b>	<b>834</b>	



Heavys	0
Trucks	68
Cars	1236
<b>Totals</b>	<b>1304</b>

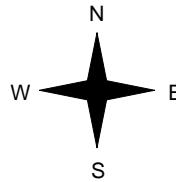
Heavys	Trucks	Cars	Totals
0	0	0	0



Trafalgar Rd



Hwy 401 South Ramp Terminal



Heavys	Trucks	Cars	Totals
0	4	41	45
0	10	175	185
0	14	216	



Trafalgar Rd



Peds Cross:  $\nabla$   
West Peds: 0  
West Entering: 230  
West Leg Total: 230

Cars	924
Trucks	95
Heavys	0
<b>Totals</b>	<b>1019</b>



Cars	0	1195	1195
Trucks	0	64	64
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>1259</b>	

Peds Cross:  $\nabla$   
South Peds: 0  
South Entering: 1259  
South Leg Total: 2278

## Comments

# Ontario Traffic Inc

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 19:00:00

### One Hour Peak

**From:** 17:00:00

**To:** 18:00:00

**Municipality:** Milton  
**Site #:** 1711400002  
**Intersection:** Trafalgar Rd & Hwy 401 South Ran  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 1942

North Entering: 756

North Peds: 0

Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	22	22
Cars	0	734	734
<b>Totals</b>	<b>0</b>	<b>756</b>	



Heavys	0
Trucks	54
Cars	1132
<b>Totals</b>	<b>1186</b>

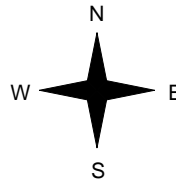
Heavys	Trucks	Cars	Totals
0	0	0	0



Trafalgar Rd



Hwy 401 South Ramp Terminal



Heavys	Trucks	Cars	Totals
0	10	171	181
0	15	351	366
0	25	522	



Trafalgar Rd



Peds Cross:  $\nabla$   
 West Peds: 0  
 West Entering: 547  
 West Leg Total: 547

Cars	1085
Trucks	37
Heavys	0
<b>Totals</b>	<b>1122</b>



Cars	0	961	961
Trucks	0	44	44
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>1005</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 1005  
 South Leg Total: 2127

## Comments



# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400002  
**Intersection:** Trafalgar Rd & Hwy 401 South Ran  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 13765  
 North Entering: 5139  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	387	387
Cars	0	4752	4752
<b>Totals</b>	<b>0</b>	<b>5139</b>	

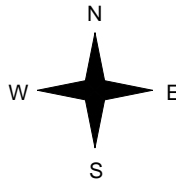
Heavys	0
Trucks	556
Cars	8070
<b>Totals</b>	<b>8626</b>

Heavys	Trucks	Cars	Totals
0	0	0	0



Hwy 401 South Ramp Terminal

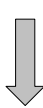
Heavys	Trucks	Cars	Totals
0	81	904	985
0	125	2085	2210
0	206	2989	



Trafalgar Rd

Peds Cross:  $\nabla$   
 West Peds: 0  
 West Entering: 3195  
 West Leg Total: 3195

Cars	6837
Trucks	512
Heavys	0
<b>Totals</b>	<b>7349</b>



Cars	0	7166	7166
Trucks	0	475	475
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>7641</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 7641  
 South Leg Total: 14990

### Comments

# Ontario Traffic Inc

## Traffic Count Summary

Intersection: Trafalgar Rd & Hwy 401 South Ra    Count Date: 4-May-17    Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	569	0	569	0	1248	7:00:00	0	679	0	679	0
8:00:00	0	786	0	786	0	1868	8:00:00	0	1082	0	1082	0
9:00:00	0	707	0	707	0	1903	9:00:00	0	1196	0	1196	0
10:00:00	0	479	0	479	0	1275	10:00:00	0	796	0	796	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	0	587	0	587	0	1538	16:00:00	0	951	0	951	0
17:00:00	0	641	0	641	0	1739	17:00:00	0	1098	0	1098	0
18:00:00	0	756	0	756	0	1761	18:00:00	0	1005	0	1005	0
19:00:00	0	614	0	614	0	1448	19:00:00	0	834	0	834	0
Totals:	0	5139	0	5139	0	12780		0	7641	0	7641	0
<b>East Approach Totals</b>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	0	0	0	0	444	7:00:00	102	0	342	444	0
8:00:00	0	0	0	0	0	385	8:00:00	71	0	314	385	0
9:00:00	0	0	0	0	0	239	9:00:00	55	0	184	239	0
10:00:00	0	0	0	0	0	301	10:00:00	101	0	200	301	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	0	0	0	0	0	437	16:00:00	171	0	266	437	0
17:00:00	0	0	0	0	0	477	17:00:00	161	0	316	477	0
18:00:00	0	0	0	0	0	547	18:00:00	181	0	366	547	0
19:00:00	0	0	0	0	0	365	19:00:00	143	0	222	365	0
Totals:	0	0	0	0	0	3195		985	0	2210	3195	0
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	7:00	8:00	9:00	10:00		16:00	17:00	18:00	19:00			
Crossing Values:	102	71	55	101		171	161	181	143			









# Ontario Traffic Inc

## Morning Peak Diagram

### Specified Period

**From:** 6:00:00  
**To:** 10:00:00

### One Hour Peak

**From:** 7:30:00  
**To:** 8:30:00

**Municipality:** Milton  
**Site #:** 1711400003  
**Intersection:** James Snow Pkwy & Hwy 401 North  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

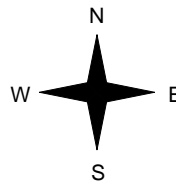
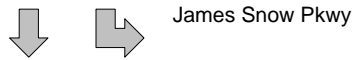
**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 1383  
North Entering: 495  
North Peds: 0  
Peds Cross:  $\times$

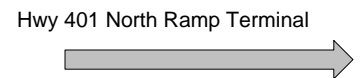
Heavys	0	0	0
Trucks	30	0	30
Cars	465	0	465
<b>Totals</b>	<b>495</b>	<b>0</b>	

Heavys	0
Trucks	96
Cars	792
<b>Totals</b>	<b>888</b>

East Leg Total: 702  
East Entering: 702  
East Peds: 0  
Peds Cross:  $\times$



	Cars	Trucks	Heavys	Totals
	319	35	0	354
	340	8	0	348
	<b>659</b>	<b>43</b>	<b>0</b>	



Cars	Trucks	Heavys	Totals
0	0	0	0

James Snow Pkwy

Cars	805	Cars	473	0	473
Trucks	38	Trucks	61	0	61
Heavys	0	Heavys	0	0	0
<b>Totals</b>	<b>843</b>	<b>Totals</b>	<b>534</b>	<b>0</b>	

Peds Cross:  $\times$   
South Peds: 0  
South Entering: 534  
South Leg Total: 1377

## Comments

# Ontario Traffic Inc

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 19:00:00

### One Hour Peak

**From:** 16:45:00

**To:** 17:45:00

**Municipality:** Milton  
**Site #:** 1711400003  
**Intersection:** James Snow Pkwy & Hwy 401 North  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

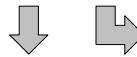
North Leg Total: 1576  
 North Entering: 888  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	0	0	0
Trucks	26	0	26
Cars	862	0	862
<b>Totals</b>	<b>888</b>	<b>0</b>	

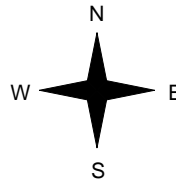


Heavys	0
Trucks	45
Cars	643
<b>Totals</b>	<b>688</b>

East Leg Total: 944  
 East Entering: 944  
 East Peds: 0  
 Peds Cross:  $\times$



James Snow Pkwy



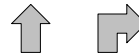
Cars	Trucks	Heavys	Totals
253	15	0	268
674	2	0	676
<b>927</b>	<b>17</b>	<b>0</b>	

Hwy 401 North Ramp Terminal



Cars	Trucks	Heavys	Totals
0	0	0	0

James Snow Pkwy



Cars	1536	Cars	390	0	390
Trucks	28	Trucks	30	0	30
Heavys	0	Heavys	0	0	0
<b>Totals</b>	<b>1564</b>	<b>Totals</b>	<b>420</b>	<b>0</b>	



Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 420  
 South Leg Total: 1984

## Comments



# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400003  
**Intersection:** James Snow Pkwy & Hwy 401 North  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

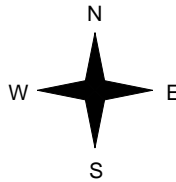
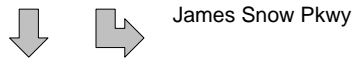
**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 9585  
 North Entering: 4417  
 North Peds: 0  
 Peds Cross:  $\times$

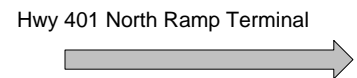
Heavys	0	0	0
Trucks	241	0	241
Cars	4176	0	4176
<b>Totals</b>	<b>4417</b>	<b>0</b>	

Heavys	0
Trucks	536
Cars	4632
<b>Totals</b>	<b>5168</b>

East Leg Total: 5685  
 East Entering: 5685  
 East Peds: 0  
 Peds Cross:  $\times$



	Cars	Trucks	Heavys	Totals
	1920	215	0	2135
	3501	49	0	3550
	5421	264	0	



	Cars	Trucks	Heavys	Totals
	0	0	0	0

	Cars	7677		Cars	2712	0	2712
	Trucks	290		Trucks	321	0	321
	Heavys	0		Heavys	0	0	0
	<b>Totals</b>	<b>7967</b>		<b>Totals</b>	<b>3033</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 3033  
 South Leg Total: 11000

### Comments

# Ontario Traffic Inc Traffic Count Summary

Intersection: James Snow Pkwy & Hwy 401 No      Count Date: 4-May-17      Municipality: Milton

North Approach Totals						North/South Total Approaches	South Approach Totals					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	262	0	262	0	482	7:00:00	0	220	0	220	0
8:00:00	0	480	0	480	0	941	8:00:00	0	461	0	461	0
9:00:00	0	456	0	456	0	957	9:00:00	0	501	0	501	0
10:00:00	0	396	0	396	0	692	10:00:00	0	296	0	296	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	0	590	0	590	0	981	16:00:00	0	391	0	391	0
17:00:00	0	825	0	825	0	1212	17:00:00	0	387	0	387	0
18:00:00	0	842	0	842	0	1251	18:00:00	0	409	0	409	0
19:00:00	0	566	0	566	0	934	19:00:00	0	368	0	368	0
Totals:	0	4417	0	4417	0	7450		0	3033	0	3033	0
East Approach Totals						East/West Total Approaches	West Approach Totals					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	281	0	192	473	0	473	7:00:00	0	0	0	0	0
8:00:00	353	0	331	684	0	684	8:00:00	0	0	0	0	0
9:00:00	300	0	335	635	0	635	9:00:00	0	0	0	0	0
10:00:00	301	0	251	552	0	552	10:00:00	0	0	0	0	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	448	0	259	707	0	707	16:00:00	0	0	0	0	0
17:00:00	489	0	205	694	0	694	17:00:00	0	0	0	0	0
18:00:00	716	0	281	997	0	997	18:00:00	0	0	0	0	0
19:00:00	662	0	281	943	0	943	19:00:00	0	0	0	0	0
Totals:	3550	0	2135	5685	0	5685		0	0	0	0	0
Calculated Values for Traffic Crossing Major Street												
Hours Ending:	7:00	8:00	9:00	10:00		16:00	17:00	18:00	19:00			
Crossing Values:	281	353	300	301		448	489	716	662			









# Ontario Traffic Inc

## Morning Peak Diagram

### Specified Period

**From:** 6:00:00  
**To:** 10:00:00

### One Hour Peak

**From:** 7:30:00  
**To:** 8:30:00

**Municipality:** Milton  
**Site #:** 1711400004  
**Intersection:** James Snow Pkwy & Hwy 401 Sou  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 1337  
North Entering: 547  
North Peds: 0  
Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	17	17
Cars	0	530	530
<b>Totals</b>	<b>0</b>	<b>547</b>	



Heavys	0
Trucks	85
Cars	705
<b>Totals</b>	<b>790</b>

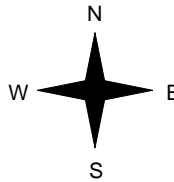
Heavys	Trucks	Cars	Totals
0	0	0	0



James Snow Pkwy



Hwy 401 South Ramp Terminal



Heavys	Trucks	Cars	Totals
0	44	122	166
0	22	221	243
0	66	343	



James Snow Pkwy

Peds Cross:  $\nabla$   
West Peds: 1  
West Entering: 409  
West Leg Total: 409

Cars	751
Trucks	39
Heavys	0
<b>Totals</b>	<b>790</b>



Cars	0	583	583
Trucks	0	41	41
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>624</b>	

Peds Cross:  $\nabla$   
South Peds: 0  
South Entering: 624  
South Leg Total: 1414

## Comments

# Ontario Traffic Inc

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 19:00:00

### One Hour Peak

**From:** 17:00:00

**To:** 18:00:00

**Municipality:** Milton  
**Site #:** 1711400004  
**Intersection:** James Snow Pkwy & Hwy 401 Sou  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 1925  
 North Entering: 1298  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	0	0	0
Trucks	0	7	7
Cars	0	1291	1291
<b>Totals</b>	<b>0</b>	<b>1298</b>	



Heavys	0
Trucks	37
Cars	590
<b>Totals</b>	<b>627</b>

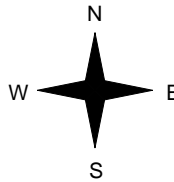
Heavys	Trucks	Cars	Totals
0	0	0	0



James Snow Pkwy



Hwy 401 South Ramp Terminal



Heavys	Trucks	Cars	Totals
0	24	41	65
0	7	202	209
0	31	243	



James Snow Pkwy



Peds Cross:  $\times$   
 West Peds: 1  
 West Entering: 274  
 West Leg Total: 274

Cars	1493
Trucks	14
Heavys	0
<b>Totals</b>	<b>1507</b>



Cars	0	549	549
Trucks	0	13	13
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>562</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 562  
 South Leg Total: 2069

## Comments



# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400004  
**Intersection:** James Snow Pkwy & Hwy 401 Sou  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 10569  
 North Entering: 6161  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	119	119
Cars	0	6042	6042
Totals	0	6161	



Heavys	0
Trucks	465
Cars	3943
Totals	4408

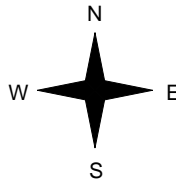
Heavys	Trucks	Cars	Totals
0	0	0	0



James Snow Pkwy



Hwy 401 South Ramp Terminal



Heavys	Trucks	Cars	Totals
0	233	449	682
0	124	1215	1339
0	357	1664	



James Snow Pkwy



Peds Cross:  $\nabla$   
 West Peds: 4  
 West Entering: 2021  
 West Leg Total: 2021

Cars	7257
Trucks	243
Heavys	0
Totals	7500



Cars	0	3494	3494
Trucks	0	232	232
Heavys	0	0	0
Totals	0	3726	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 3726  
 South Leg Total: 11226

### Comments

# Ontario Traffic Inc Traffic Count Summary

Intersection: James Snow Pkwy & Hwy 401 So | Count Date: 4-May-17 | Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	353	0	353	0	650	7:00:00	0	297	0	297	0
8:00:00	0	549	0	549	0	1125	8:00:00	0	576	0	576	0
9:00:00	0	522	0	522	0	1080	9:00:00	0	558	0	558	0
10:00:00	0	518	0	518	0	864	10:00:00	0	346	0	346	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	0	827	0	827	0	1285	16:00:00	0	458	0	458	0
17:00:00	0	1035	0	1035	0	1542	17:00:00	0	507	0	507	0
18:00:00	0	1298	0	1298	0	1860	18:00:00	0	562	0	562	0
19:00:00	0	1059	0	1059	0	1481	19:00:00	0	422	0	422	0
Totals:	0	6161	0	6161	0	9887		0	3726	0	3726	0
<b>East Approach Totals</b>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	0	0	0	0	215	7:00:00	69	0	146	215	0
8:00:00	0	0	0	0	0	309	8:00:00	112	0	197	309	2
9:00:00	0	0	0	0	0	389	9:00:00	169	0	220	389	1
10:00:00	0	0	0	0	0	164	10:00:00	64	0	100	164	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	0	0	0	0	0	236	16:00:00	96	0	140	236	0
17:00:00	0	0	0	0	0	224	17:00:00	55	0	169	224	0
18:00:00	0	0	0	0	0	274	18:00:00	65	0	209	274	1
19:00:00	0	0	0	0	0	210	19:00:00	52	0	158	210	0
Totals:	0	0	0	0	0	2021		682	0	1339	2021	4
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	7:00	8:00	9:00	10:00			16:00	17:00	18:00	19:00		
Crossing Values:	69	112	169	64			96	55	65	52		









# Eighth Line @ 5 Side Road

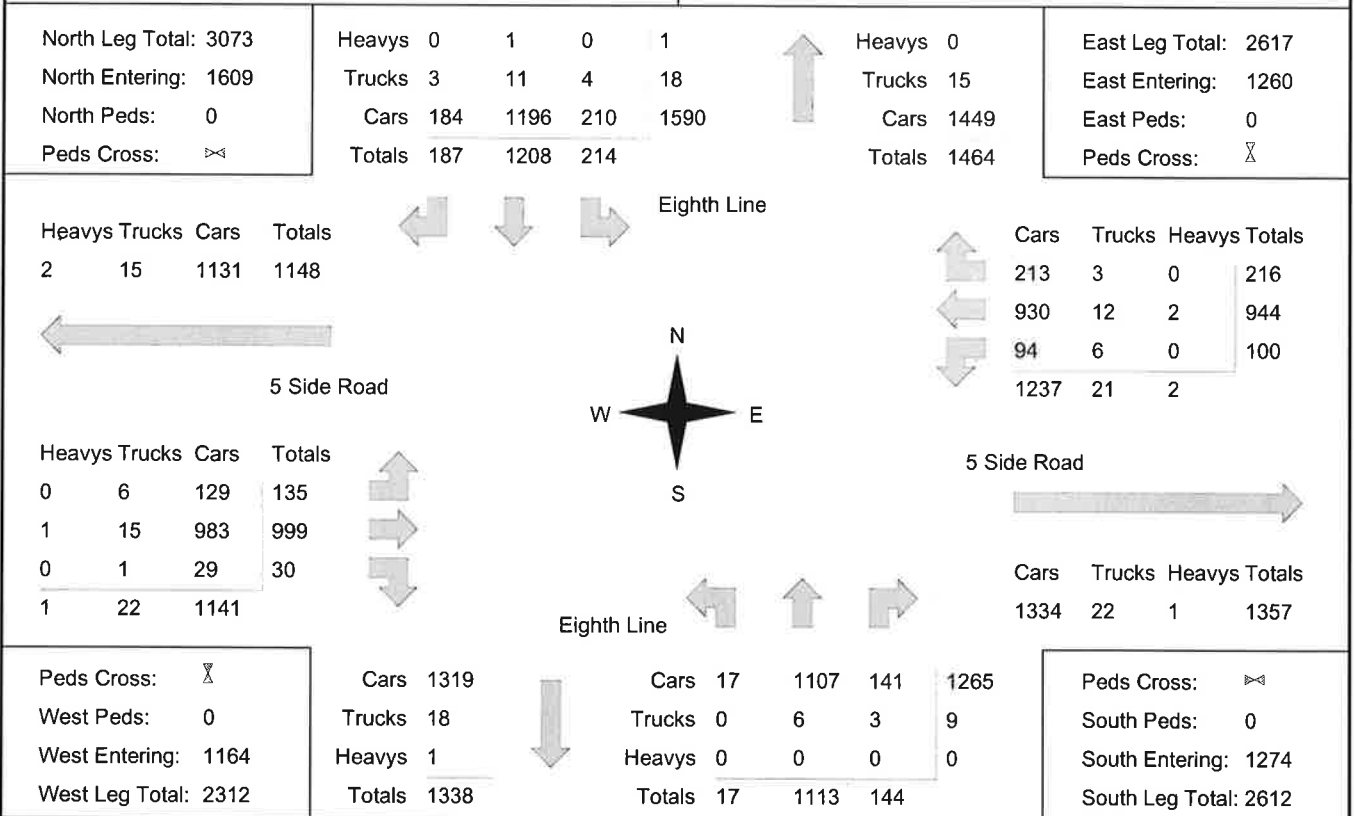
## Total Count Diagram

**Municipality:** Town of Halton Hills  
**Site #:** 0000000177  
**Intersection:** Eighth Line & 5 Side Road  
**TFR File #:** 16  
**Count date:** 23-Sep-14

**Weather conditions:**  
 Sunny / Dry  
**Person(s) who counted:**  
 Roumen Kotev

**\*\* Non-Signalized Intersection \*\***

**Major Road:** Eighth Line runs N/S



### Comments





# Eighth Line @ 5 Side Road

## Morning Peak Diagram

### Specified Period

**From:** 7:00:00  
**To:** 9:00:00

### One Hour Peak

**From:** 7:30:00  
**To:** 8:30:00

**Municipality:** Town of Halton Hills  
**Site #:** 0000000177  
**Intersection:** Eighth Line & 5 Side Road  
**TFR File #:** 16  
**Count date:** 23-Sep-14

### Weather conditions:

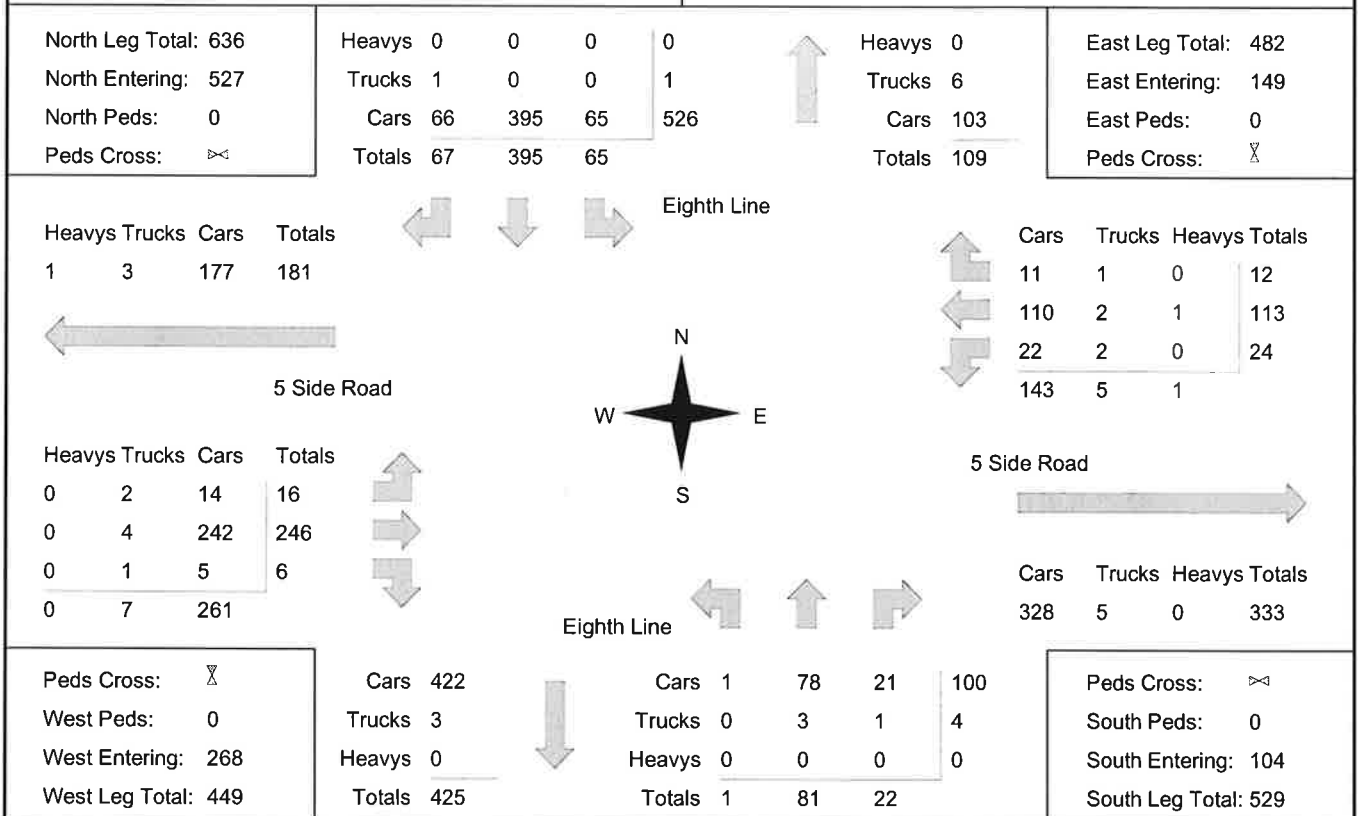
Sunny / Dry

### Person(s) who counted:

Roumen Kotev

### \*\* Non-Signalized Intersection \*\*

**Major Road:** Eighth Line runs N/S



## Comments



# Eighth Line @ 5 Side Road

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00  
**To:** 14:00:00

### One Hour Peak

**From:** 11:45:00  
**To:** 12:45:00

**Municipality:** Town of Halton Hills  
**Site #:** 0000000177  
**Intersection:** Eighth Line & 5 Side Road  
**TFR File #:** 16  
**Count date:** 23-Sep-14

### Weather conditions:

Sunny / Dry

### Person(s) who counted:

Roumen Kotev

### \*\* Non-Signalized Intersection \*\*

**Major Road:** Eighth Line runs N/S

North Leg Total: 207  
North Entering: 108  
North Peds: 0  
Peds Cross: ∅

Heavys	0	0	0	0
Trucks	0	2	1	3
Cars	14	76	15	105
Totals	14	78	16	

Heavys 0  
Trucks 1  
Cars 98  
Totals 99

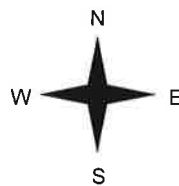
East Leg Total: 136  
East Entering: 70  
East Peds: 0  
Peds Cross: ∅

Heavys	0
Trucks	2
Cars	65
Totals	67



Cars	9	0	0	9
Trucks	50	2	0	52
Heavys	7	2	0	9
Totals	66	4	0	

Heavys	0
Trucks	0
Cars	9
Totals	9
Heavys	0
Trucks	0
Cars	37
Totals	37
Heavys	0
Trucks	0
Cars	4
Totals	4
Heavys	0
Trucks	0
Cars	50
Totals	



5 Side Road



Eighth Line



Cars	65	1	0	66
Trucks				
Heavys				
Totals				

Peds Cross: ∅  
West Peds: 0  
West Entering: 50  
West Leg Total: 117

Cars	87
Trucks	4
Heavys	0
Totals	91



Cars	1	80	13	94
Trucks	0	1	0	1
Heavys	0	0	0	0
Totals	1	81	13	

Peds Cross: ∅  
South Peds: 0  
South Entering: 95  
South Leg Total: 186

## Comments



# Eighth Line @ 5 Side Road

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00  
**To:** 18:00:00

### One Hour Peak

**From:** 16:45:00  
**To:** 17:45:00

**Municipality:** Town of Halton Hills  
**Site #:** 0000000177  
**Intersection:** Eighth Line & 5 Side Road  
**TFR File #:** 16  
**Count date:** 23-Sep-14

### Weather conditions:

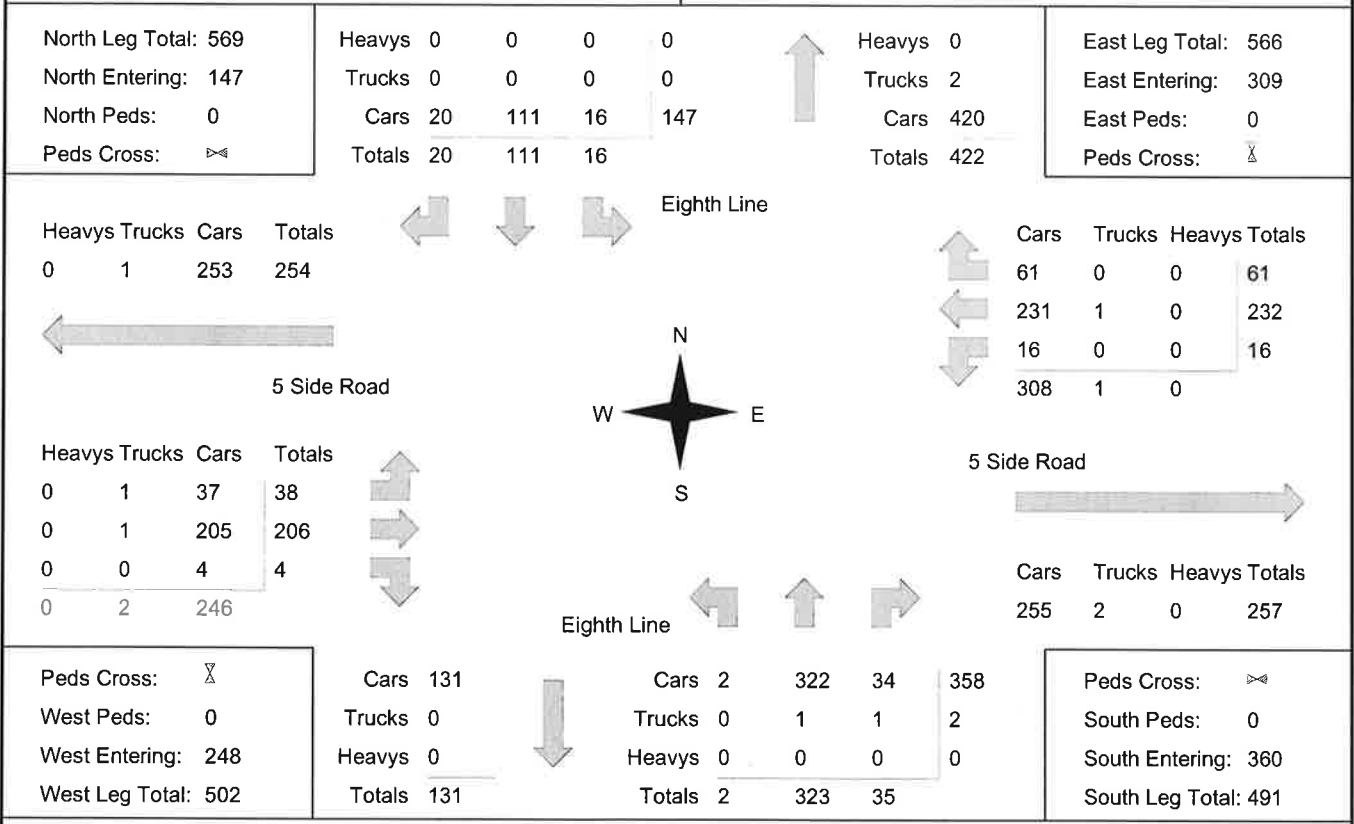
Sunny / Dry

### Person(s) who counted:

Roumen Kotev

### \*\* Non-Signalized Intersection \*\*

**Major Road:** Eighth Line runs N/S



## Comments



# Eighth Line @ 5 Side Road Traffic Count Summary

Intersection: Eighth Line & 5 Side Road					Count Date: 23-Sep-14		Municipality: Town of Halton Hills					
North Approach Totals						South Approach Totals						
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds	North/South Total Approaches	Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
8:00:00	79	389	55	523	0	599	8:00:00	1	60	15	76	0
9:00:00	49	289	40	378	0	472	9:00:00	4	73	17	94	0
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	13	80	13	106	0	175	12:00:00	1	58	10	69	0
13:00:00	15	78	12	105	0	190	13:00:00	1	74	10	85	0
14:00:00	13	70	10	93	0	191	14:00:00	0	93	5	98	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	15	98	22	135	0	329	16:00:00	2	172	20	194	0
17:00:00	15	106	17	138	0	413	17:00:00	6	239	30	275	0
18:00:00	15	98	18	131	0	514	18:00:00	2	344	37	383	0
<b>Totals:</b>	214	1208	187	1609	0	2883		17	1113	144	1274	0
East Approach Totals						West Approach Totals						
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds	East/West Total Approaches	Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
8:00:00	22	96	7	125	0	355	8:00:00	11	218	1	230	0
9:00:00	14	97	8	119	0	347	9:00:00	13	205	10	228	0
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	9	49	20	78	0	115	12:00:00	6	26	5	37	0
13:00:00	6	53	8	67	0	115	13:00:00	8	37	3	48	0
14:00:00	9	45	12	66	0	123	14:00:00	6	50	1	57	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	10	153	41	204	0	340	16:00:00	21	111	4	136	0
17:00:00	11	205	61	277	0	487	17:00:00	31	176	3	210	0
18:00:00	19	246	59	324	0	542	18:00:00	39	176	3	218	0
<b>Totals:</b>	100	944	216	1260	0	2424		135	999	30	1164	0
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	8:00	9:00	12:00	13:00		14:00	16:00	17:00	18:00			
Crossing Values:	251	232	64	67		65	184	247	304			







Paradigm Transportation Solutions Limited  
22 King Street South, Suite 300

Waterloo, Ontario, Canada N2J 1N8  
519-896-3163 cbowness@ptsl.com

Count Name: 5 Side Road & Fifth Line  
Site Code:  
Start Date: 12/13/2016  
Page No: 1

### Turning Movement Data

Start Time	5 Side Road Eastbound						5 Side Road Westbound						Fifth Line Road Northbound						Fifth Line Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:00 AM	2	60	3	0	0	65	4	33	0	0	0	37	0	2	3	0	0	5	7	8	2	0	0	17	124
7:15 AM	1	106	4	0	0	111	4	40	0	0	0	44	1	4	5	0	0	10	5	10	7	0	0	22	187
7:30 AM	1	129	1	0	0	131	7	46	0	0	0	53	0	6	3	0	0	9	13	10	10	0	0	33	226
7:45 AM	5	129	4	0	0	138	9	45	1	0	0	55	1	1	4	0	0	6	8	14	8	0	0	30	229
Hourly Total	9	424	12	0	0	445	24	164	1	0	0	189	2	13	15	0	0	30	33	42	27	0	0	102	766
8:00 AM	1	165	3	0	0	169	2	27	1	0	0	30	2	8	5	0	0	15	12	15	2	0	0	29	243
8:15 AM	3	140	3	0	0	146	4	42	1	0	0	47	1	5	7	0	0	13	3	7	5	0	0	15	221
8:30 AM	4	104	2	0	0	110	8	29	2	0	0	39	2	2	7	0	0	11	2	4	8	0	0	14	174
8:45 AM	2	69	2	0	0	73	3	26	1	0	0	30	1	2	4	0	0	7	2	6	4	0	0	12	122
Hourly Total	10	478	10	0	0	498	17	124	5	0	0	146	6	17	23	0	0	46	19	32	19	0	0	70	760
9:00 AM	5	39	1	0	0	45	1	26	2	0	0	29	0	1	3	0	0	4	1	2	2	0	0	5	83
9:15 AM	1	34	0	0	0	35	2	15	1	0	0	18	0	1	4	0	0	5	4	2	2	0	0	8	66
9:30 AM	0	21	0	0	0	21	0	21	0	0	0	21	2	4	1	0	0	7	3	1	6	0	0	10	59
9:45 AM	1	18	0	0	0	19	0	11	0	0	0	11	1	0	1	0	0	2	0	2	2	0	0	4	36
Hourly Total	7	112	1	0	0	120	3	73	3	0	0	79	3	6	9	0	0	18	8	7	12	0	0	27	244
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	2	19	1	0	0	22	1	28	0	0	0	29	0	1	2	0	0	3	3	1	1	0	0	5	59
11:15 AM	1	14	0	0	0	15	3	14	0	0	0	17	1	1	2	0	0	4	1	3	3	0	0	7	43
11:30 AM	1	9	0	0	0	10	1	25	0	0	0	26	0	2	4	0	0	6	0	3	3	0	0	6	48
11:45 AM	1	14	0	0	0	15	1	19	0	0	0	20	0	5	2	0	0	7	1	1	2	0	0	4	46
Hourly Total	5	56	1	0	0	62	6	86	0	0	0	92	1	9	10	0	0	20	5	8	9	0	0	22	196
12:00 PM	1	13	1	0	0	15	3	10	0	0	0	13	1	1	2	0	0	4	0	4	4	0	0	8	40
12:15 PM	1	17	3	0	0	21	0	23	0	0	0	23	0	1	2	0	0	3	1	2	3	0	0	6	53
12:30 PM	3	17	4	0	0	24	1	23	1	0	0	25	1	2	5	0	0	8	0	4	1	0	0	5	62
12:45 PM	1	16	1	0	0	18	0	19	1	0	0	20	0	3	2	0	0	5	0	3	1	0	0	4	47
Hourly Total	6	63	9	0	0	78	4	75	2	0	0	81	2	7	11	0	0	20	1	13	9	0	0	23	202
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	7	42	3	0	0	52	3	28	3	0	0	34	0	3	7	0	0	10	0	2	2	0	0	4	100
3:15 PM	2	28	1	0	0	31	3	49	2	0	0	54	1	3	3	0	0	7	0	2	6	0	0	8	100
3:30 PM	3	33	2	0	0	38	3	43	3	0	0	49	1	2	3	0	0	6	0	5	1	0	0	6	99
3:45 PM	2	51	2	0	0	55	1	69	5	0	0	75	0	4	3	0	0	7	1	5	2	0	0	8	145
Hourly Total	14	154	8	0	0	176	10	189	13	0	0	212	2	12	16	0	0	30	1	14	11	0	0	26	444
4:00 PM	10	49	0	0	0	59	3	83	4	0	0	90	3	15	7	0	0	25	0	8	5	0	0	13	187
4:15 PM	9	43	1	0	0	53	7	79	5	0	0	91	1	8	3	0	0	12	1	1	1	0	0	3	159
4:30 PM	8	59	0	0	0	67	4	84	1	0	0	89	3	18	2	0	0	23	0	8	5	0	0	13	192

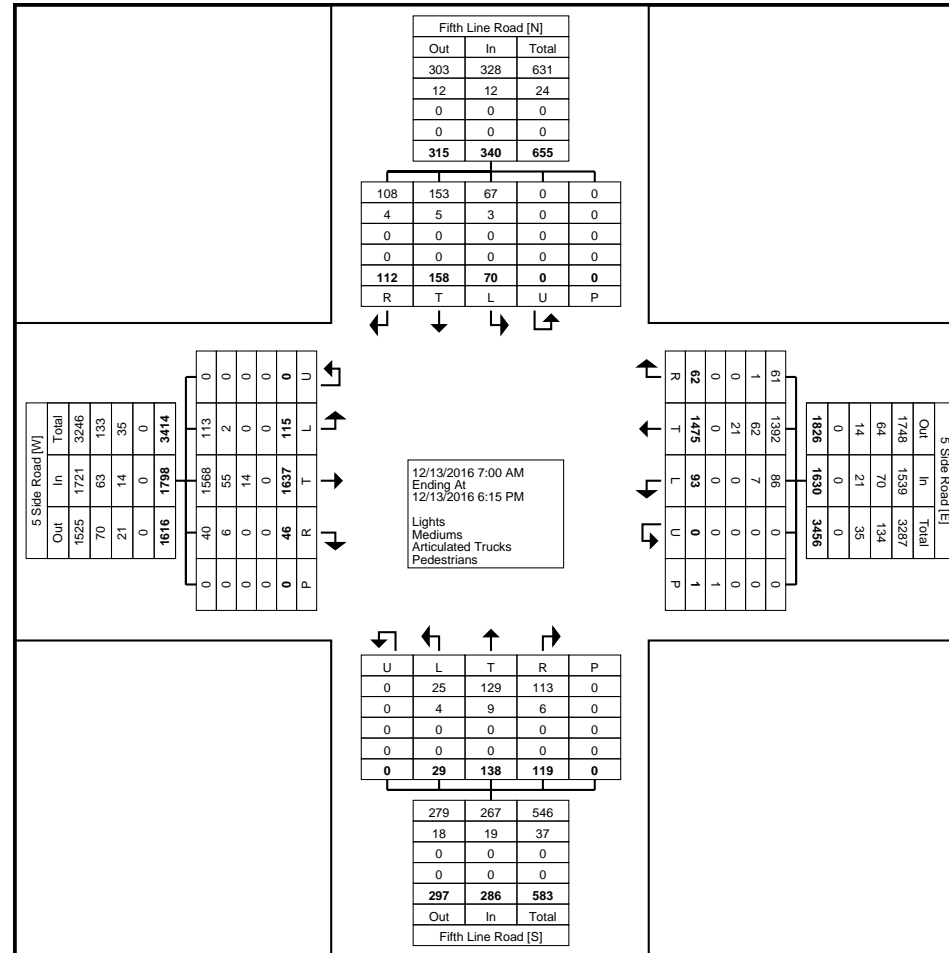




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Count Name: 5 Side Road & Fifth Line  
Site Code:  
Start Date: 12/13/2016  
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Turning Movement Data Plot



Paradigm Transportation Solutions Limited  
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Count Name: 5 Side Road & Fifth Line  
Site Code:  
Start Date: 12/13/2016  
Page No: 4

### Turning Movement Peak Hour Data (7:30 AM)

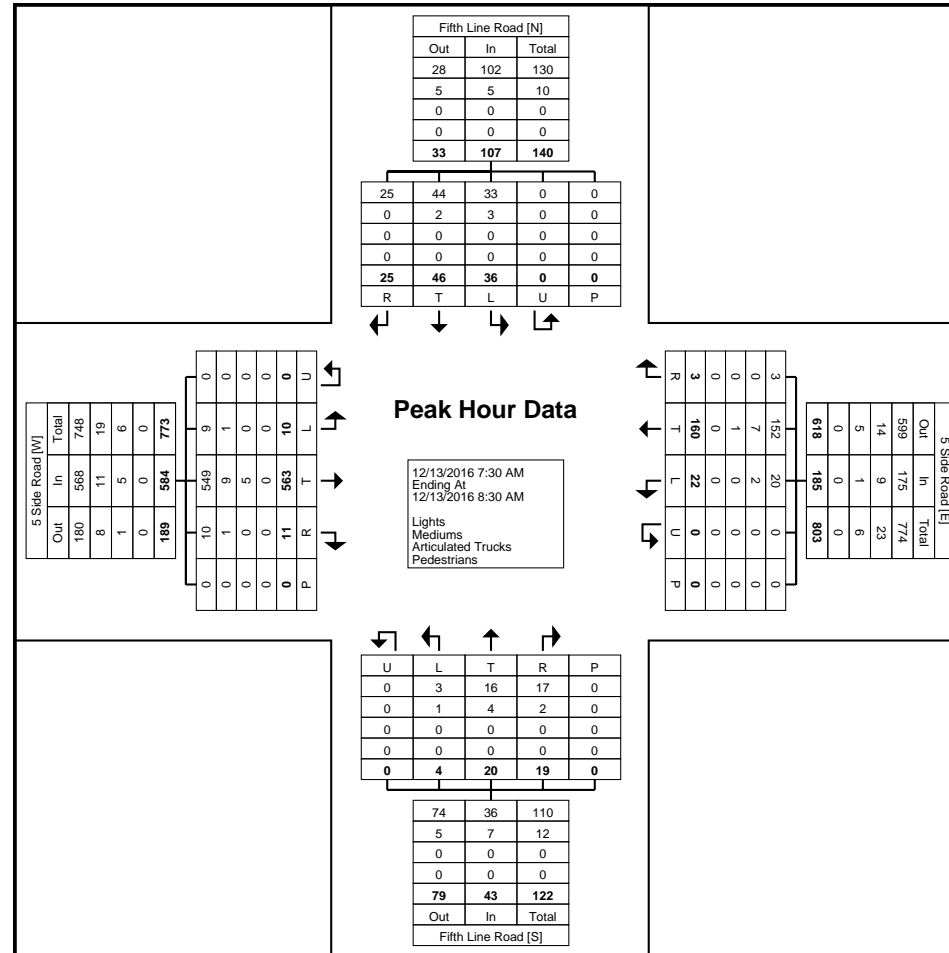
Start Time	5 Side Road Eastbound						5 Side Road Westbound						Fifth Line Road Northbound						Fifth Line Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	1	129	1	0	0	131	7	46	0	0	0	53	0	6	3	0	0	9	13	10	10	0	0	33	226
7:45 AM	5	129	4	0	0	138	9	45	1	0	0	55	1	1	4	0	0	6	8	14	8	0	0	30	229
8:00 AM	1	165	3	0	0	169	2	27	1	0	0	30	2	8	5	0	0	15	12	15	2	0	0	29	243
8:15 AM	3	140	3	0	0	146	4	42	1	0	0	47	1	5	7	0	0	13	3	7	5	0	0	15	221
Total	10	563	11	0	0	584	22	160	3	0	0	185	4	20	19	0	0	43	36	46	25	0	0	107	919
Approach %	1.7	96.4	1.9	0.0	-	-	11.9	86.5	1.6	0.0	-	-	9.3	46.5	44.2	0.0	-	-	33.6	43.0	23.4	0.0	-	-	-
Total %	1.1	61.3	1.2	0.0	-	63.5	2.4	17.4	0.3	0.0	-	20.1	0.4	2.2	2.1	0.0	-	4.7	3.9	5.0	2.7	0.0	-	11.6	-
PHF	0.500	0.853	0.688	0.000	-	0.864	0.611	0.870	0.750	0.000	-	0.841	0.500	0.625	0.679	0.000	-	0.717	0.692	0.767	0.625	0.000	-	0.811	0.945
Lights	9	549	10	0	-	568	20	152	3	0	-	175	3	16	17	0	-	36	33	44	25	0	-	102	881
% Lights	90.0	97.5	90.9	-	-	97.3	90.9	95.0	100.0	-	-	94.6	75.0	80.0	89.5	-	-	83.7	91.7	95.7	100.0	-	-	95.3	95.9
Mediums	1	9	1	0	-	11	2	7	0	0	-	9	1	4	2	0	-	7	3	2	0	0	-	5	32
% Mediums	10.0	1.6	9.1	-	-	1.9	9.1	4.4	0.0	-	-	4.9	25.0	20.0	10.5	-	-	16.3	8.3	4.3	0.0	-	-	4.7	3.5
Articulated Trucks	0	5	0	0	-	5	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	6
% Articulated Trucks	0.0	0.9	0.0	-	-	0.9	0.0	0.6	0.0	-	-	0.5	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.7
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: 5 Side Road & Fifth Line  
Site Code:  
Start Date: 12/13/2016  
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Turning Movement Peak Hour Data Plot (7:30 AM)



Paradigm Transportation Solutions Limited  
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Count Name: 5 Side Road & Fifth Line  
Site Code:  
Start Date: 12/13/2016  
Page No: 6

### Turning Movement Peak Hour Data (11:00 AM)

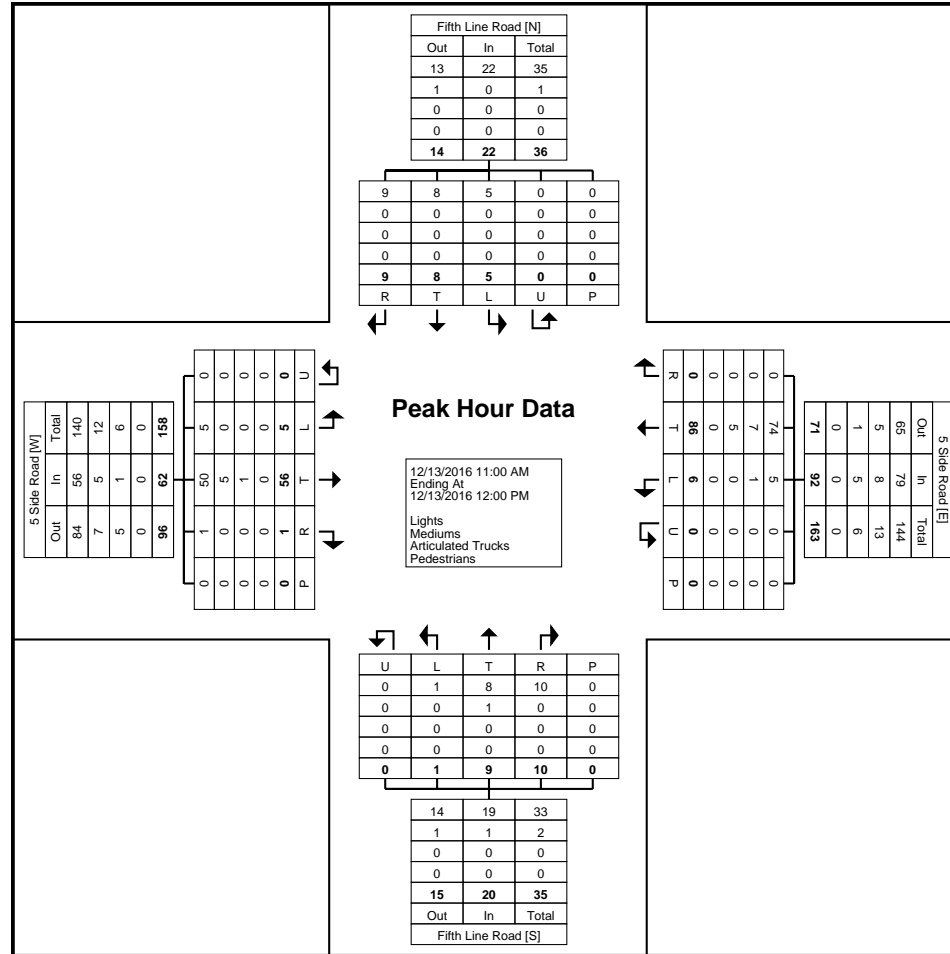
Start Time	5 Side Road Eastbound						5 Side Road Westbound						Fifth Line Road Northbound						Fifth Line Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
11:00 AM	2	19	1	0	0	22	1	28	0	0	0	29	0	1	2	0	0	3	3	1	1	0	0	5	59
11:15 AM	1	14	0	0	0	15	3	14	0	0	0	17	1	1	2	0	0	4	1	3	3	0	0	7	43
11:30 AM	1	9	0	0	0	10	1	25	0	0	0	26	0	2	4	0	0	6	0	3	3	0	0	6	48
11:45 AM	1	14	0	0	0	15	1	19	0	0	0	20	0	5	2	0	0	7	1	1	2	0	0	4	46
Total	5	56	1	0	0	62	6	86	0	0	0	92	1	9	10	0	0	20	5	8	9	0	0	22	196
Approach %	8.1	90.3	1.6	0.0	-	-	6.5	93.5	0.0	0.0	-	-	5.0	45.0	50.0	0.0	-	-	22.7	36.4	40.9	0.0	-	-	-
Total %	2.6	28.6	0.5	0.0	-	31.6	3.1	43.9	0.0	0.0	-	46.9	0.5	4.6	5.1	0.0	-	10.2	2.6	4.1	4.6	0.0	-	11.2	-
PHF	0.625	0.737	0.250	0.000	-	0.705	0.500	0.768	0.000	0.000	-	0.793	0.250	0.450	0.625	0.000	-	0.714	0.417	0.667	0.750	0.000	-	0.786	0.831
Lights	5	50	1	0	-	56	5	74	0	0	-	79	1	8	10	0	-	19	5	8	9	0	-	22	176
% Lights	100.0	89.3	100.0	-	-	90.3	83.3	86.0	-	-	-	85.9	100.0	88.9	100.0	-	-	95.0	100.0	100.0	100.0	-	-	100.0	89.8
Mediums	0	5	0	0	-	5	1	7	0	0	-	8	0	1	0	0	-	1	0	0	0	0	-	0	14
% Mediums	0.0	8.9	0.0	-	-	8.1	16.7	8.1	-	-	-	8.7	0.0	11.1	0.0	-	-	5.0	0.0	0.0	0.0	-	-	0.0	7.1
Articulated Trucks	0	1	0	0	-	1	0	5	0	0	-	5	0	0	0	0	-	0	0	0	0	0	-	0	6
% Articulated Trucks	0.0	1.8	0.0	-	-	1.6	0.0	5.8	-	-	-	5.4	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	3.1
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: 5 Side Road & Fifth Line  
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Start Date: 12/13/2016  
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Turning Movement Peak Hour Data Plot (11:00 AM)



Paradigm Transportation Solutions Limited  
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Count Name: 5 Side Road & Fifth Line  
Site Code:  
Start Date: 12/13/2016  
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### Turning Movement Peak Hour Data (12:00 PM)

Start Time	5 Side Road Eastbound						5 Side Road Westbound						Fifth Line Road Northbound						Fifth Line Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
12:00 PM	1	13	1	0	0	15	3	10	0	0	0	13	1	1	2	0	0	4	0	4	4	0	0	8	40
12:15 PM	1	17	3	0	0	21	0	23	0	0	0	23	0	1	2	0	0	3	1	2	3	0	0	6	53
12:30 PM	3	17	4	0	0	24	1	23	1	0	0	25	1	2	5	0	0	8	0	4	1	0	0	5	62
12:45 PM	1	16	1	0	0	18	0	19	1	0	0	20	0	3	2	0	0	5	0	3	1	0	0	4	47
Total	6	63	9	0	0	78	4	75	2	0	0	81	2	7	11	0	0	20	1	13	9	0	0	23	202
Approach %	7.7	80.8	11.5	0.0	-	-	4.9	92.6	2.5	0.0	-	-	10.0	35.0	55.0	0.0	-	-	4.3	56.5	39.1	0.0	-	-	-
Total %	3.0	31.2	4.5	0.0	-	38.6	2.0	37.1	1.0	0.0	-	40.1	1.0	3.5	5.4	0.0	-	9.9	0.5	6.4	4.5	0.0	-	11.4	-
PHF	0.500	0.926	0.563	0.000	-	0.813	0.333	0.815	0.500	0.000	-	0.810	0.500	0.583	0.550	0.000	-	0.625	0.250	0.813	0.563	0.000	-	0.719	0.815
Lights	5	57	8	0	-	70	4	59	2	0	-	65	2	7	10	0	-	19	1	13	9	0	-	23	177
% Lights	83.3	90.5	88.9	-	-	89.7	100.0	78.7	100.0	-	-	80.2	100.0	100.0	90.9	-	-	95.0	100.0	100.0	100.0	-	-	100.0	87.6
Mediums	1	6	1	0	-	8	0	14	0	0	-	14	0	0	1	0	-	1	0	0	0	0	-	0	23
% Mediums	16.7	9.5	11.1	-	-	10.3	0.0	18.7	0.0	-	-	17.3	0.0	0.0	9.1	-	-	5.0	0.0	0.0	0.0	-	-	0.0	11.4
Articulated Trucks	0	0	0	0	-	0	0	2	0	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	2
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	2.7	0.0	-	-	2.5	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	1.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

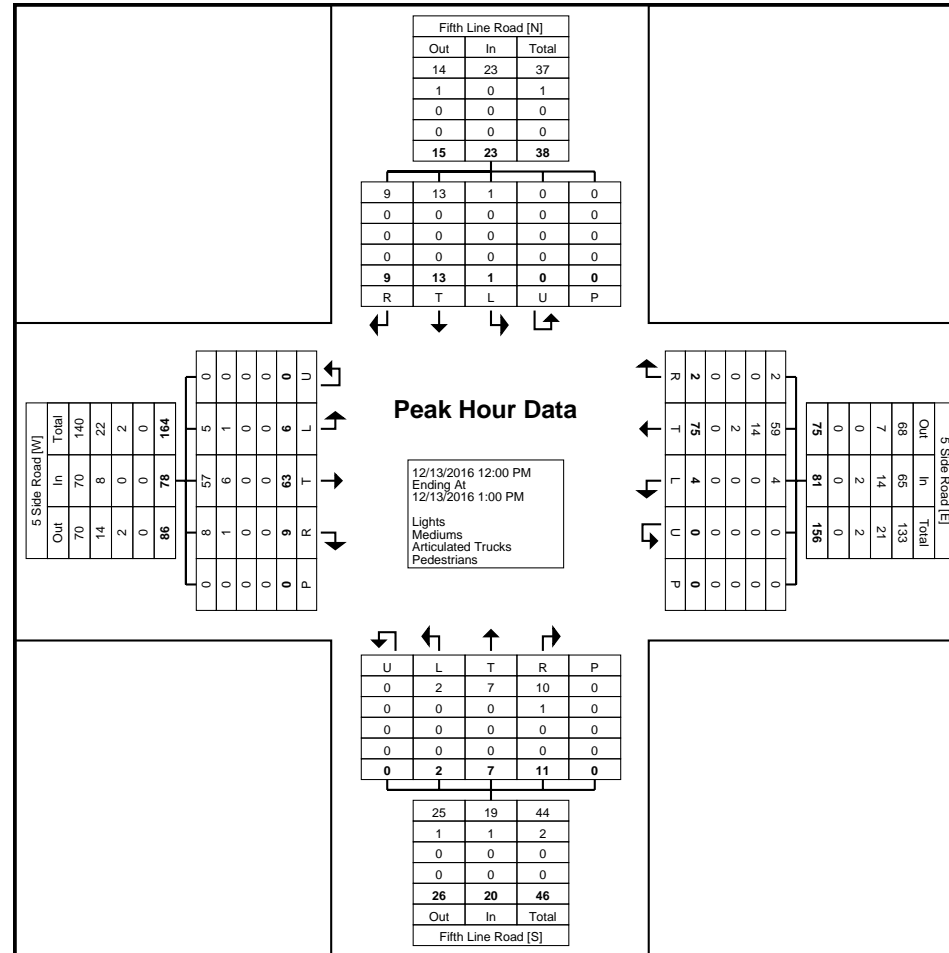




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Count Name: 5 Side Road & Fifth Line  
Site Code:  
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Turning Movement Peak Hour Data Plot (12:00 PM)



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Count Name: 5 Side Road & Fifth Line  
Site Code:  
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### Turning Movement Peak Hour Data (4:30 PM)

Start Time	5 Side Road Eastbound						5 Side Road Westbound						Fifth Line Road Northbound						Fifth Line Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
4:30 PM	8	59	0	0	0	67	4	84	1	0	0	89	3	18	2	0	0	23	0	8	5	0	0	13	192
4:45 PM	8	37	1	0	0	46	6	84	7	0	0	97	1	7	9	0	0	17	0	6	2	0	0	8	168
5:00 PM	9	60	1	0	0	70	2	112	8	0	0	122	1	6	5	0	0	12	0	5	4	0	0	9	213
5:15 PM	9	35	2	0	0	46	2	125	3	0	0	130	1	7	5	0	0	13	1	7	1	0	0	9	198
Total	34	191	4	0	0	229	14	405	19	0	0	438	6	38	21	0	0	65	1	26	12	0	0	39	771
Approach %	14.8	83.4	1.7	0.0	-	-	3.2	92.5	4.3	0.0	-	-	9.2	58.5	32.3	0.0	-	-	2.6	66.7	30.8	0.0	-	-	-
Total %	4.4	24.8	0.5	0.0	-	29.7	1.8	52.5	2.5	0.0	-	56.8	0.8	4.9	2.7	0.0	-	8.4	0.1	3.4	1.6	0.0	-	5.1	-
PHF	0.944	0.796	0.500	0.000	-	0.818	0.583	0.810	0.594	0.000	-	0.842	0.500	0.528	0.583	0.000	-	0.707	0.250	0.813	0.600	0.000	-	0.750	0.905
Lights	34	187	4	0	-	225	14	393	19	0	-	426	5	38	20	0	-	63	1	26	10	0	-	37	751
% Lights	100.0	97.9	100.0	-	-	98.3	100.0	97.0	100.0	-	-	97.3	83.3	100.0	95.2	-	-	96.9	100.0	100.0	83.3	-	-	94.9	97.4
Mediums	0	4	0	0	-	4	0	9	0	0	-	9	1	0	1	0	-	2	0	0	2	0	-	2	17
% Mediums	0.0	2.1	0.0	-	-	1.7	0.0	2.2	0.0	-	-	2.1	16.7	0.0	4.8	-	-	3.1	0.0	0.0	16.7	-	-	5.1	2.2
Articulated Trucks	0	0	0	0	-	0	0	3	0	0	-	3	0	0	0	0	-	0	0	0	0	0	-	0	3
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.7	0.0	-	-	0.7	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.4
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-





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Count Name: 5 Side Road & Fifth Line  
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Start Date: 12/13/2016  
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Paradigm Transportation Solutions Limited  
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Count Name: 5 Side Road & Sixth Line  
Site Code:  
Start Date: 12/13/2016  
Page No: 1

### Turning Movement Data

Start Time	5 Side Road Eastbound						5 Side Road Westbound						Sixth Line Northbound						Sixth Line Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:00 AM	1	66	1	0	0	68	4	34	0	0	0	38	0	1	2	0	0	3	4	0	3	0	0	7	116
7:15 AM	1	106	0	0	0	107	0	36	0	0	0	36	0	5	5	0	0	10	2	0	4	0	0	6	159
7:30 AM	1	128	0	0	0	129	0	49	1	0	0	50	0	3	5	0	0	8	4	2	5	0	0	11	198
7:45 AM	4	130	0	0	0	134	0	50	0	0	0	50	1	4	6	0	0	11	6	7	6	0	0	19	214
Hourly Total	7	430	1	0	0	438	4	169	1	0	0	174	1	13	18	0	0	32	16	9	18	0	0	43	687
8:00 AM	2	172	0	0	0	174	2	32	1	0	0	35	0	0	11	0	0	11	6	3	1	0	0	10	230
8:15 AM	0	165	1	0	0	166	6	40	1	0	0	47	0	2	8	0	0	10	4	5	4	0	0	13	236
8:30 AM	2	117	1	0	0	120	4	29	0	0	0	33	1	1	7	0	0	9	5	3	5	0	0	13	175
8:45 AM	2	81	1	0	0	84	3	27	0	0	0	30	0	0	2	0	0	2	4	3	1	0	0	8	124
Hourly Total	6	535	3	0	0	544	15	128	2	0	0	145	1	3	28	0	0	32	19	14	11	0	0	44	765
9:00 AM	3	35	0	0	0	38	2	27	1	0	0	30	2	1	3	0	0	6	0	1	0	0	0	1	75
9:15 AM	1	47	0	0	0	48	1	17	0	0	0	18	1	0	4	0	0	5	0	2	0	0	0	2	73
9:30 AM	1	22	0	0	0	23	3	21	3	0	0	27	0	0	1	0	0	1	2	0	0	0	0	2	53
9:45 AM	2	15	0	0	0	17	0	12	1	0	0	13	0	1	2	0	0	3	1	1	0	0	0	2	35
Hourly Total	7	119	0	0	0	126	6	77	5	0	0	88	3	2	10	0	0	15	3	4	0	0	0	7	236
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 AM	3	19	0	0	0	22	0	22	1	0	0	23	2	1	1	0	0	4	4	1	2	0	0	7	56
11:15 AM	3	21	0	0	0	24	2	20	1	0	0	23	0	2	5	0	0	7	1	0	0	0	0	1	55
11:30 AM	1	8	0	0	0	9	0	22	0	0	0	22	1	3	3	0	0	7	3	0	7	0	0	10	48
11:45 AM	0	17	1	0	0	18	0	16	1	0	0	17	0	4	0	0	0	4	2	0	0	0	0	2	41
Hourly Total	7	65	1	0	0	73	2	80	3	0	0	85	3	10	9	0	0	22	10	1	9	0	0	20	200
12:00 PM	1	11	1	0	0	13	2	14	1	0	0	17	0	2	6	0	0	8	0	2	3	0	0	5	43
12:15 PM	1	16	0	0	0	17	3	19	2	0	0	24	1	2	3	0	0	6	2	1	2	0	0	5	52
12:30 PM	1	16	0	0	0	17	0	20	0	0	0	20	1	1	1	0	0	3	0	0	4	0	0	4	44
12:45 PM	0	19	0	0	0	19	1	23	2	0	0	26	1	2	2	0	0	5	0	1	0	0	0	1	51
Hourly Total	3	62	1	0	0	66	6	76	5	0	0	87	3	7	12	0	0	22	2	4	9	0	0	15	190
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	2	47	1	0	0	50	5	31	2	0	0	38	1	0	4	0	0	5	0	1	2	0	0	3	96
3:15 PM	1	30	2	0	0	33	8	43	1	0	0	52	0	7	2	0	0	9	1	6	8	0	0	15	109
3:30 PM	3	32	0	0	0	35	2	46	4	0	0	52	1	5	8	0	0	14	1	2	2	0	0	5	106
3:45 PM	0	61	1	0	0	62	3	65	1	0	0	69	5	3	2	0	0	10	1	3	3	0	0	7	148
Hourly Total	6	170	4	0	0	180	18	185	8	0	0	211	7	15	16	0	0	38	3	12	15	0	0	30	459
4:00 PM	1	46	2	0	0	49	1	83	2	0	0	86	1	2	5	0	0	8	0	4	4	0	0	8	151

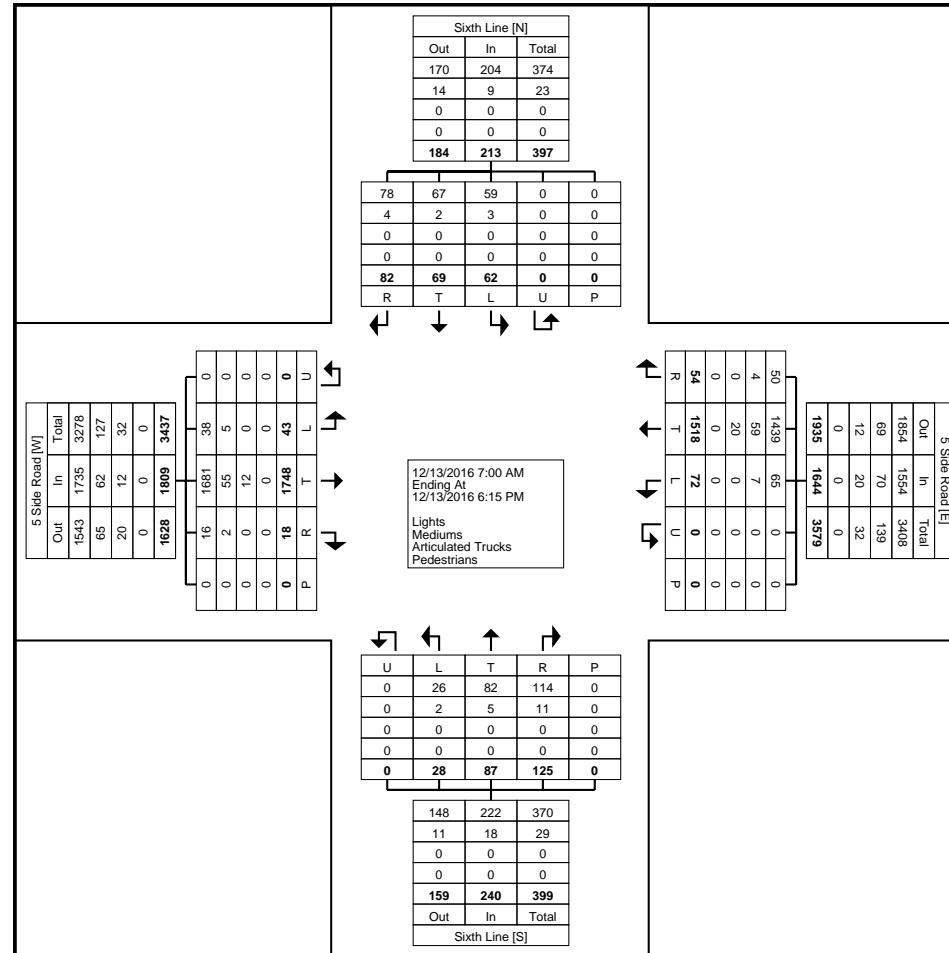




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Count Name: 5 Side Road & Sixth Line  
Site Code:  
Start Date: 12/13/2016  
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Turning Movement Data Plot



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Count Name: 5 Side Road & Sixth Line  
Site Code:  
Start Date: 12/13/2016  
Page No: 4

### Turning Movement Peak Hour Data (7:30 AM)

Start Time	5 Side Road Eastbound						5 Side Road Westbound						Sixth Line Northbound						Sixth Line Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	1	128	0	0	0	129	0	49	1	0	0	50	0	3	5	0	0	8	4	2	5	0	0	11	198
7:45 AM	4	130	0	0	0	134	0	50	0	0	0	50	1	4	6	0	0	11	6	7	6	0	0	19	214
8:00 AM	2	172	0	0	0	174	2	32	1	0	0	35	0	0	11	0	0	11	6	3	1	0	0	10	230
8:15 AM	0	165	1	0	0	166	6	40	1	0	0	47	0	2	8	0	0	10	4	5	4	0	0	13	236
Total	7	595	1	0	0	603	8	171	3	0	0	182	1	9	30	0	0	40	20	17	16	0	0	53	878
Approach %	1.2	98.7	0.2	0.0	-	-	4.4	94.0	1.6	0.0	-	-	2.5	22.5	75.0	0.0	-	-	37.7	32.1	30.2	0.0	-	-	-
Total %	0.8	67.8	0.1	0.0	-	68.7	0.9	19.5	0.3	0.0	-	20.7	0.1	1.0	3.4	0.0	-	4.6	2.3	1.9	1.8	0.0	-	6.0	-
PHF	0.438	0.865	0.250	0.000	-	0.866	0.333	0.855	0.750	0.000	-	0.910	0.250	0.563	0.682	0.000	-	0.909	0.833	0.607	0.667	0.000	-	0.697	0.930
Lights	6	580	0	0	-	586	8	162	2	0	-	172	1	9	25	0	-	35	18	17	16	0	-	51	844
% Lights	85.7	97.5	0.0	-	-	97.2	100.0	94.7	66.7	-	-	94.5	100.0	100.0	83.3	-	-	87.5	90.0	100.0	100.0	-	-	96.2	96.1
Mediums	1	11	1	0	-	13	0	8	1	0	-	9	0	0	5	0	-	5	2	0	0	0	-	2	29
% Mediums	14.3	1.8	100.0	-	-	2.2	0.0	4.7	33.3	-	-	4.9	0.0	0.0	16.7	-	-	12.5	10.0	0.0	0.0	-	-	3.8	3.3
Articulated Trucks	0	4	0	0	-	4	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	5
% Articulated Trucks	0.0	0.7	0.0	-	-	0.7	0.0	0.6	0.0	-	-	0.5	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.6
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

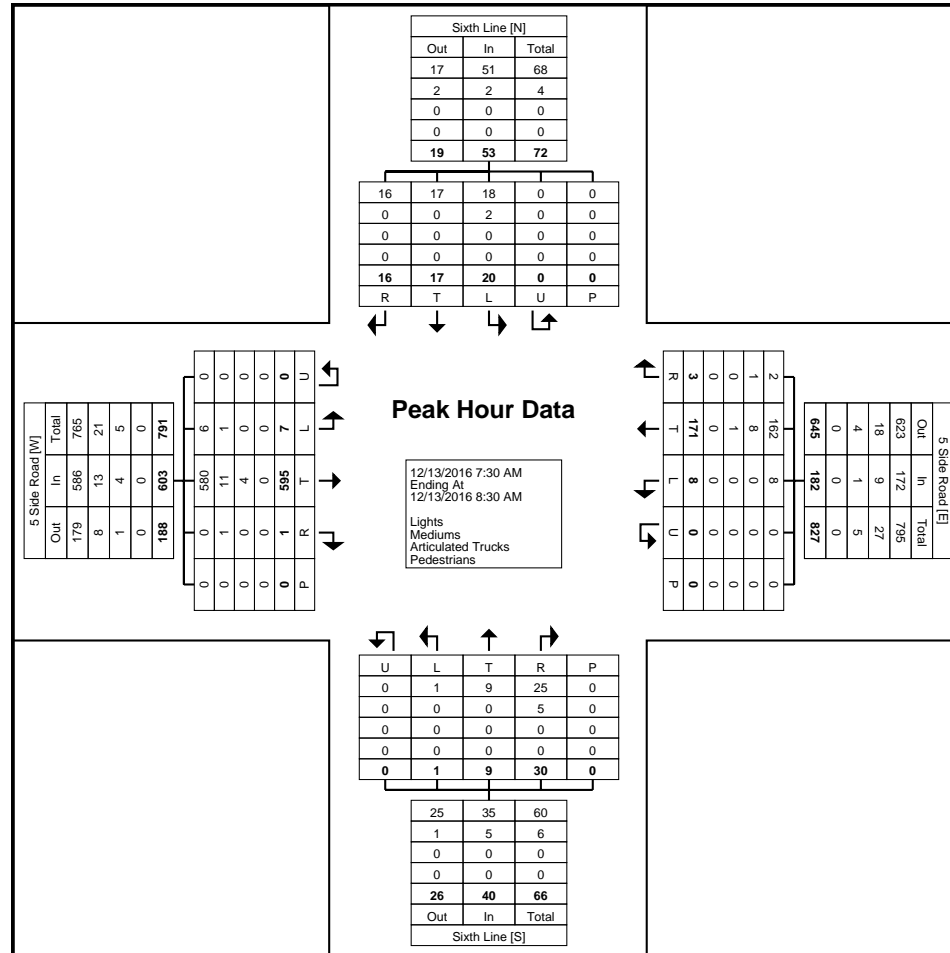




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Count Name: 5 Side Road & Sixth Line  
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Turning Movement Peak Hour Data Plot (7:30 AM)



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Count Name: 5 Side Road & Sixth Line  
Site Code:  
Start Date: 12/13/2016  
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### Turning Movement Peak Hour Data (11:00 AM)

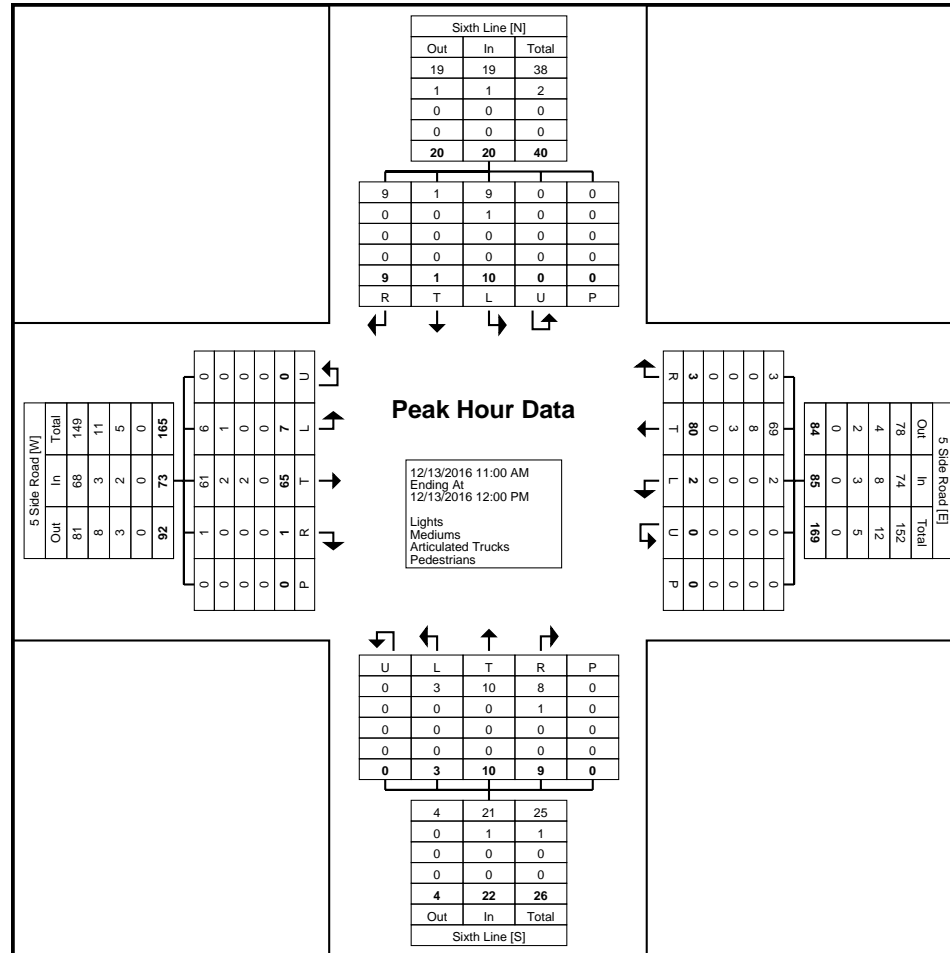
Start Time	5 Side Road Eastbound						5 Side Road Westbound						Sixth Line Northbound						Sixth Line Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
11:00 AM	3	19	0	0	0	22	0	22	1	0	0	23	2	1	1	0	0	4	4	1	2	0	0	7	56
11:15 AM	3	21	0	0	0	24	2	20	1	0	0	23	0	2	5	0	0	7	1	0	0	0	0	1	55
11:30 AM	1	8	0	0	0	9	0	22	0	0	0	22	1	3	3	0	0	7	3	0	7	0	0	10	48
11:45 AM	0	17	1	0	0	18	0	16	1	0	0	17	0	4	0	0	0	4	2	0	0	0	0	2	41
Total	7	65	1	0	0	73	2	80	3	0	0	85	3	10	9	0	0	22	10	1	9	0	0	20	200
Approach %	9.6	89.0	1.4	0.0	-	-	2.4	94.1	3.5	0.0	-	-	13.6	45.5	40.9	0.0	-	-	50.0	5.0	45.0	0.0	-	-	-
Total %	3.5	32.5	0.5	0.0	-	36.5	1.0	40.0	1.5	0.0	-	42.5	1.5	5.0	4.5	0.0	-	11.0	5.0	0.5	4.5	0.0	-	10.0	-
PHF	0.583	0.774	0.250	0.000	-	0.760	0.250	0.909	0.750	0.000	-	0.924	0.375	0.625	0.450	0.000	-	0.786	0.625	0.250	0.321	0.000	-	0.500	0.893
Lights	6	61	1	0	-	68	2	69	3	0	-	74	3	10	8	0	-	21	9	1	9	0	-	19	182
% Lights	85.7	93.8	100.0	-	-	93.2	100.0	86.3	100.0	-	-	87.1	100.0	100.0	88.9	-	-	95.5	90.0	100.0	100.0	-	-	95.0	91.0
Mediums	1	2	0	0	-	3	0	8	0	0	-	8	0	0	1	0	-	1	1	0	0	0	-	1	13
% Mediums	14.3	3.1	0.0	-	-	4.1	0.0	10.0	0.0	-	-	9.4	0.0	0.0	11.1	-	-	4.5	10.0	0.0	0.0	-	-	5.0	6.5
Articulated Trucks	0	2	0	0	-	2	0	3	0	0	-	3	0	0	0	0	-	0	0	0	0	0	-	0	5
% Articulated Trucks	0.0	3.1	0.0	-	-	2.7	0.0	3.8	0.0	-	-	3.5	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	2.5
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: 5 Side Road & Sixth Line  
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Start Date: 12/13/2016  
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Turning Movement Peak Hour Data Plot (11:00 AM)



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Count Name: 5 Side Road & Sixth Line  
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Start Date: 12/13/2016  
Page No: 8

### Turning Movement Peak Hour Data (12:00 PM)

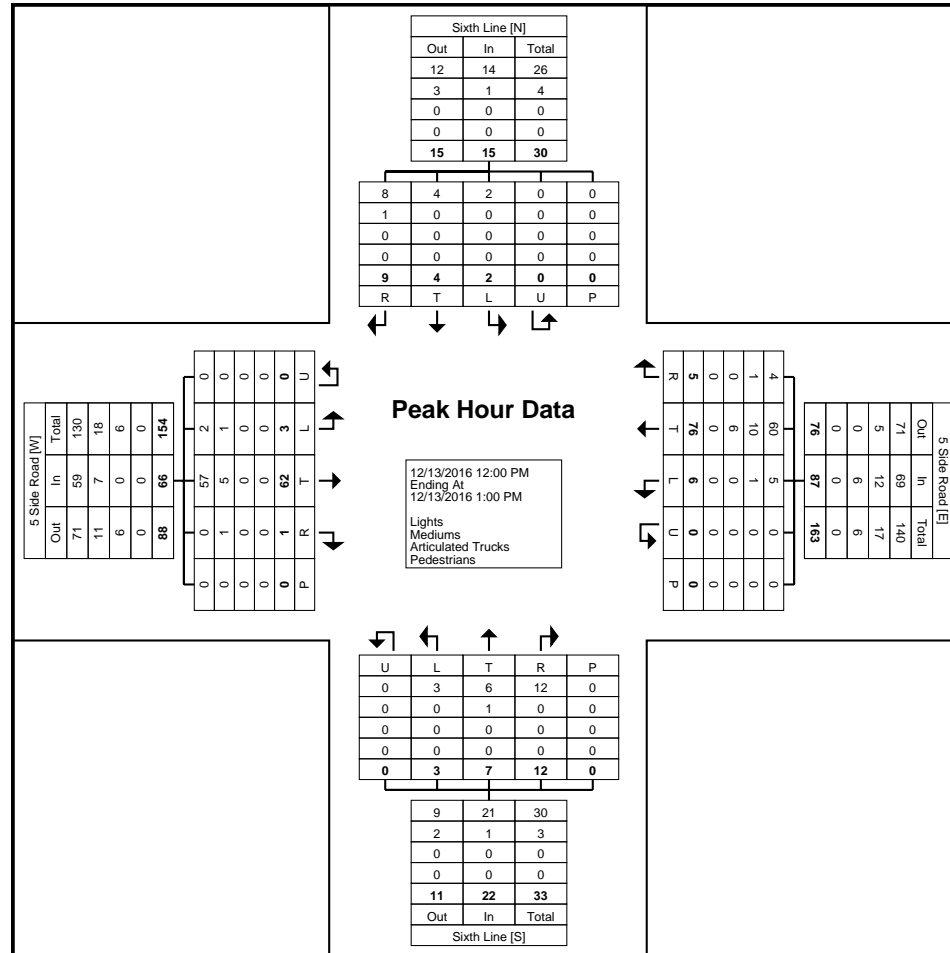
Start Time	5 Side Road Eastbound						5 Side Road Westbound						Sixth Line Northbound						Sixth Line Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
12:00 PM	1	11	1	0	0	13	2	14	1	0	0	17	0	2	6	0	0	8	0	2	3	0	0	5	43
12:15 PM	1	16	0	0	0	17	3	19	2	0	0	24	1	2	3	0	0	6	2	1	2	0	0	5	52
12:30 PM	1	16	0	0	0	17	0	20	0	0	0	20	1	1	1	0	0	3	0	0	4	0	0	4	44
12:45 PM	0	19	0	0	0	19	1	23	2	0	0	26	1	2	2	0	0	5	0	1	0	0	0	1	51
Total	3	62	1	0	0	66	6	76	5	0	0	87	3	7	12	0	0	22	2	4	9	0	0	15	190
Approach %	4.5	93.9	1.5	0.0	-	-	6.9	87.4	5.7	0.0	-	-	13.6	31.8	54.5	0.0	-	-	13.3	26.7	60.0	0.0	-	-	-
Total %	1.6	32.6	0.5	0.0	-	34.7	3.2	40.0	2.6	0.0	-	45.8	1.6	3.7	6.3	0.0	-	11.6	1.1	2.1	4.7	0.0	-	7.9	-
PHF	0.750	0.816	0.250	0.000	-	0.868	0.500	0.826	0.625	0.000	-	0.837	0.750	0.875	0.500	0.000	-	0.688	0.250	0.500	0.563	0.000	-	0.750	0.913
Lights	2	57	0	0	-	59	5	60	4	0	-	69	3	6	12	0	-	21	2	4	8	0	-	14	163
% Lights	66.7	91.9	0.0	-	-	89.4	83.3	78.9	80.0	-	-	79.3	100.0	85.7	100.0	-	-	95.5	100.0	100.0	88.9	-	-	93.3	85.8
Mediums	1	5	1	0	-	7	1	10	1	0	-	12	0	1	0	0	-	1	0	0	1	0	-	1	21
% Mediums	33.3	8.1	100.0	-	-	10.6	16.7	13.2	20.0	-	-	13.8	0.0	14.3	0.0	-	-	4.5	0.0	0.0	11.1	-	-	6.7	11.1
Articulated Trucks	0	0	0	0	-	0	0	6	0	0	-	6	0	0	0	0	-	0	0	0	0	0	-	0	6
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	7.9	0.0	-	-	6.9	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	3.2
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: 5 Side Road & Sixth Line  
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Start Date: 12/13/2016  
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Turning Movement Peak Hour Data Plot (12:00 PM)



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Count Name: 5 Side Road & Sixth Line  
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Page No: 10

### Turning Movement Peak Hour Data (4:45 PM)

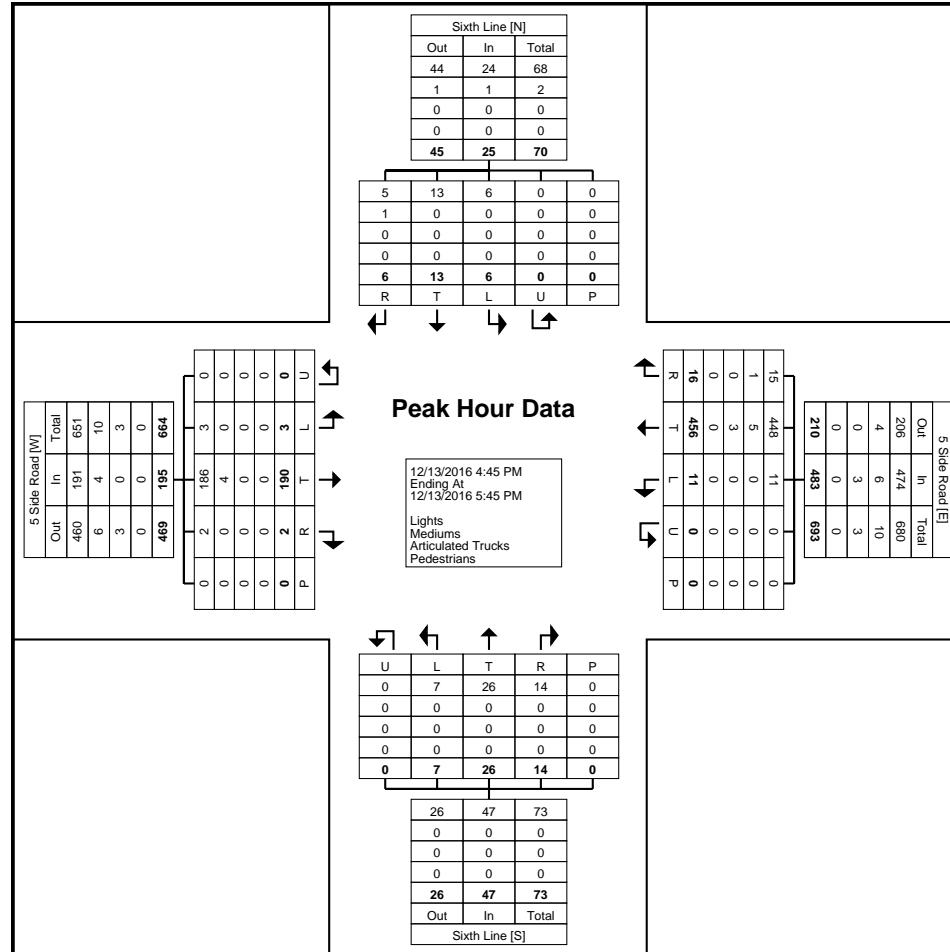
Start Time	5 Side Road Eastbound						5 Side Road Westbound						Sixth Line Northbound						Sixth Line Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
4:45 PM	1	41	0	0	0	42	3	88	6	0	0	97	4	7	1	0	0	12	2	5	2	0	0	9	160
5:00 PM	0	60	1	0	0	61	2	107	6	0	0	115	0	5	7	0	0	12	3	3	1	0	0	7	195
5:15 PM	0	50	1	0	0	51	3	141	3	0	0	147	1	5	5	0	0	11	0	3	2	0	0	5	214
5:30 PM	2	39	0	0	0	41	3	120	1	0	0	124	2	9	1	0	0	12	1	2	1	0	0	4	181
Total	3	190	2	0	0	195	11	456	16	0	0	483	7	26	14	0	0	47	6	13	6	0	0	25	750
Approach %	1.5	97.4	1.0	0.0	-	-	2.3	94.4	3.3	0.0	-	-	14.9	55.3	29.8	0.0	-	-	24.0	52.0	24.0	0.0	-	-	-
Total %	0.4	25.3	0.3	0.0	-	26.0	1.5	60.8	2.1	0.0	-	64.4	0.9	3.5	1.9	0.0	-	6.3	0.8	1.7	0.8	0.0	-	3.3	-
PHF	0.375	0.792	0.500	0.000	-	0.799	0.917	0.809	0.667	0.000	-	0.821	0.438	0.722	0.500	0.000	-	0.979	0.500	0.650	0.750	0.000	-	0.694	0.876
Lights	3	186	2	0	-	191	11	448	15	0	-	474	7	26	14	0	-	47	6	13	5	0	-	24	736
% Lights	100.0	97.9	100.0	-	-	97.9	100.0	98.2	93.8	-	-	98.1	100.0	100.0	100.0	-	-	100.0	100.0	100.0	83.3	-	-	96.0	98.1
Mediums	0	4	0	0	-	4	0	5	1	0	-	6	0	0	0	0	-	0	0	0	1	0	-	1	11
% Mediums	0.0	2.1	0.0	-	-	2.1	0.0	1.1	6.3	-	-	1.2	0.0	0.0	0.0	-	-	0.0	0.0	0.0	16.7	-	-	4.0	1.5
Articulated Trucks	0	0	0	0	-	0	0	3	0	0	-	3	0	0	0	0	-	0	0	0	0	0	-	0	3
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.7	0.0	-	-	0.6	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.4
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Turning Movement Peak Hour Data Plot (4:45 PM)



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Count Name: 5 Side Road & Sixth Line  
Site Code:  
Start Date: 12/13/2016  
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# Ontario Traffic Inc

## Morning Peak Diagram

### Specified Period

**From:** 6:00:00  
**To:** 10:00:00

### One Hour Peak

**From:** 7:30:00  
**To:** 8:30:00

**Municipality:** Milton  
**Site #:** 1711400005  
**Intersection:** Winston Churchill Blvd & Hwy 401 N  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

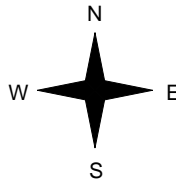
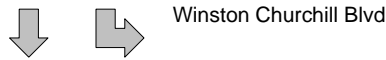
**Major Road:** Winston Churchill Blvd runs N/S

North Leg Total: 2609  
North Entering: 1051  
North Peds: 0  
Peds Cross:  $\times$

Heavys	0	0	0
Trucks	46	0	46
Cars	1005	0	1005
<b>Totals</b>	<b>1051</b>	<b>0</b>	

Heavys	0
Trucks	93
Cars	1465
<b>Totals</b>	<b>1558</b>

East Leg Total: 618  
East Entering: 618  
East Peds: 0  
Peds Cross:  $\times$



Cars	Trucks	Heavys	Totals
323	47	0	370
228	20	0	248
<b>551</b>	<b>67</b>	<b>0</b>	

Hwy 401 North Ramp Termin



Winston Churchill Blvd

Cars	1233	Cars	1142	0	1142
Trucks	66	Trucks	46	0	46
Heavys	0	Heavys	0	0	0
<b>Totals</b>	<b>1299</b>	<b>Totals</b>	<b>1188</b>	<b>0</b>	

Peds Cross:  $\times$   
South Peds: 0  
South Entering: 1188  
South Leg Total: 2487

## Comments

# Ontario Traffic Inc

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 19:00:00

### One Hour Peak

**From:** 16:45:00

**To:** 17:45:00

**Municipality:** Milton  
**Site #:** 1711400005  
**Intersection:** Winston Churchill Blvd & Hwy 401 N  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Winston Churchill Blvd runs N/S

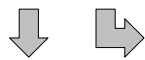
North Leg Total: 2787  
 North Entering: 1087  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	0	0	0
Trucks	36	0	36
Cars	1051	0	1051
<b>Totals</b>	<b>1087</b>	<b>0</b>	

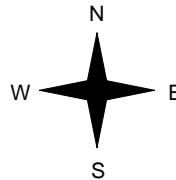


Heavys	0
Trucks	89
Cars	1611
<b>Totals</b>	<b>1700</b>

East Leg Total: 895  
 East Entering: 895  
 East Peds: 0  
 Peds Cross:  $\times$



Winston Churchill Blvd



	Cars	Trucks	Heavys	Totals
	466	26	0	492
	392	11	0	403
	858	37	0	

Hwy 401 North Ramp Termin



Cars	Trucks	Heavys	Totals
0	0	0	0

Cars	1443
Trucks	47
Heavys	0
<b>Totals</b>	<b>1490</b>



Winston Churchill Blvd

Cars	1145	0	1145
Trucks	63	0	63
Heavys	0	0	0
<b>Totals</b>	<b>1208</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 1208  
 South Leg Total: 2698

## Comments

# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400005  
**Intersection:** Winston Churchill Blvd & Hwy 401 N  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Winston Churchill Blvd runs N/S

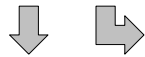
North Leg Total: 17648  
 North Entering: 6767  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	0	0	0
Trucks	378	0	378
Cars	6389	0	6389
<b>Totals</b>	<b>6767</b>	<b>0</b>	

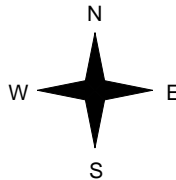


Heavys	0
Trucks	754
Cars	10127
<b>Totals</b>	<b>10881</b>

East Leg Total: 5613  
 East Entering: 5613  
 East Peds: 0  
 Peds Cross:  $\times$



Winston Churchill Blvd



	Cars	Trucks	Heavys	Totals
	2832	314	0	3146
	2357	110	0	2467
	5189	424	0	

Hwy 401 North Ramp Termin



Cars	Trucks	Heavys	Totals
0	0	0	0

Cars	8746
Trucks	488
Heavys	0
<b>Totals</b>	<b>9234</b>



Winston Churchill Blvd

Cars	7295	0	7295
Trucks	440	0	440
Heavys	0	0	0
<b>Totals</b>	<b>7735</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 7735  
 South Leg Total: 16969

### Comments

# Ontario Traffic Inc

## Traffic Count Summary

Intersection: Winston Churchill Blvd & Hwy 401      Count Date: 4-May-17      Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	583	0	583	0	1363	7:00:00	0	780	0	780	0
8:00:00	0	914	0	914	0	1942	8:00:00	0	1028	0	1028	0
9:00:00	0	972	0	972	0	2218	9:00:00	0	1246	0	1246	0
10:00:00	0	672	0	672	0	1422	10:00:00	0	750	0	750	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	0	816	0	816	0	1717	16:00:00	0	901	0	901	0
17:00:00	0	997	0	997	0	2084	17:00:00	0	1087	0	1087	0
18:00:00	0	1040	0	1040	0	2203	18:00:00	0	1163	0	1163	0
19:00:00	0	773	0	773	0	1553	19:00:00	0	780	0	780	0
Totals:	0	6767	0	6767	0	14502		0	7735	0	7735	0
<b>East Approach Totals</b>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	128	0	320	448	0	448	7:00:00	0	0	0	0	0
8:00:00	228	0	378	606	0	606	8:00:00	0	0	0	0	0
9:00:00	215	0	311	526	0	526	9:00:00	0	0	0	0	0
10:00:00	244	0	217	461	0	461	10:00:00	0	0	0	0	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	315	0	429	744	0	744	16:00:00	0	0	0	0	0
17:00:00	391	0	534	925	0	925	17:00:00	0	0	0	0	0
18:00:00	455	0	512	967	0	967	18:00:00	0	0	0	0	0
19:00:00	491	0	445	936	0	936	19:00:00	0	0	0	0	0
Totals:	2467	0	3146	5613	0	5613		0	0	0	0	0
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	7:00	8:00	9:00	10:00		16:00	17:00	18:00	19:00			
Crossing Values:	128	228	215	244		315	391	455	491			











# Ontario Traffic Inc

## Morning Peak Diagram

### Specified Period

**From:** 6:00:00  
**To:** 10:00:00

### One Hour Peak

**From:** 7:45:00  
**To:** 8:45:00

**Municipality:** Milton  
**Site #:** 1711400006  
**Intersection:** Winston Churchill Blvd & Hwy 401 S  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Winston Churchill Blvd runs N/S

North Leg Total: 2488  
North Entering: 995  
North Peds: 0  
Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	43	43
Cars	0	952	952
<b>Totals</b>	<b>0</b>	<b>995</b>	



Heavys	0
Trucks	55
Cars	1438
<b>Totals</b>	<b>1493</b>

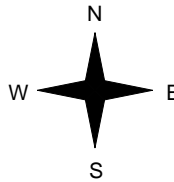
Heavys	Trucks	Cars	Totals
0	0	0	0



Winston Churchill Blvd



Hwy 401 South Ramp Termin



Heavys	Trucks	Cars	Totals
0	21	438	459
0	10	332	342
0	31	770	



Winston Churchill Blvd



Peds Cross:  $\nabla$   
West Peds: 0  
West Entering: 801  
West Leg Total: 801

Cars	1284
Trucks	53
Heavys	0
<b>Totals</b>	<b>1337</b>



Cars	0	1000	1000
Trucks	0	34	34
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>1034</b>	

Peds Cross:  $\nabla$   
South Peds: 0  
South Entering: 1034  
South Leg Total: 2371

## Comments

# Ontario Traffic Inc

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 19:00:00

### One Hour Peak

**From:** 17:00:00

**To:** 18:00:00

**Municipality:** Milton  
**Site #:** 1711400006  
**Intersection:** Winston Churchill Blvd & Hwy 401 S  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Winston Churchill Blvd runs N/S

North Leg Total: 2789

North Entering: 1314

North Peds: 0

Peds Cross:  $\times$

Heavys	0	0	0
Trucks	0	23	23
Cars	0	1291	1291
<b>Totals</b>	<b>0</b>	<b>1314</b>	



Heavys 0

Trucks 61

Cars 1414

**Totals 1475**

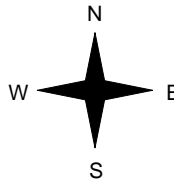
Heavys	Trucks	Cars	Totals
0	0	0	0



Winston Churchill Blvd



Hwy 401 South Ramp Termin



Heavys	Trucks	Cars	Totals
0	33	286	319
0	8	284	292
0	41	570	



Winston Churchill Blvd

Peds Cross:  $\times$   
 West Peds: 0  
 West Entering: 611  
 West Leg Total: 611

Cars	1575
Trucks	31
Heavys	0
<b>Totals</b>	<b>1606</b>



Cars	0	1128	1128
Trucks	0	28	28
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>1156</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 1156  
 South Leg Total: 2762

## Comments

# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400006  
**Intersection:** Winston Churchill Blvd & Hwy 401 S  
**TFR File #:** 1  
**Count date:** 4-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Winston Churchill Blvd runs N/S

North Leg Total: 16822  
 North Entering: 7375  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	260	260
Cars	0	7115	7115
<b>Totals</b>	<b>0</b>	<b>7375</b>	



Heavys	0
Trucks	507
Cars	8940
<b>Totals</b>	<b>9447</b>

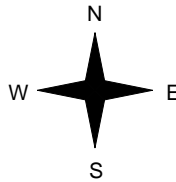
Heavys	Trucks	Cars	Totals
0	0	0	0



Winston Churchill Blvd



Hwy 401 South Ramp Termin



Heavys	Trucks	Cars	Totals
0	260	2465	2725
0	74	2284	2358
0	334	4749	



Winston Churchill Blvd

Peds Cross:  $\nabla$   
 West Peds: 0  
 West Entering: 5083  
 West Leg Total: 5083

Cars	9399
Trucks	334
Heavys	0
<b>Totals</b>	<b>9733</b>



Cars	0	6475	6475
Trucks	0	247	247
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>6722</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 6722  
 South Leg Total: 16455

### Comments

# Ontario Traffic Inc Traffic Count Summary

Intersection: Winston Churchill Blvd & Hwy 401    Count Date: 4-May-17    Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	450	0	450	0	969	7:00:00	0	519	0	519	0
8:00:00	0	876	0	876	0	1737	8:00:00	0	861	0	861	0
9:00:00	0	923	0	923	0	1936	9:00:00	0	1013	0	1013	0
10:00:00	0	680	0	680	0	1268	10:00:00	0	588	0	588	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	0	901	0	901	0	1702	16:00:00	0	801	0	801	0
17:00:00	0	1128	0	1128	0	2168	17:00:00	0	1040	0	1040	0
18:00:00	0	1314	0	1314	0	2470	18:00:00	0	1156	0	1156	0
19:00:00	0	1103	0	1103	0	1847	19:00:00	0	744	0	744	0
Totals:	0	7375	0	7375	0	14097		0	6722	0	6722	0
<b>East Approach Totals</b>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	0	0	0	0	741	7:00:00	403	0	338	741	0
8:00:00	0	0	0	0	0	729	8:00:00	397	0	332	729	0
9:00:00	0	0	0	0	0	821	9:00:00	470	0	351	821	0
10:00:00	0	0	0	0	0	636	10:00:00	323	0	313	636	0
15:00:00	0	0	0	0	0	0	15:00:00	0	0	0	0	0
16:00:00	0	0	0	0	0	597	16:00:00	293	0	304	597	0
17:00:00	0	0	0	0	0	523	17:00:00	294	0	229	523	0
18:00:00	0	0	0	0	0	611	18:00:00	319	0	292	611	0
19:00:00	0	0	0	0	0	425	19:00:00	226	0	199	425	0
Totals:	0	0	0	0	0	5083		2725	0	2358	5083	0
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	7:00	8:00	9:00	10:00		16:00	17:00	18:00	19:00			
Crossing Values:	403	397	470	323		293	294	319	226			











# Ontario Traffic Inc

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00  
**To:** 15:00:00

### One Hour Peak

**From:** 13:45:00  
**To:** 14:45:00

**Municipality:** Milton  
**Site #:** 0711400007  
**Intersection:** Trafalgar Rd & Hwy 401 North Rarr  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

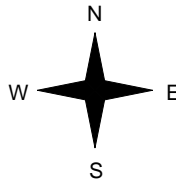
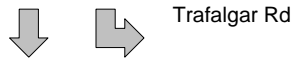
**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 2154  
North Entering: 866  
North Peds: 0  
Peds Cross:  $\times$

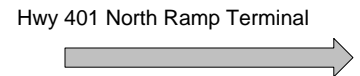
Heavys	0	0	0
Trucks	27	0	27
Cars	839	0	839
<b>Totals</b>	<b>866</b>	<b>0</b>	

Heavys	0
Trucks	26
Cars	1262
<b>Totals</b>	<b>1288</b>

East Leg Total: 725  
East Entering: 725  
East Peds: 0  
Peds Cross:  $\times$



Cars	Trucks	Heavys	Totals
578	12	0	590
129	6	0	135
<b>707</b>	<b>18</b>	<b>0</b>	



Cars	Trucks	Heavys	Totals
0	0	0	0

Cars	968
Trucks	33
Heavys	0
<b>Totals</b>	<b>1001</b>

Cars	684	0	684
Trucks	14	0	14
Heavys	0	0	0
<b>Totals</b>	<b>698</b>	<b>0</b>	

Peds Cross:  $\times$   
South Peds: 0  
South Entering: 698  
South Leg Total: 1699

## Comments

# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 0711400007  
**Intersection:** Trafalgar Rd & Hwy 401 North Rarr  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

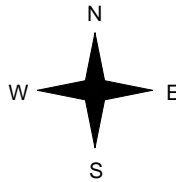
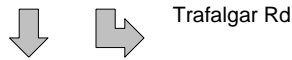
**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 7997  
 North Entering: 2997  
 North Peds: 0  
 Peds Cross:  $\times$

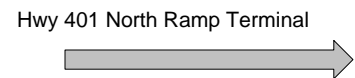
Heavys	0	0	0
Trucks	104	0	104
Cars	2893	0	2893
<b>Totals</b>	<b>2997</b>	<b>0</b>	

Heavys	0
Trucks	109
Cars	4891
<b>Totals</b>	<b>5000</b>

East Leg Total: 2831  
 East Entering: 2831  
 East Peds: 0  
 Peds Cross:  $\times$



	Cars	Trucks	Heavys	Totals
	2276	68	0	2344
	441	46	0	487
	<b>2717</b>	<b>114</b>	<b>0</b>	



	Cars	Trucks	Heavys	Totals
	0	0	0	0

Cars	3334
Trucks	150
Heavys	0
<b>Totals</b>	<b>3484</b>

Cars	2615	0	2615
Trucks	41	0	41
Heavys	0	0	0
<b>Totals</b>	<b>2656</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 2656  
 South Leg Total: 6140

### Comments

# Ontario Traffic Inc Traffic Count Summary

Intersection: Trafalgar Rd & Hwy 401 North Rd    Count Date: 6-May-17    Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	0	716	0	716	0	1303	12:00:00	0	587	0	587	0
13:00:00	0	676	0	676	0	1377	13:00:00	0	701	0	701	0
14:00:00	0	745	0	745	0	1429	14:00:00	0	684	0	684	0
15:00:00	0	860	0	860	0	1544	15:00:00	0	684	0	684	0
<b>Totals:</b>	0	2997	0	2997	0	5653		0	2656	0	2656	0
<b>East Approach Totals</b>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	118	0	526	644	0	644	12:00:00	0	0	0	0	0
13:00:00	130	0	584	714	0	714	13:00:00	0	0	0	0	0
14:00:00	122	0	657	779	0	779	14:00:00	0	0	0	0	0
15:00:00	117	0	577	694	0	694	15:00:00	0	0	0	0	0
<b>Totals:</b>	487	0	2344	2831	0	2831		0	0	0	0	0
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	11:00	12:00	13:00	13:00		14:00	14:00	15:00	15:00			
Crossing Values:	0	118	130	130		122	122	117	117			









# Ontario Traffic Inc

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00  
**To:** 15:00:00

### One Hour Peak

**From:** 13:45:00  
**To:** 14:45:00

**Municipality:** Milton  
**Site #:** 1711400008  
**Intersection:** Trafalgar Rd & Hwy 401 South Ram  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 1668  
North Entering: 571  
North Peds: 0  
Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	9	9
Cars	0	562	562
<b>Totals</b>	<b>0</b>	<b>571</b>	



Heavys	0
Trucks	42
Cars	1055
<b>Totals</b>	<b>1097</b>

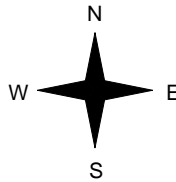
Heavys	Trucks	Cars	Totals
0	0	0	0



Trafalgar Rd



Hwy 401 South Ramp Terminal



Heavys	Trucks	Cars	Totals
0	6	238	244
0	9	215	224
0	15	453	



Trafalgar Rd



Peds Cross:  $\nabla$   
West Peds: 0  
West Entering: 468  
West Leg Total: 468

Cars	777
Trucks	18
Heavys	0
<b>Totals</b>	<b>795</b>



Cars	0	817	817
Trucks	0	36	36
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>853</b>	

Peds Cross:  $\nabla$   
South Peds: 0  
South Entering: 853  
South Leg Total: 1648

## Comments



# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400008  
**Intersection:** Trafalgar Rd & Hwy 401 South Ram  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 6161  
 North Entering: 1976  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	0	0	0
Trucks	0	62	62
Cars	0	1914	1914
<b>Totals</b>	<b>0</b>	<b>1976</b>	

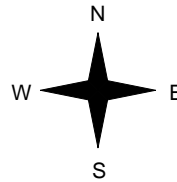
Heavys	0
Trucks	130
Cars	4055
<b>Totals</b>	<b>4185</b>

Heavys	Trucks	Cars	Totals
0	0	0	0



Hwy 401 South Ramp Terminal

Heavys	Trucks	Cars	Totals
0	22	881	903
0	33	838	871
0	55	1719	



Trafalgar Rd

Peds Cross:  $\nabla$   
 West Peds: 0  
 West Entering: 1774  
 West Leg Total: 1774

Cars	2752
Trucks	95
Heavys	0
<b>Totals</b>	<b>2847</b>



Cars	0	3174	3174
Trucks	0	108	108
Heavys	0	0	0
<b>Totals</b>	<b>0</b>	<b>3282</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 3282  
 South Leg Total: 6129

### Comments

# Ontario Traffic Inc Traffic Count Summary

Intersection: Trafalgar Rd & Hwy 401 South Rar      Count Date: 6-May-17      Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	0	476	0	476	0	1234	12:00:00	0	758	0	758	0
13:00:00	0	455	0	455	0	1282	13:00:00	0	827	0	827	0
14:00:00	0	494	0	494	0	1365	14:00:00	0	871	0	871	0
15:00:00	0	551	0	551	0	1377	15:00:00	0	826	0	826	0
Totals:	0	1976	0	1976	0	5258	0	3282	0	3282	0	
<b>East Approach Totals</b>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	0	0	0	0	0	412	12:00:00	195	0	217	412	0
13:00:00	0	0	0	0	0	452	13:00:00	236	0	216	452	0
14:00:00	0	0	0	0	0	424	14:00:00	226	0	198	424	0
15:00:00	0	0	0	0	0	486	15:00:00	246	0	240	486	0
Totals:	0	0	0	0	0	1774	903	0	871	1774	0	
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	0:00	0:00	0:00	11:00		12:00	13:00	14:00	15:00			
Crossing Values:	0	0	0	0		195	236	226	246			









# Ontario Traffic Inc

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00

**To:** 15:00:00

### One Hour Peak

**From:** 13:15:00

**To:** 14:15:00

**Municipality:** Milton  
**Site #:** 1711400009  
**Intersection:** James Snow Pkwy & Hwy 401 North  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

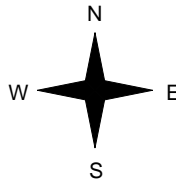
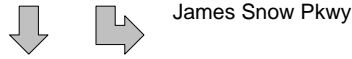
**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 1359  
 North Entering: 683  
 North Peds: 0  
 Peds Cross:  $\times$

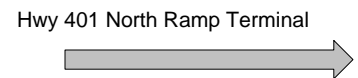
Heavys	0	0	0
Trucks	14	0	14
Cars	669	0	669
<b>Totals</b>	<b>683</b>	<b>0</b>	

Heavys	0
Trucks	34
Cars	642
<b>Totals</b>	<b>676</b>

East Leg Total: 868  
 East Entering: 868  
 East Peds: 0  
 Peds Cross:  $\times$



	Cars	Trucks	Heavys	Totals
	246	13	0	259
	606	3	0	609
	852	16	0	



	Cars	Trucks	Heavys	Totals
	0	0	0	0

Cars	1275	Cars	396	0	396
Trucks	17	Trucks	21	0	21
Heavys	0	Heavys	0	0	0
<b>Totals</b>	<b>1292</b>	<b>Totals</b>	<b>417</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 417  
 South Leg Total: 1709

## Comments

# Ontario Traffic Inc

## Total Count Diagram

**Municipality:** Milton  
**Site #:** 1711400009  
**Intersection:** James Snow Pkwy & Hwy 401 North  
**TFR File #:** 1  
**Count date:** 6-May-17

**Weather conditions:**  
**Person(s) who counted:**

**\*\* Signalized Intersection \*\***

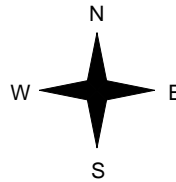
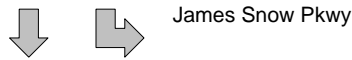
**Major Road:** James Snow Pkwy runs N/S

North Leg Total: 5146  
 North Entering: 2617  
 North Peds: 0  
 Peds Cross:  $\times$

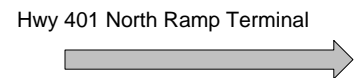
Heavys	0	0	0
Trucks	66	0	66
Cars	2551	0	2551
<b>Totals</b>	<b>2617</b>	<b>0</b>	

Heavys	0
Trucks	128
Cars	2401
<b>Totals</b>	<b>2529</b>

East Leg Total: 3154  
 East Entering: 3154  
 East Peds: 0  
 Peds Cross:  $\times$



	Cars	Trucks	Heavys	Totals
	917	48	0	965
	2174	15	0	2189
	<b>3091</b>	<b>63</b>	<b>0</b>	



	Cars	Trucks	Heavys	Totals
	0	0	0	0

Cars	4725	Cars	1484	0	1484
Trucks	81	Trucks	80	0	80
Heavys	0	Heavys	0	0	0
<b>Totals</b>	<b>4806</b>	<b>Totals</b>	<b>1564</b>	<b>0</b>	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 1564  
 South Leg Total: 6370

### Comments



# Ontario Traffic Inc Traffic Count Summary

Intersection: James Snow Pkwy & Hwy 401 Nor    Count Date: 6-May-17    Municipality: Milton

<b>North Approach Totals</b>						North/South Total Approaches	<b>South Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	0	634	0	634	0	1010	12:00:00	0	376	0	376	0
13:00:00	0	608	0	608	0	990	13:00:00	0	382	0	382	0
14:00:00	0	682	0	682	0	1088	14:00:00	0	406	0	406	0
15:00:00	0	693	0	693	0	1093	15:00:00	0	400	0	400	0
<b>Totals:</b>	0	2617	0	2617	0	4181		0	1564	0	1564	0
<b>East Approach Totals</b>						East/West Total Approaches	<b>West Approach Totals</b>					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
11:00:00	0	0	0	0	0	0	11:00:00	0	0	0	0	0
12:00:00	499	0	203	702	0	702	12:00:00	0	0	0	0	0
13:00:00	545	0	242	787	0	787	13:00:00	0	0	0	0	0
14:00:00	589	0	270	859	0	859	14:00:00	0	0	0	0	0
15:00:00	556	0	250	806	0	806	15:00:00	0	0	0	0	0
<b>Totals:</b>	2189	0	965	3154	0	3154		0	0	0	0	0
<b>Calculated Values for Traffic Crossing Major Street</b>												
Hours Ending:	11:00	12:00	13:00	13:00		14:00	14:00	15:00	15:00			
Crossing Values:	0	499	545	545		589	589	556	556			











Paradigm Transportation Solutions Limited  
22 King Street South, Suite 300

Waterloo, Ontario, Canada N2J 1N8  
519-896-3163 cbowness@ptsl.com

Count Name: Steeles Avenue & Eight  
Line/Toronto Outlets - Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 1

### Turning Movement Data

Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Toronto Premium Outlets Northbound						Eighth Line Southbound						Int. Total	
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total		
6:00 AM	10	87	0	0	0	97	2	90	2	0	0	94	0	0	0	0	0	0	9	0	37	0	0	0	46	237
6:15 AM	6	100	2	0	0	108	0	89	1	0	0	90	0	0	1	0	0	1	9	0	38	0	0	0	47	246
6:30 AM	5	113	2	0	0	120	2	112	1	0	0	115	0	0	1	0	0	1	24	1	55	0	0	0	80	316
6:45 AM	16	137	8	0	0	161	9	86	2	0	0	97	0	0	0	0	0	0	15	1	55	0	0	0	71	329
Hourly Total	37	437	12	0	0	486	13	377	6	0	0	396	0	0	2	0	0	2	57	2	185	0	0	0	244	1128
7:00 AM	11	178	4	0	0	193	3	123	3	0	0	129	3	0	4	0	0	7	21	1	56	0	0	0	78	407
7:15 AM	14	198	2	0	0	214	4	98	2	0	0	104	1	0	2	0	0	3	39	2	87	0	0	0	128	449
7:30 AM	17	217	3	0	0	237	4	117	6	0	0	127	0	0	4	0	0	4	43	0	86	0	0	0	129	497
7:45 AM	20	223	6	0	0	249	13	106	5	0	0	124	1	1	1	0	0	3	51	3	98	0	0	0	152	528
Hourly Total	62	816	15	0	0	893	24	444	16	0	0	484	5	1	11	0	0	17	154	6	327	0	0	0	487	1881
8:00 AM	20	212	2	0	0	234	6	103	6	0	0	115	1	0	2	0	0	3	42	1	74	0	0	0	117	469
8:15 AM	17	215	2	0	0	234	6	116	5	0	0	127	1	0	1	0	0	2	41	1	96	0	0	0	138	501
8:30 AM	16	171	1	0	0	188	11	129	4	0	0	144	0	1	2	1	0	4	30	2	66	0	0	0	98	434
8:45 AM	16	160	6	0	0	182	8	90	5	0	0	103	3	0	8	0	0	11	20	3	42	0	0	0	65	361
Hourly Total	69	758	11	0	0	838	31	438	20	0	0	489	5	1	13	1	0	20	133	7	278	0	0	0	418	1765
9:00 AM	10	127	3	0	0	140	11	95	1	0	0	107	2	0	2	0	0	4	10	1	35	0	0	0	46	297
9:15 AM	19	113	6	0	0	138	6	93	2	0	0	101	4	0	3	0	0	7	12	2	35	0	0	0	49	295
9:30 AM	13	98	10	0	0	121	14	80	3	0	0	97	0	0	2	0	0	2	12	2	37	0	0	0	51	271
9:45 AM	12	93	10	0	0	115	36	90	4	0	0	130	4	0	3	0	0	7	6	11	17	0	0	0	34	286
Hourly Total	54	431	29	0	0	514	67	358	10	0	0	435	10	0	10	0	0	20	40	16	124	0	0	0	180	1149
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	37	128	5	0	0	170	8	116	9	0	0	133	15	5	19	0	0	39	3	4	17	0	0	0	24	366
3:15 PM	42	143	3	0	0	188	7	169	17	0	0	193	18	0	21	0	0	39	2	2	11	0	0	0	15	435
3:30 PM	66	132	4	0	0	202	16	169	14	0	0	199	18	5	17	0	0	40	8	6	19	0	0	0	33	474
3:45 PM	41	145	2	0	0	188	28	266	12	0	0	306	8	5	20	0	0	33	3	3	22	0	0	0	28	555
Hourly Total	186	548	14	0	0	748	59	720	52	0	0	831	59	15	77	0	0	151	16	15	69	0	0	0	100	1830
4:00 PM	34	158	2	0	0	194	16	261	19	0	0	296	20	6	28	0	0	54	6	5	29	0	0	0	40	584
4:15 PM	38	131	3	1	0	173	20	290	31	1	0	342	11	3	20	0	0	34	8	7	21	0	0	0	36	585
4:30 PM	38	147	3	0	0	188	24	321	29	0	0	374	10	6	17	0	0	33	6	8	17	0	0	0	31	626
4:45 PM	56	137	7	0	0	200	45	296	24	0	0	365	18	10	21	0	0	49	6	6	19	0	0	0	31	645
Hourly Total	166	573	15	1	0	755	105	1168	103	1	0	1377	59	25	86	0	0	170	26	26	86	0	0	0	138	2440
5:00 PM	66	166	3	0	0	235	28	261	25	0	0	314	18	7	39	0	0	64	11	0	11	0	0	0	22	635
5:15 PM	57	182	2	0	0	241	17	297	35	0	0	349	6	4	22	0	0	32	10	5	11	0	0	0	26	648
5:30 PM	61	130	1	1	0	193	27	283	26	0	0	336	16	7	20	0	0	43	6	4	21	0	0	0	31	603
5:45 PM	65	121	4	0	0	190	23	280	28	0	0	331	7	3	24	0	0	34	7	1	10	0	0	0	18	573
Hourly Total	249	599	10	1	0	859	95	1121	114	0	0	1330	47	21	105	0	0	173	34	10	53	0	0	0	97	2459
6:00 PM	57	120	6	0	0	183	18	198	20	0	0	236	8	3	29	0	0	40	3	2	10	0	0	0	15	474

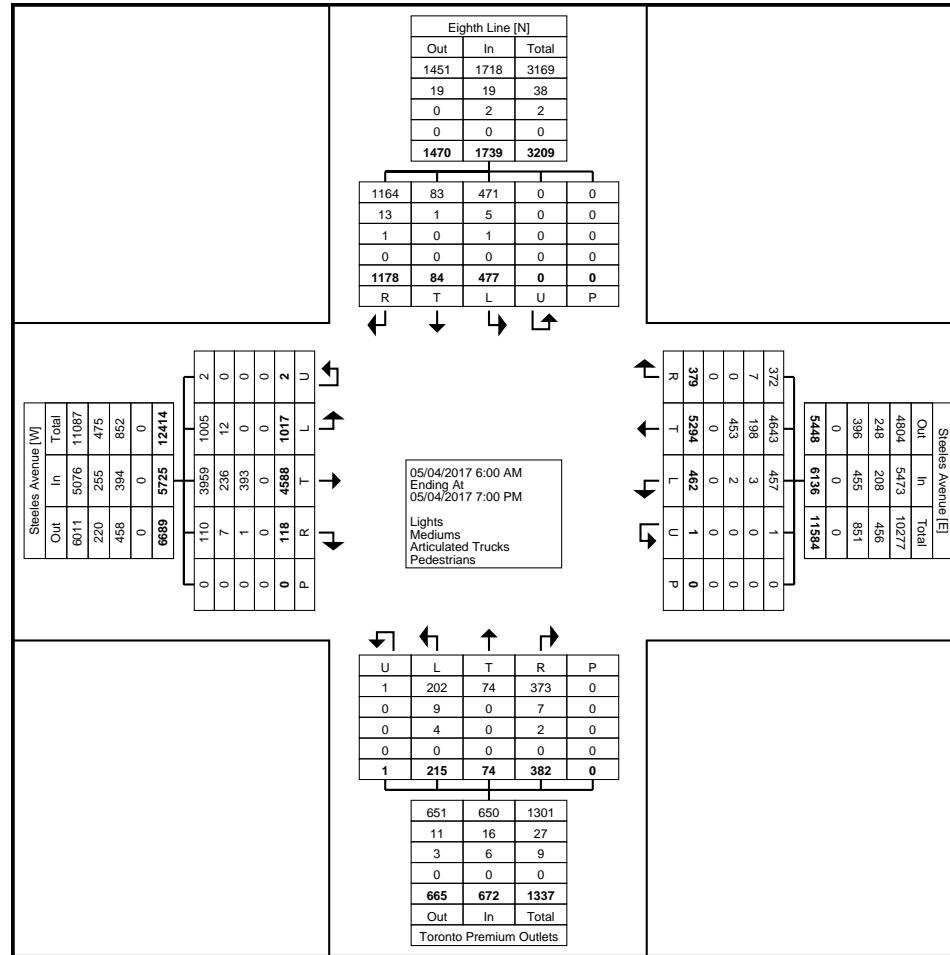




Paradigm Transportation Solutions Limited  
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Count Name: Steeles Avenue & Eight  
 Line/Toronto Outlets - Weekday  
 Site Code:  
 Start Date: 05/04/2017  
 Page No: 3



Turning Movement Data Plot



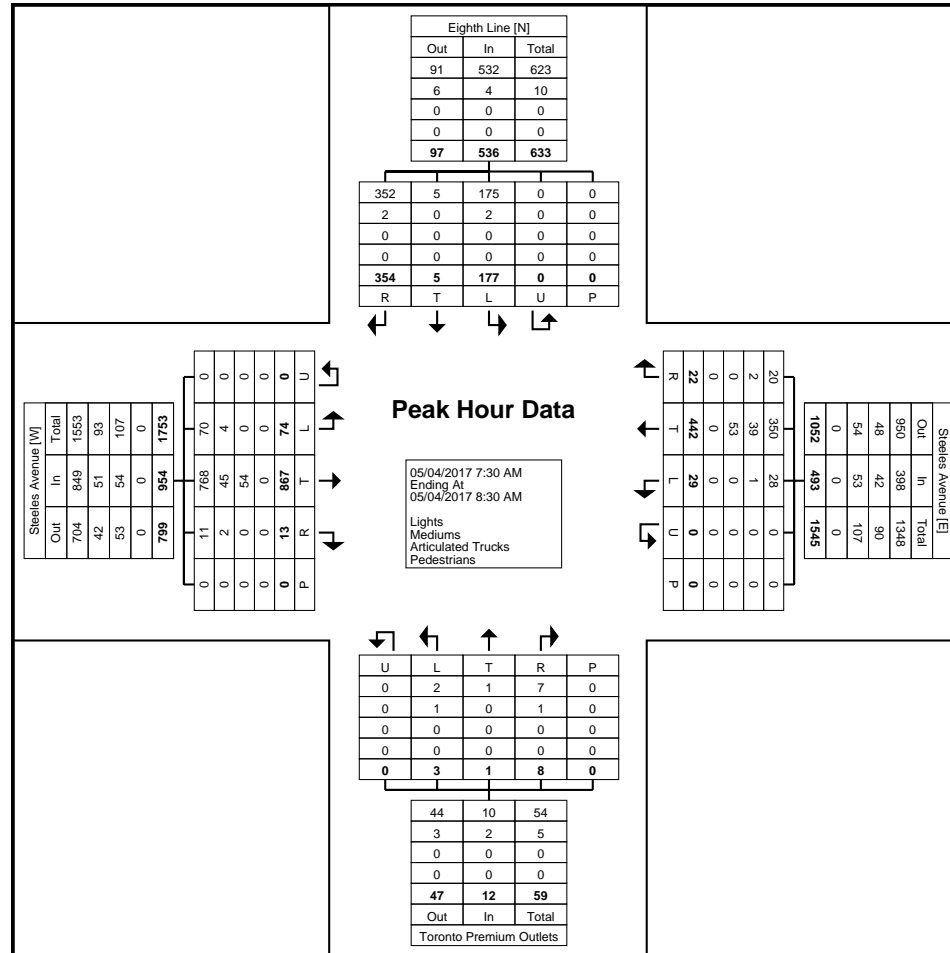




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Count Name: Steeles Avenue & Eight  
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Site Code:  
Start Date: 05/04/2017  
Page No: 5



Turning Movement Peak Hour Data Plot (7:30 AM)

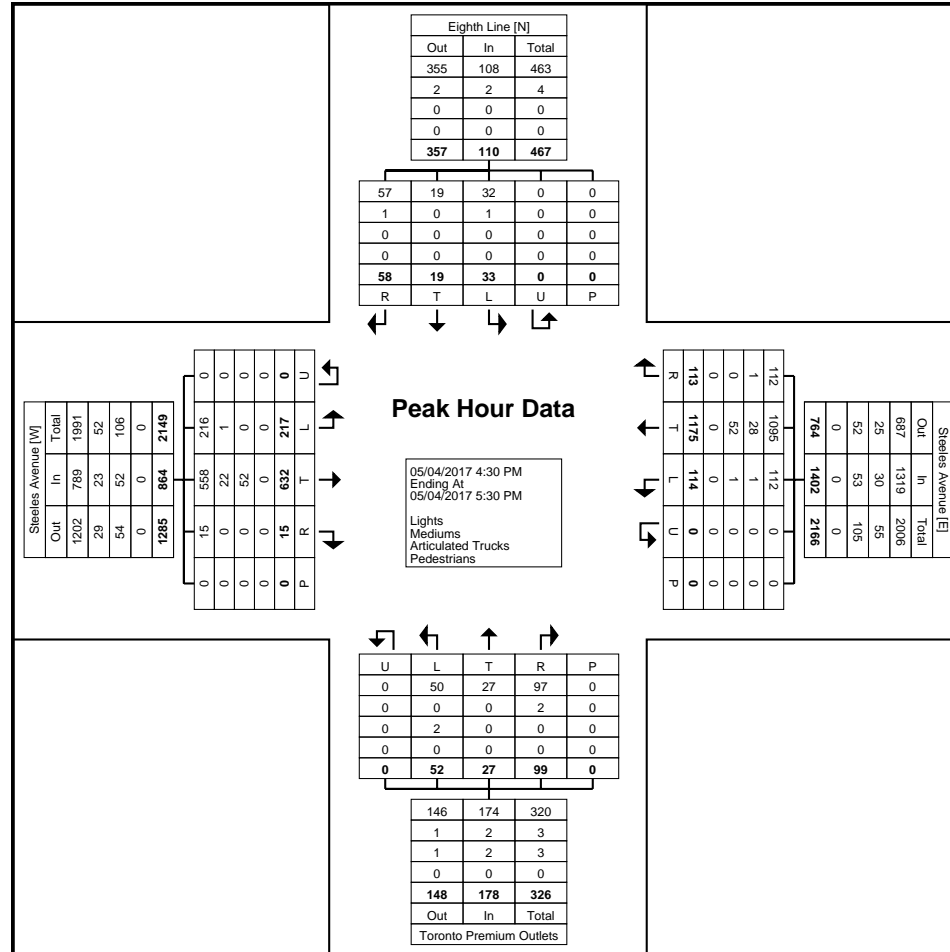




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Count Name: Steeles Avenue & Eight  
Line/Toronto Outlets - Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 7



Turning Movement Peak Hour Data Plot (4:30 PM)



Paradigm Transportation Solutions Limited  
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Count Name: Steeles Avenue & Eight  
Line/Toronto Outlets - Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 8

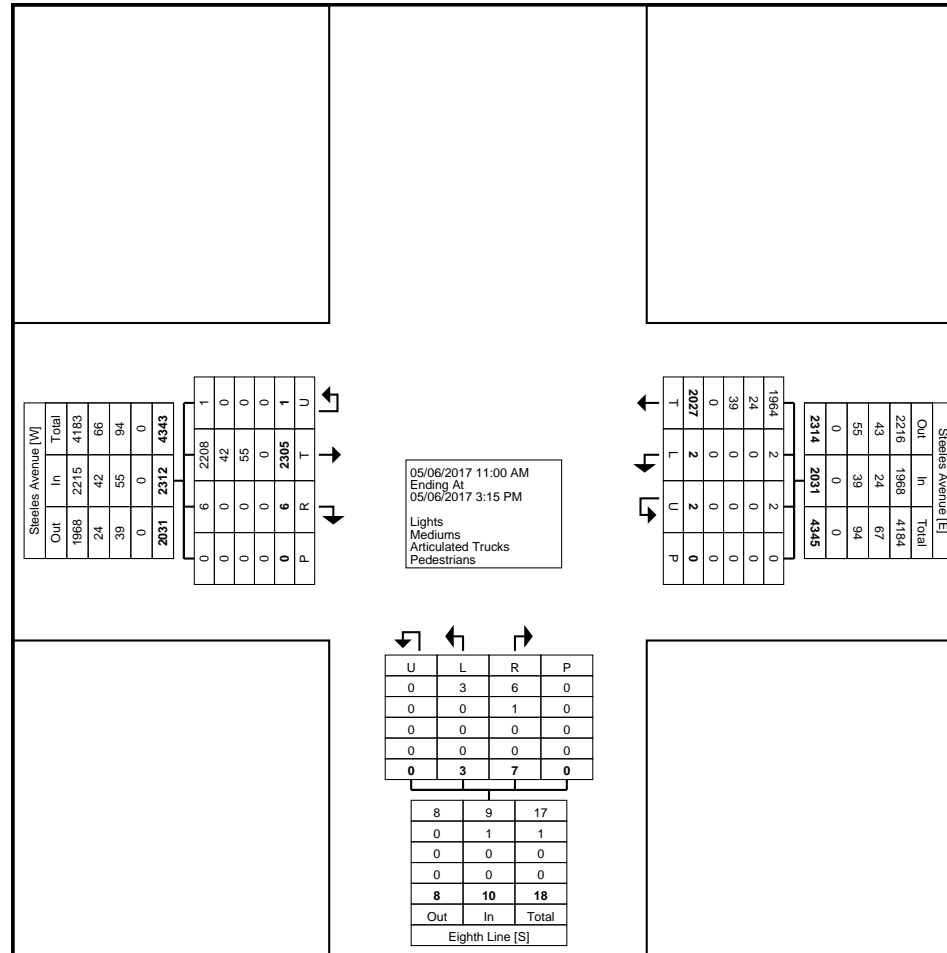




Paradigm Transportation Solutions Limited  
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Count Name: Steeles Avenue & Eighth Line  
(South) - Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 2



Turning Movement Data Plot



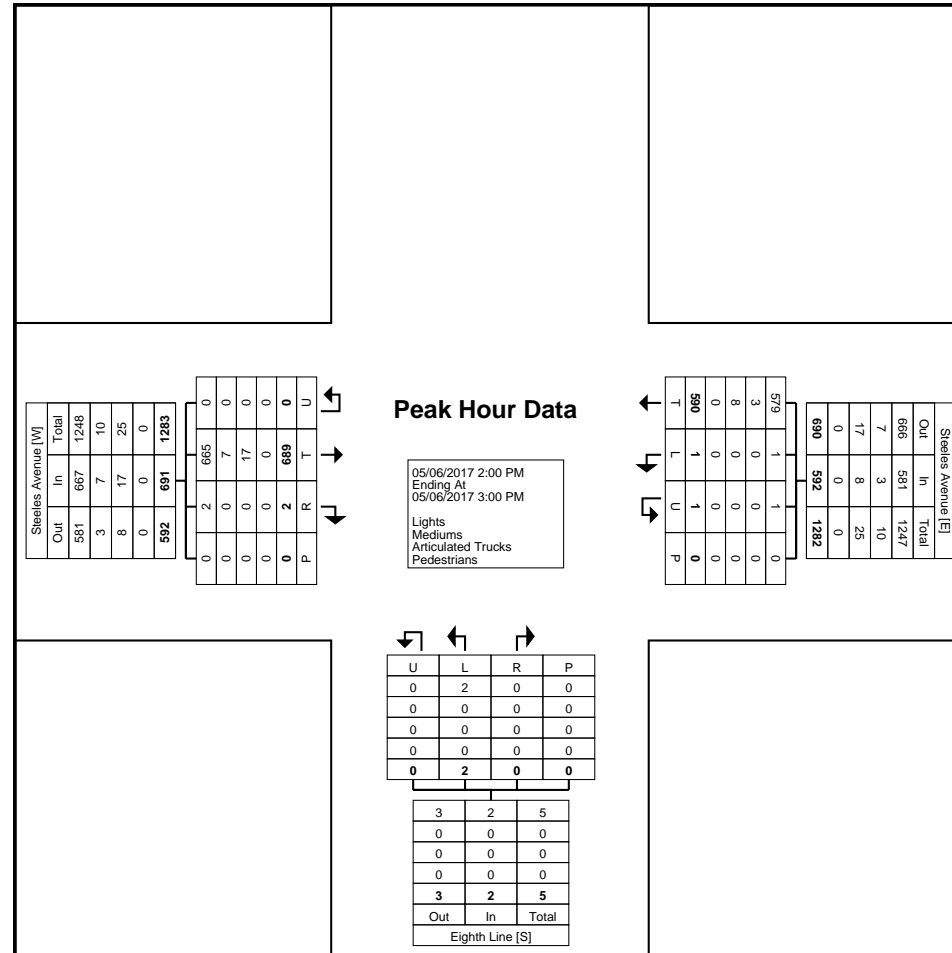




Paradigm Transportation Solutions Limited  
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Count Name: Steeles Avenue & Eighth Line  
(South) - Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 4



Turning Movement Peak Hour Data Plot (2:00 PM)



Paradigm Transportation Solutions Limited  
22 King Street South, Suite 300

Waterloo, Ontario, Canada N2J 1N8  
519-896-3163 cbowness@ptsl.com

Count Name: Steeles Avenue & Eighth Line  
(South) - Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 5



Paradigm Transportation Solutions Limited  
22 King Street South, Suite 300

Waterloo, Ontario, Canada N2J 1N8  
519-896-3163 cbowness@ptsl.com

Count Name: Steeles Avenue & Eighth Line  
(South) - Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 1

### Turning Movement Data

Start Time	Steeles Avenue Eastbound					Steeles Avenue Westbound					Eighth Line Northbound					Int. Total
	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
6:00 AM	94	0	0	0	94	0	95	0	0	95	0	0	0	0	0	189
6:15 AM	109	0	0	0	109	0	89	0	0	89	0	0	0	0	0	198
6:30 AM	143	0	0	0	143	0	115	0	0	115	0	0	0	0	0	258
6:45 AM	155	0	0	0	155	2	99	0	0	101	1	0	0	0	1	257
Hourly Total	501	0	0	0	501	2	398	0	0	400	1	0	0	0	1	902
7:00 AM	199	0	0	0	199	1	119	0	0	120	0	1	0	0	1	320
7:15 AM	242	1	0	0	243	1	108	0	0	109	0	1	0	0	1	353
7:30 AM	251	0	0	0	251	1	126	0	0	127	0	0	0	0	0	378
7:45 AM	279	0	0	0	279	0	122	1	0	123	0	0	0	0	0	402
Hourly Total	971	1	0	0	972	3	475	1	0	479	0	2	0	0	2	1453
8:00 AM	262	0	0	0	262	0	116	0	0	116	0	0	0	0	0	378
8:15 AM	261	1	0	0	262	0	129	0	0	129	1	0	0	0	1	392
8:30 AM	197	0	0	0	197	0	142	0	0	142	0	0	0	0	0	339
8:45 AM	192	1	0	0	193	0	103	0	0	103	0	0	0	0	0	296
Hourly Total	912	2	0	0	914	0	490	0	0	490	1	0	0	0	1	1405
9:00 AM	139	0	0	0	139	1	109	1	0	111	0	0	0	0	0	250
9:15 AM	124	0	0	0	124	1	101	0	0	102	1	0	0	0	1	227
9:30 AM	119	1	0	0	120	0	105	0	0	105	0	0	0	0	0	225
9:45 AM	101	1	0	0	102	0	129	0	0	129	0	1	0	0	1	232
Hourly Total	483	2	0	0	485	2	444	1	0	447	1	1	0	0	2	934
10:00 AM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	1
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	1
3:00 PM	144	0	0	0	144	0	135	0	0	135	0	0	0	0	0	279
3:15 PM	180	0	0	0	180	0	184	0	0	184	0	0	0	0	0	364
3:30 PM	160	0	0	0	160	0	227	0	0	227	1	0	0	0	1	388
3:45 PM	181	1	0	0	182	0	314	0	0	314	0	0	0	0	0	496
Hourly Total	665	1	0	0	666	0	860	0	0	860	1	0	0	0	1	1527
4:00 PM	197	1	0	0	198	0	316	0	0	316	0	2	0	0	2	516
4:15 PM	163	0	0	0	163	1	373	0	0	374	1	0	0	0	1	538
4:30 PM	167	0	0	0	167	0	370	0	0	370	0	0	0	0	0	537
4:45 PM	164	2	0	0	166	0	360	0	0	360	0	1	0	0	1	527
Hourly Total	691	3	0	0	694	1	1419	0	0	1420	1	3	0	0	4	2118
5:00 PM	196	0	0	0	196	0	316	0	0	316	1	2	0	0	3	515
5:15 PM	207	0	0	0	207	0	365	0	0	365	0	2	0	0	2	574
5:30 PM	169	0	0	0	169	1	346	0	0	347	1	0	0	0	1	517
5:45 PM	156	1	0	0	157	1	291	0	0	292	1	1	0	0	2	451
Hourly Total	728	1	0	0	729	2	1318	0	0	1320	3	5	0	0	8	2057

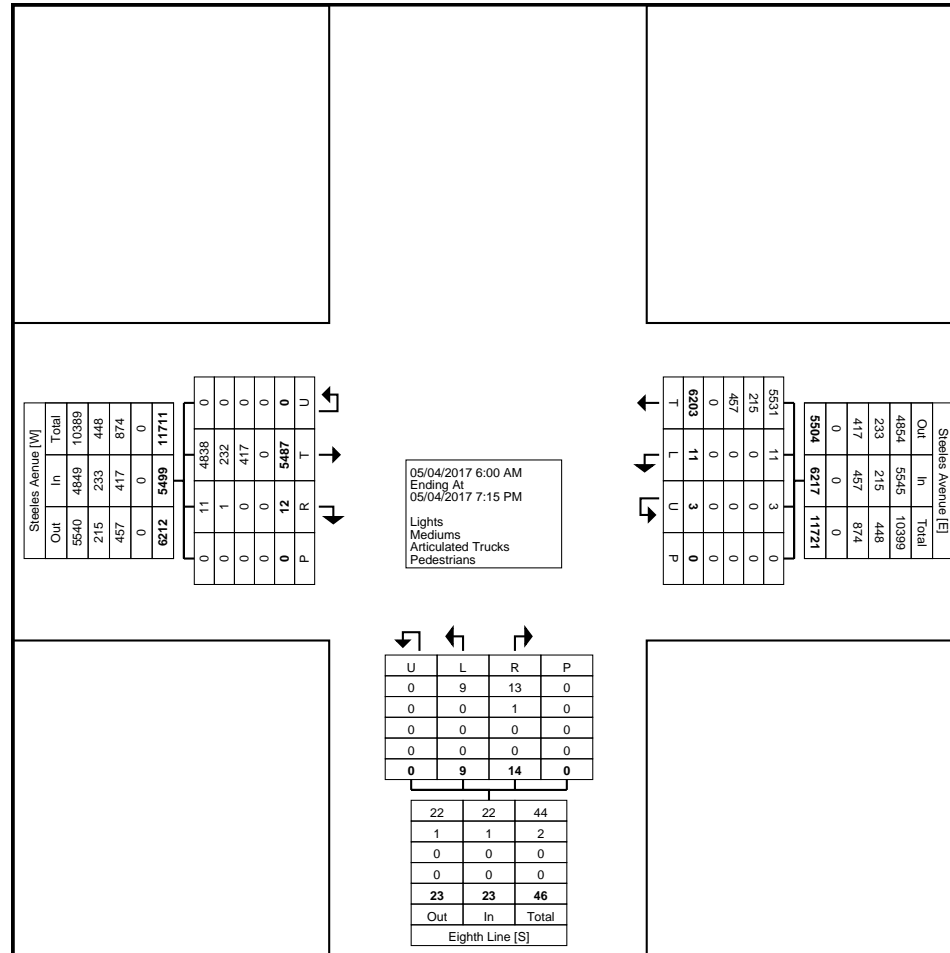




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Count Name: Steeles Avenue & Eighth Line  
 (South) - Weekday  
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Turning Movement Data Plot

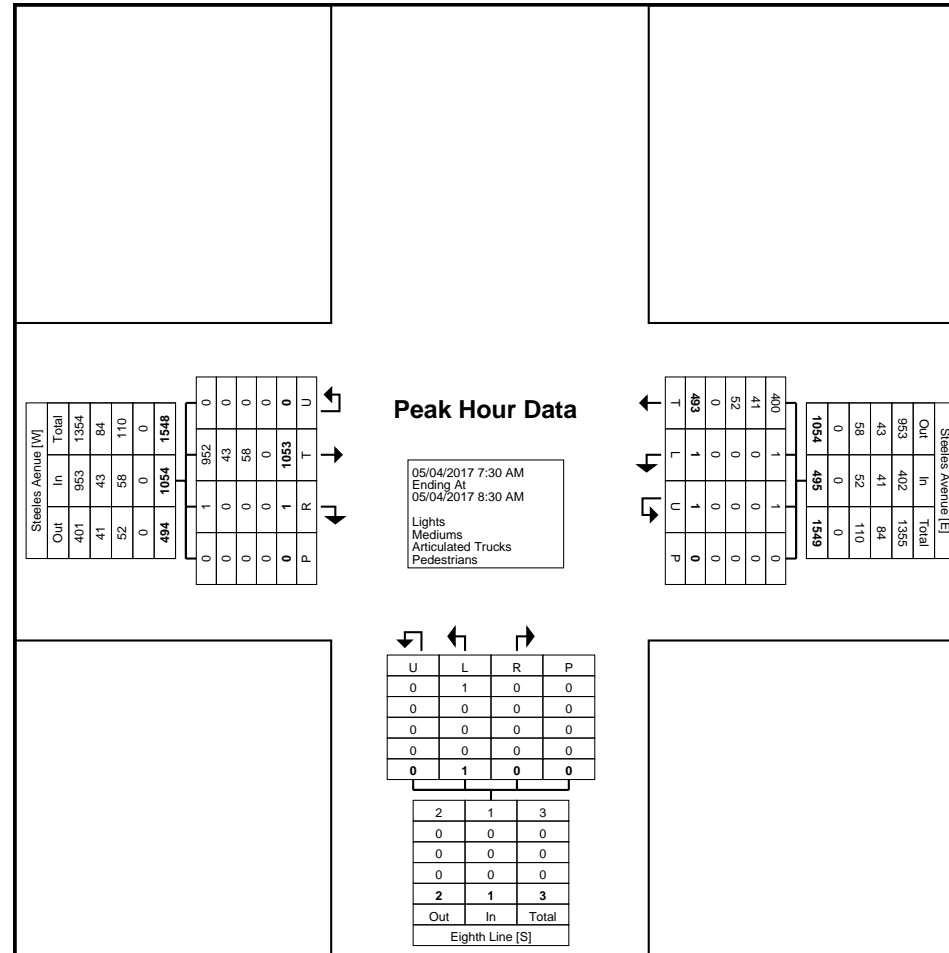




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Turning Movement Peak Hour Data Plot (7:30 AM)









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Count Name: Steeles Avenue & Eighth  
Line/Toronto Outlets - Saturday  
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### Turning Movement Data

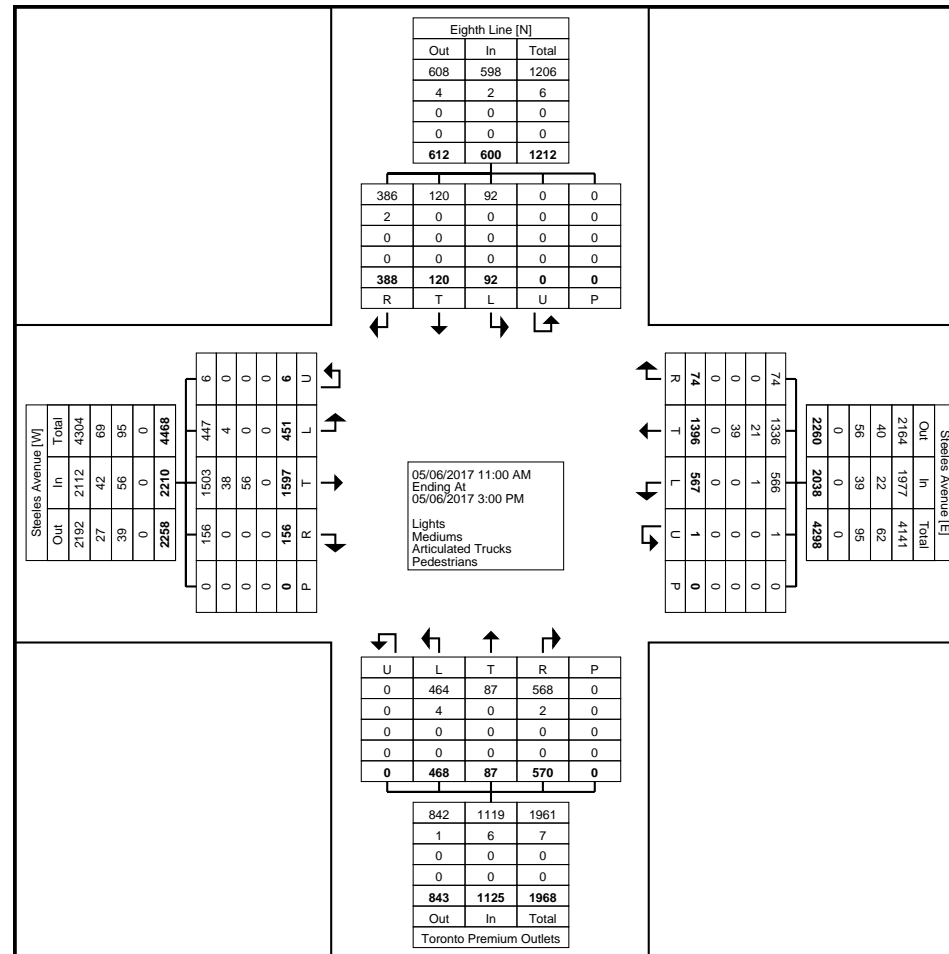
Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Toronto Premium Outlets Northbound						Eighth Line Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
11:00 AM	26	54	8	0	0	88	22	54	4	0	0	80	12	6	19	0	0	37	9	7	25	0	0	41	246
11:15 AM	21	93	8	0	0	122	29	68	5	0	0	102	13	4	26	0	0	43	4	4	22	0	0	30	297
11:30 AM	29	72	5	0	0	106	30	83	6	0	0	119	15	2	34	0	0	51	9	7	31	0	0	47	323
11:45 AM	27	93	17	0	0	137	33	96	3	0	0	132	20	6	23	0	0	49	1	7	22	0	0	30	348
Hourly Total	103	312	38	0	0	453	114	301	18	0	0	433	60	18	102	0	0	180	23	25	100	0	0	148	1214
12:00 PM	15	85	5	0	0	105	23	68	4	0	0	95	25	8	34	0	0	67	8	5	19	0	0	32	299
12:15 PM	36	94	12	1	0	143	48	103	1	0	0	152	21	2	26	0	0	49	2	5	28	0	0	35	379
12:30 PM	29	87	13	0	0	129	33	74	4	0	0	111	21	3	36	0	0	60	7	7	29	0	0	43	343
12:45 PM	31	111	23	0	0	165	40	72	6	0	0	118	37	9	47	0	0	93	3	16	26	0	0	45	421
Hourly Total	111	377	53	1	0	542	144	317	15	0	0	476	104	22	143	0	0	269	20	33	102	0	0	155	1442
1:00 PM	19	95	13	1	0	128	31	84	1	1	0	117	34	3	49	0	0	86	10	6	27	0	0	43	374
1:15 PM	34	121	9	1	0	165	27	101	8	0	0	136	28	5	19	0	0	52	11	7	22	0	0	40	393
1:30 PM	23	103	9	1	0	136	38	79	9	0	0	126	31	4	39	0	0	74	8	14	26	0	0	48	384
1:45 PM	33	95	6	0	0	134	55	107	4	0	0	166	47	6	45	0	0	98	4	8	23	0	0	35	433
Hourly Total	109	414	37	3	0	563	151	371	22	1	0	545	140	18	152	0	0	310	33	35	98	0	0	166	1584
2:00 PM	34	114	11	0	0	159	31	95	3	0	0	129	46	13	49	0	0	108	5	6	17	0	0	28	424
2:15 PM	33	125	4	2	0	164	40	106	7	0	0	153	35	7	34	0	0	76	5	7	27	0	0	39	432
2:30 PM	29	120	1	0	0	150	41	110	6	0	0	157	44	6	46	0	0	96	3	6	23	0	0	32	435
2:45 PM	32	135	12	0	0	179	46	96	3	0	0	145	39	3	44	0	0	86	3	8	21	0	0	32	442
Hourly Total	128	494	28	2	0	652	158	407	19	0	0	584	164	29	173	0	0	366	16	27	88	0	0	131	1733
Grand Total	451	1597	156	6	0	2210	567	1396	74	1	0	2038	468	87	570	0	0	1125	92	120	388	0	0	600	5973
Approach %	20.4	72.3	7.1	0.3	-	-	27.8	68.5	3.6	0.0	-	-	41.6	7.7	50.7	0.0	-	-	15.3	20.0	64.7	0.0	-	-	-
Total %	7.6	26.7	2.6	0.1	-	37.0	9.5	23.4	1.2	0.0	-	34.1	7.8	1.5	9.5	0.0	-	18.8	1.5	2.0	6.5	0.0	-	10.0	-
Lights	447	1503	156	6	-	2112	566	1336	74	1	-	1977	464	87	568	0	-	1119	92	120	386	0	-	598	5806
% Lights	99.1	94.1	100.0	100.0	-	95.6	99.8	95.7	100.0	100.0	-	97.0	99.1	100.0	99.6	-	-	99.5	100.0	100.0	99.5	-	-	99.7	97.2
Mediums	4	38	0	0	-	42	1	21	0	0	-	22	4	0	2	0	-	6	0	0	2	0	-	2	72
% Mediums	0.9	2.4	0.0	0.0	-	1.9	0.2	1.5	0.0	0.0	-	1.1	0.9	0.0	0.4	0	-	0.5	0.0	0.0	0.5	-	-	0.3	1.2
Articulated Trucks	0	56	0	0	-	56	0	39	0	0	-	39	0	0	0	0	-	0	0	0	0	0	-	0	95
% Articulated Trucks	0.0	3.5	0.0	0.0	-	2.5	0.0	2.8	0.0	0.0	-	1.9	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	1.6
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: Steeles Avenue & Eighth Line/Toronto Outlets - Saturday  
Site Code:  
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Turning Movement Data Plot

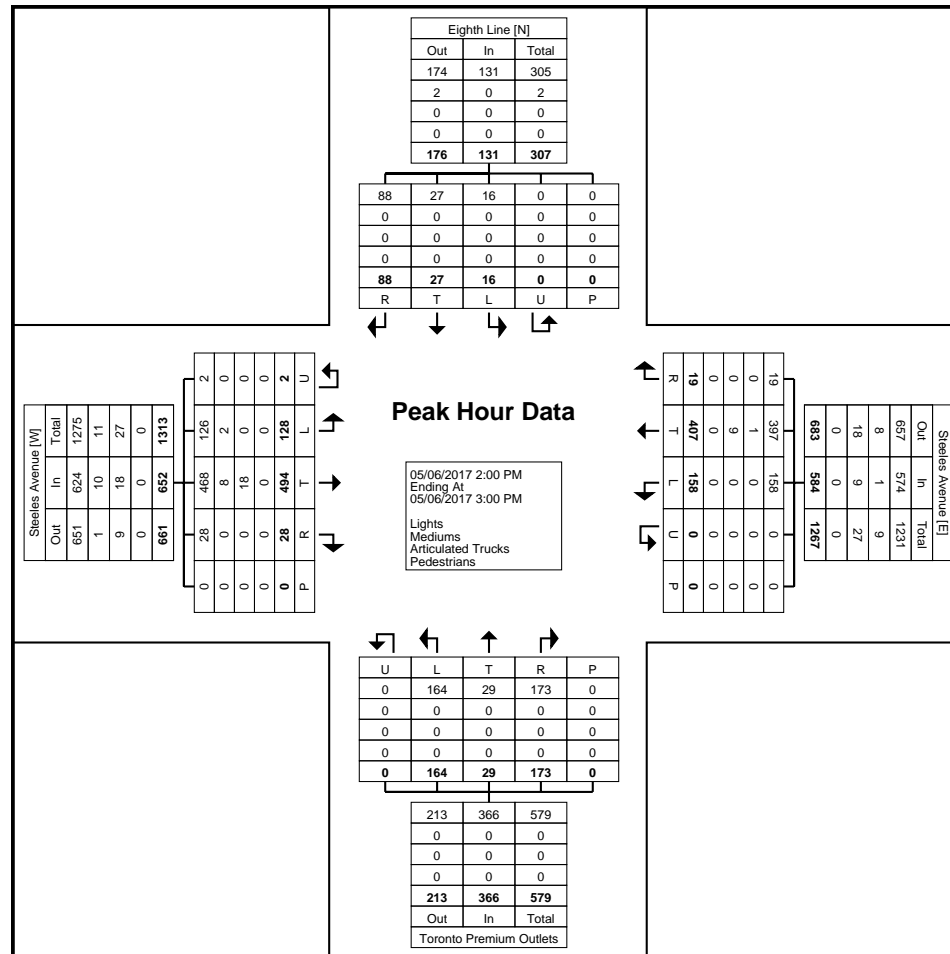




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Turning Movement Peak Hour Data Plot (2:00 PM)



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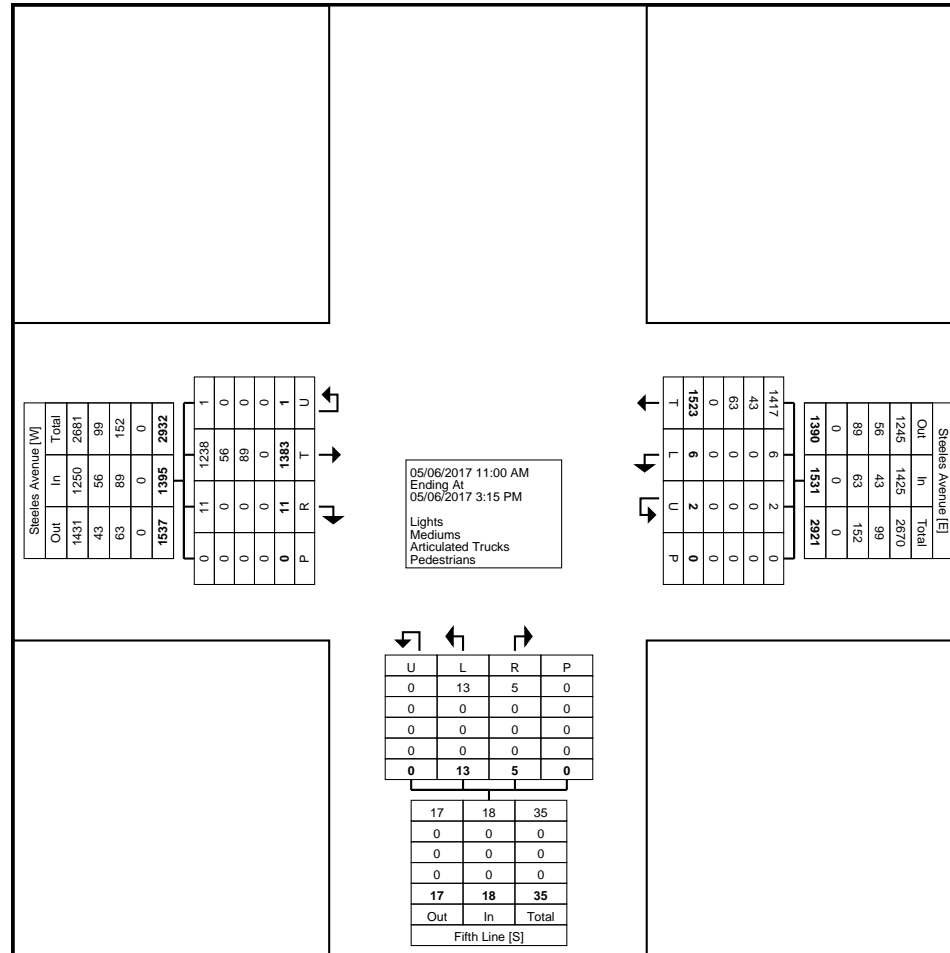




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Turning Movement Data Plot

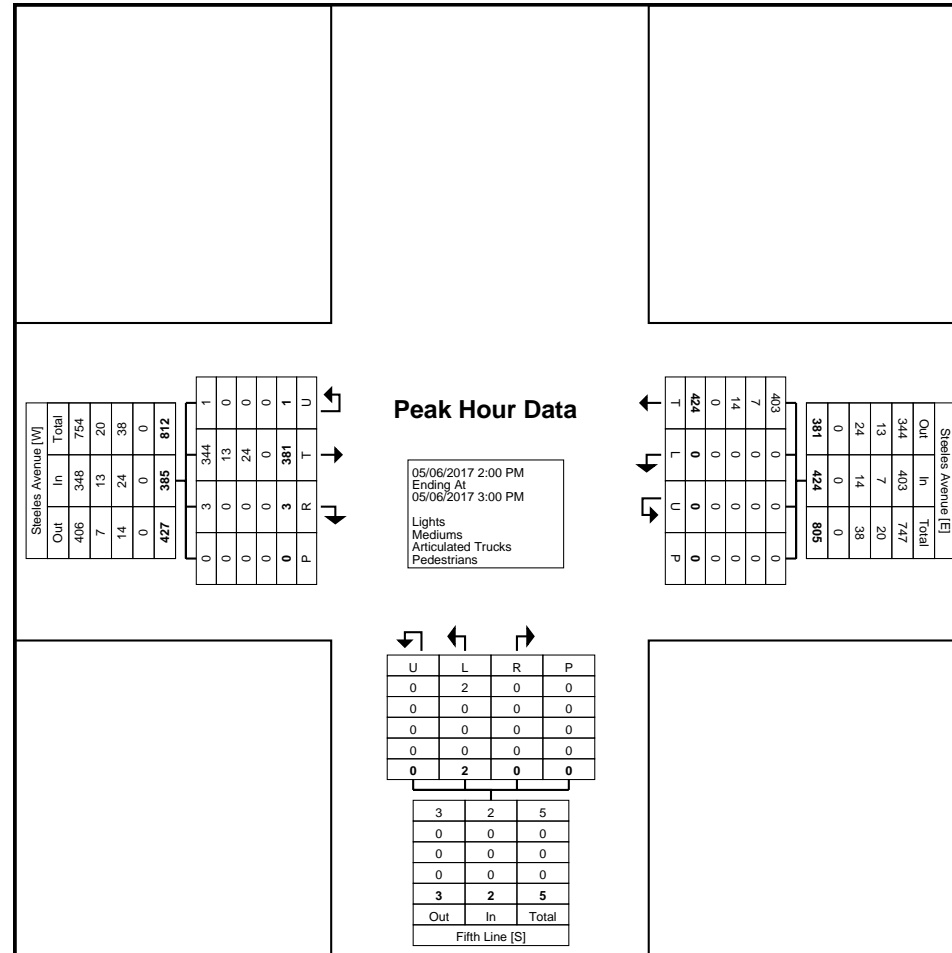




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Count Name: Steeles Avenue & Fifth Line  
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Turning Movement Peak Hour Data Plot (2:00 PM)



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Count Name: Steeles Avenue & Fifth Line  
(South) - Saturday  
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Start Date: 05/06/2017  
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Count Name: Steeles Avenue & Fifth Line  
(South) - Weekday  
Site Code:  
Start Date: 05/04/2017  
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### Turning Movement Data

Start Time	Steeles Avenue Eastbound					Steeles Avenue Westbound					Fifth Line Northbound					Int. Total
	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
6:00 AM	52	2	0	0	54	0	54	0	0	54	0	0	0	0	0	108
6:15 AM	62	2	0	0	64	1	82	0	0	83	0	0	0	0	0	147
6:30 AM	92	3	0	0	95	2	76	0	0	78	1	0	0	0	1	174
6:45 AM	97	10	0	0	107	3	78	0	0	81	0	0	0	0	0	188
Hourly Total	303	17	0	0	320	6	290	0	0	296	1	0	0	0	1	617
7:00 AM	149	3	0	0	152	1	89	0	0	90	0	0	0	0	0	242
7:15 AM	153	1	0	0	154	0	90	0	0	90	4	1	0	0	5	249
7:30 AM	209	1	0	0	210	1	126	0	0	127	0	0	0	0	0	337
7:45 AM	174	7	0	0	181	0	113	0	0	113	0	2	0	0	2	296
Hourly Total	685	12	0	0	697	2	418	0	0	420	4	3	0	0	7	1124
8:00 AM	190	4	0	0	194	3	111	0	0	114	2	1	0	0	3	311
8:15 AM	186	9	0	0	195	2	124	0	0	126	4	0	0	0	4	325
8:30 AM	164	3	0	0	167	6	114	0	0	120	2	2	0	0	4	291
8:45 AM	114	4	0	0	118	1	98	0	0	99	3	4	0	0	7	224
Hourly Total	654	20	0	0	674	12	447	0	0	459	11	7	0	0	18	1151
9:00 AM	102	3	0	0	105	0	105	0	0	105	1	1	0	0	2	212
9:15 AM	95	1	0	0	96	0	101	0	0	101	0	2	0	0	2	199
9:30 AM	73	3	0	0	76	0	83	1	0	84	1	0	0	0	1	161
9:45 AM	77	2	0	0	79	4	96	0	0	100	2	1	0	0	3	182
Hourly Total	347	9	0	0	356	4	385	1	0	390	4	4	0	0	8	754
10:00 AM	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	2
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	2
3:00 PM	134	3	0	0	137	0	156	0	0	156	1	3	0	0	4	297
3:15 PM	124	0	0	0	124	1	137	1	0	139	0	1	0	0	1	264
3:30 PM	125	1	0	0	126	1	158	0	0	159	3	0	0	0	3	288
3:45 PM	129	0	0	0	129	0	176	0	0	176	3	0	0	0	3	308
Hourly Total	512	4	0	0	516	2	627	1	0	630	7	4	0	0	11	1157
4:00 PM	127	3	0	0	130	0	214	0	0	214	7	2	0	0	9	353
4:15 PM	105	1	0	0	106	1	236	0	0	237	5	5	0	0	10	353
4:30 PM	138	0	0	0	138	1	227	0	0	228	7	3	0	0	10	376
4:45 PM	119	1	0	0	120	0	232	2	0	234	1	1	0	0	2	356
Hourly Total	489	5	0	0	494	2	909	2	0	913	20	11	0	0	31	1438
5:00 PM	142	0	0	0	142	1	222	0	0	223	6	2	0	0	8	373
5:15 PM	128	0	0	0	128	0	259	0	0	259	0	0	0	0	0	387
5:30 PM	95	0	0	0	95	0	232	1	0	233	0	1	0	0	1	329
5:45 PM	71	0	0	0	71	0	278	0	0	278	1	0	0	0	1	350
Hourly Total	436	0	0	0	436	1	991	1	0	993	7	3	0	0	10	1439

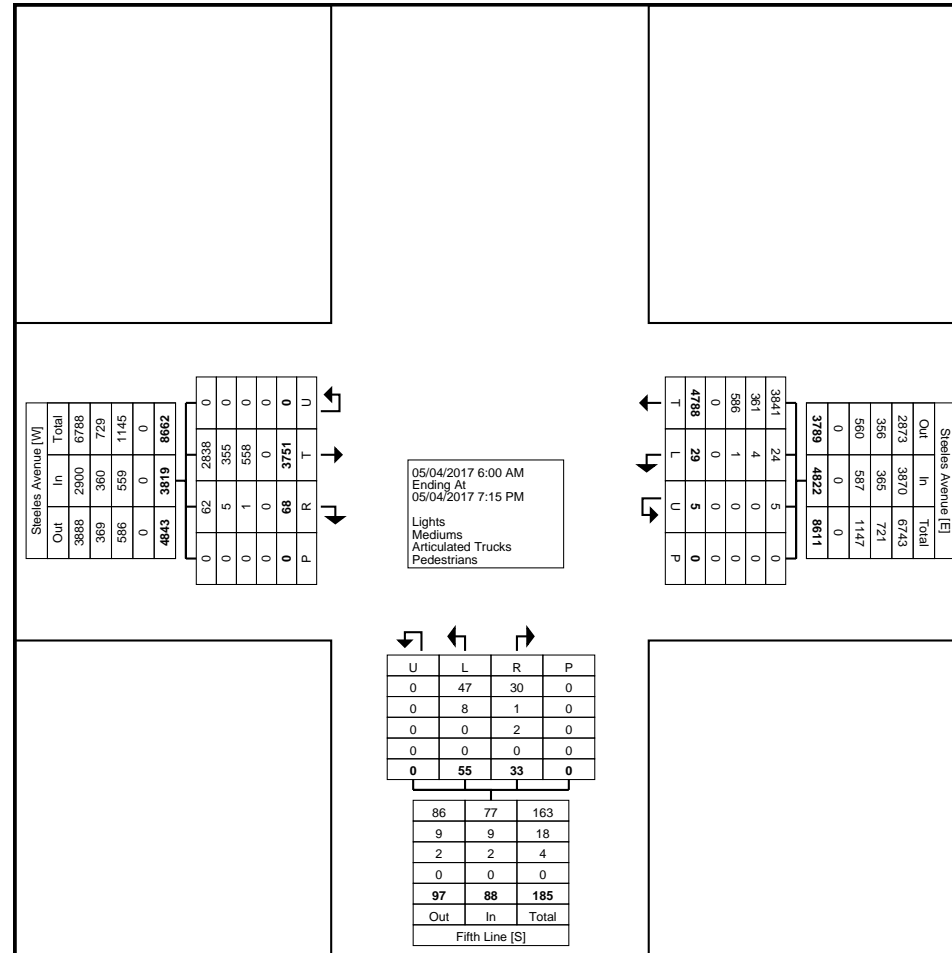




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Count Name: Steeles Avenue & Fifth Line  
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Turning Movement Data Plot



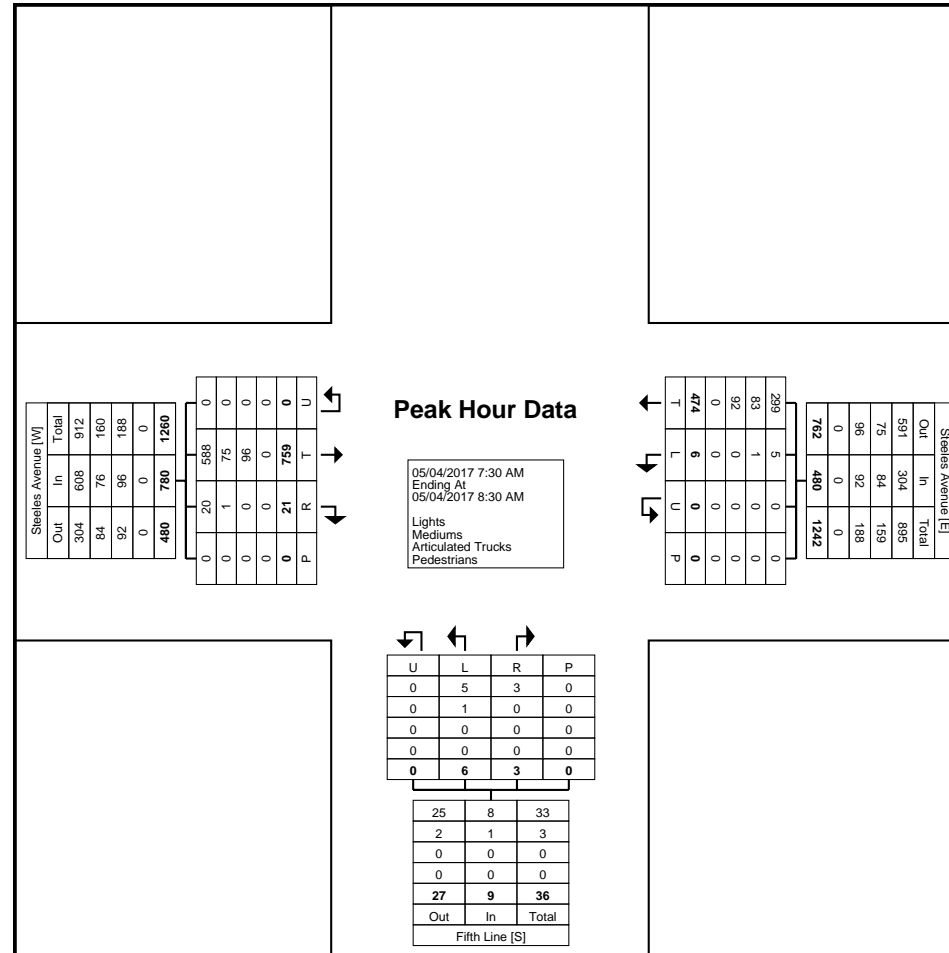




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Turning Movement Peak Hour Data Plot (7:30 AM)







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Count Name: Steeles Avenue & Fifth  
Line/Brownridge - Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 1

### Turning Movement Data

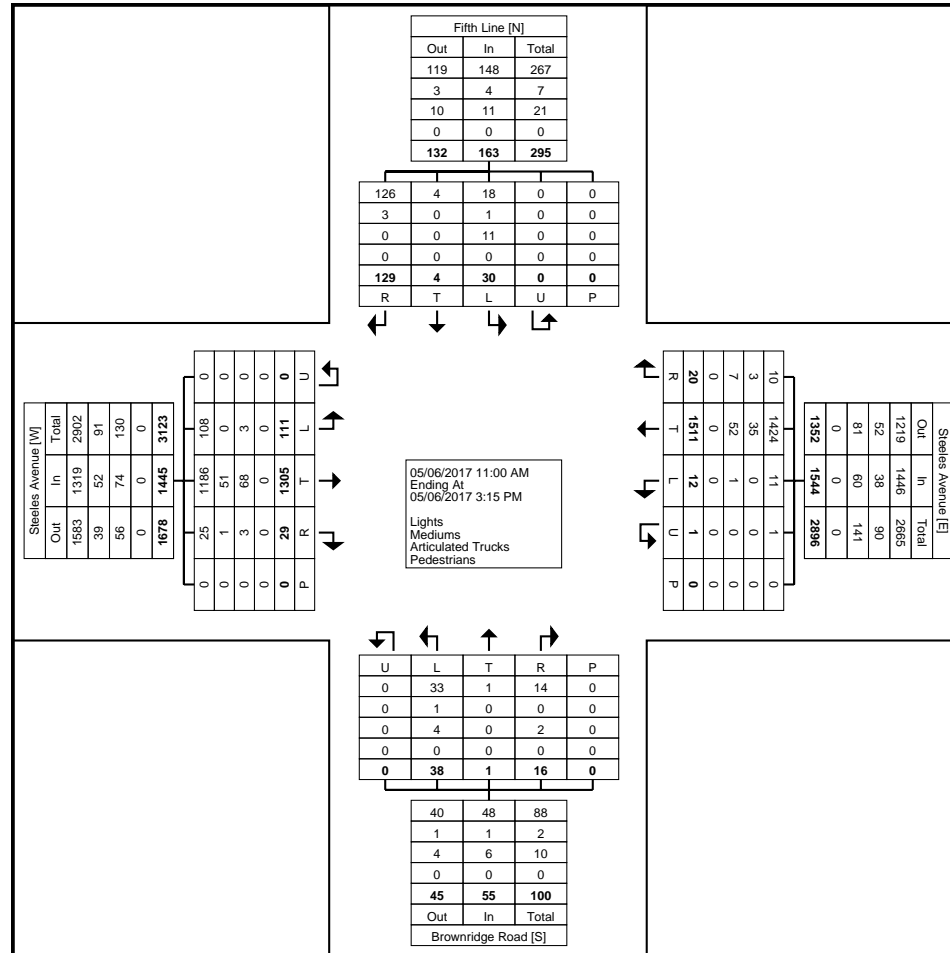
Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Brownridge Road Northbound						Fifth Line Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
11:00 AM	4	70	1	0	0	75	1	80	1	0	0	82	1	0	0	0	0	1	2	0	13	0	0	15	173
11:15 AM	10	57	2	0	0	69	2	79	1	0	0	82	3	0	1	0	0	4	4	0	6	0	0	10	165
11:30 AM	8	81	2	0	0	91	3	86	1	1	0	91	4	0	1	0	0	5	1	0	10	0	0	11	198
11:45 AM	7	74	1	0	0	82	0	87	1	0	0	88	2	0	1	0	0	3	0	0	6	0	0	6	179
Hourly Total	29	282	6	0	0	317	6	332	4	1	0	343	10	0	3	0	0	13	7	0	35	0	0	42	715
12:00 PM	4	76	4	0	0	84	0	72	1	0	0	73	4	0	0	0	0	4	2	1	4	0	0	7	168
12:15 PM	10	86	3	0	0	99	0	87	1	0	0	88	2	0	0	0	0	2	2	0	13	0	0	15	204
12:30 PM	7	86	1	0	0	94	0	77	1	0	0	78	1	0	1	0	0	2	1	0	6	0	0	7	181
12:45 PM	8	68	2	0	0	78	2	103	1	0	0	106	2	0	1	0	0	3	1	0	5	0	0	6	193
Hourly Total	29	316	10	0	0	355	2	339	4	0	0	345	9	0	2	0	0	11	6	1	28	0	0	35	746
1:00 PM	7	85	2	0	0	94	0	114	1	0	0	115	4	0	1	0	0	5	3	0	7	0	0	10	224
1:15 PM	8	91	0	0	0	99	1	103	5	0	0	109	1	0	1	0	0	2	1	0	10	0	0	11	221
1:30 PM	2	80	4	0	0	86	1	86	2	0	0	89	2	0	3	0	0	5	0	1	7	0	0	8	188
1:45 PM	9	87	3	0	0	99	0	102	1	0	0	103	2	0	1	0	0	3	1	0	8	0	0	9	214
Hourly Total	26	343	9	0	0	378	2	405	9	0	0	416	9	0	6	0	0	15	5	1	32	0	0	38	847
2:00 PM	7	100	1	0	0	108	0	108	1	0	0	109	5	1	2	0	0	8	2	1	6	0	0	9	234
2:15 PM	5	86	3	0	0	94	1	89	1	0	0	91	1	0	1	0	0	2	4	1	9	0	0	14	201
2:30 PM	5	91	0	0	0	96	1	125	0	0	0	126	3	0	0	0	0	3	2	0	8	0	0	10	235
2:45 PM	10	87	0	0	0	97	0	113	1	0	0	114	1	0	2	0	0	3	4	0	11	0	0	15	229
Hourly Total	27	364	4	0	0	395	2	435	3	0	0	440	10	1	5	0	0	16	12	2	34	0	0	48	899
3:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	111	1305	29	0	0	1445	12	1511	20	1	0	1544	38	1	16	0	0	55	30	4	129	0	0	163	3207
Approach %	7.7	90.3	2.0	0.0	-	-	0.8	97.9	1.3	0.1	-	-	69.1	1.8	29.1	0.0	-	-	18.4	2.5	79.1	0.0	-	-	-
Total %	3.5	40.7	0.9	0.0	-	45.1	0.4	47.1	0.6	0.0	-	48.1	1.2	0.0	0.5	0.0	-	1.7	0.9	0.1	4.0	0.0	-	5.1	-
Lights	108	1186	25	0	-	1319	11	1424	10	1	-	1446	33	1	14	0	-	48	18	4	126	0	-	148	2961
% Lights	97.3	90.9	86.2	-	-	91.3	91.7	94.2	50.0	100.0	-	93.7	86.8	100.0	87.5	-	-	87.3	60.0	100.0	97.7	-	-	90.8	92.3
Mediums	0	51	1	0	-	52	0	35	3	0	-	38	1	0	0	0	-	1	1	0	3	0	-	4	95
% Mediums	0.0	3.9	3.4	-	-	3.6	0.0	2.3	15.0	0.0	-	2.5	2.6	0.0	0.0	-	-	1.8	3.3	0.0	2.3	-	-	2.5	3.0
Articulated Trucks	3	68	3	0	-	74	1	52	7	0	-	60	4	0	2	0	-	6	11	0	0	0	-	11	151
% Articulated Trucks	2.7	5.2	10.3	-	-	5.1	8.3	3.4	35.0	0.0	-	3.9	10.5	0.0	12.5	-	-	10.9	36.7	0.0	0.0	-	-	6.7	4.7
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: Steeles Avenue & Fifth  
Line/Brownridge - Saturday  
Site Code:  
Start Date: 05/06/2017  
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Turning Movement Data Plot

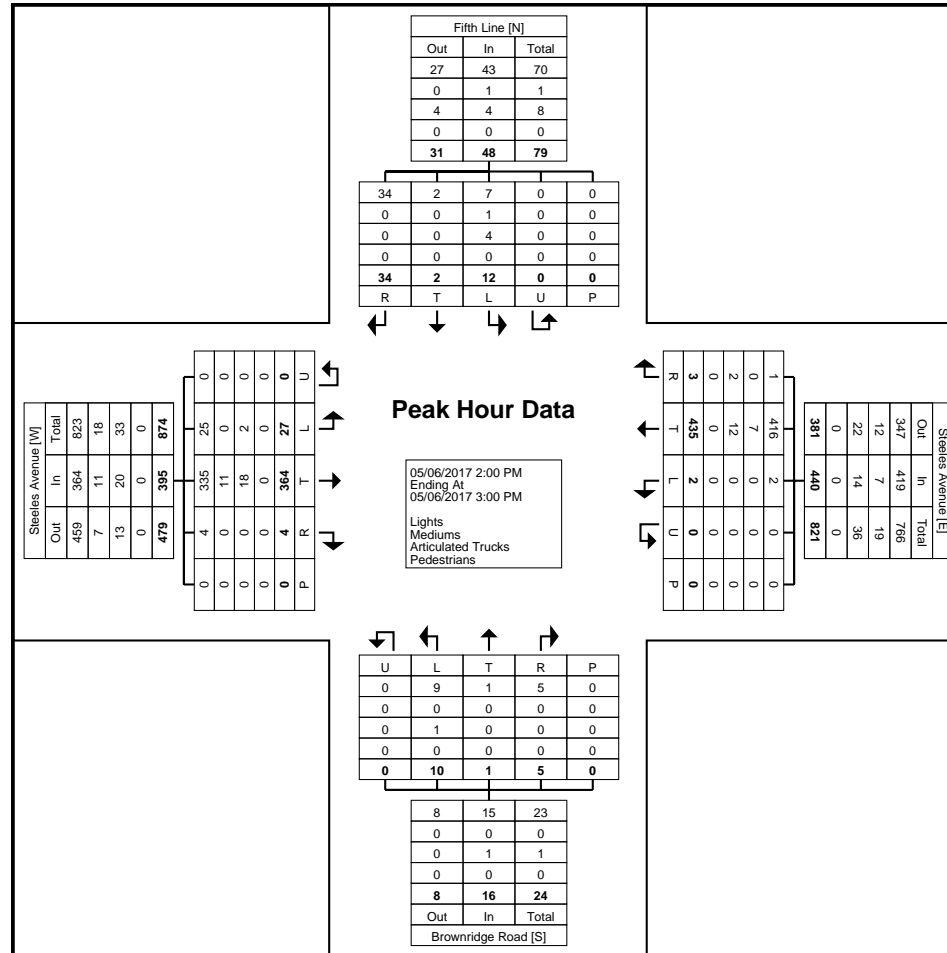




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Turning Movement Peak Hour Data Plot (2:00 PM)





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Count Name: Steeles Avenue & Fifth  
Line/Brownridge - Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 1

### Turning Movement Data

Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Brownridge Road Northbound						Fifth Line Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
6:00 AM	7	45	2	0	0	54	0	56	5	0	0	61	1	0	1	0	0	2	2	0	2	0	0	4	121
6:15 AM	11	58	2	0	0	71	3	74	5	0	0	82	0	0	0	0	0	0	3	0	9	0	0	12	165
6:30 AM	12	87	7	0	0	106	1	63	6	0	0	70	0	0	0	0	0	0	2	0	4	0	0	6	182
6:45 AM	13	107	6	0	0	126	1	72	8	0	0	81	1	0	0	0	0	1	4	0	7	0	0	11	219
Hourly Total	43	297	17	0	0	357	5	265	24	0	0	294	2	0	1	0	0	3	11	0	22	0	0	33	687
7:00 AM	22	149	6	0	1	177	1	78	9	0	0	88	3	0	2	0	0	5	4	0	9	0	0	13	283
7:15 AM	36	148	9	0	0	193	2	87	6	0	0	95	1	0	0	0	0	1	4	1	18	0	0	23	312
7:30 AM	25	189	12	0	0	226	3	105	7	0	0	115	2	0	3	0	0	5	6	0	14	0	0	20	366
7:45 AM	41	183	18	0	0	242	8	109	5	0	0	122	2	0	1	0	1	3	4	3	16	0	0	23	390
Hourly Total	124	669	45	0	1	838	14	379	27	0	0	420	8	0	6	0	1	14	18	4	57	0	0	79	1351
8:00 AM	33	185	6	0	0	224	2	98	9	1	0	110	5	0	1	0	0	6	4	2	15	0	0	21	361
8:15 AM	36	170	15	1	0	222	3	108	16	0	0	127	0	0	0	0	0	0	6	1	18	0	0	25	374
8:30 AM	28	146	11	0	0	185	2	101	7	0	0	110	4	0	3	0	0	7	5	0	16	0	0	21	323
8:45 AM	11	106	8	0	0	125	4	90	4	0	0	98	3	0	0	0	0	3	7	2	13	0	0	22	248
Hourly Total	108	607	40	1	0	756	11	397	36	1	0	445	12	0	4	0	0	16	22	5	62	0	0	89	1306
9:00 AM	10	90	10	0	0	110	1	101	3	0	0	105	1	0	2	0	0	3	5	0	13	0	0	18	236
9:15 AM	9	95	11	0	0	115	0	87	7	0	0	94	4	0	2	0	0	6	6	0	9	0	0	15	230
9:30 AM	9	76	5	0	0	90	0	91	3	0	0	94	3	0	0	0	0	3	3	1	17	0	0	21	208
9:45 AM	12	67	8	0	0	87	5	90	3	0	0	98	9	1	1	0	0	11	3	1	9	0	0	13	209
Hourly Total	40	328	34	0	0	402	6	369	16	0	0	391	17	1	5	0	0	23	17	2	48	0	0	67	883
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	18	110	5	0	0	133	3	132	5	0	0	140	5	1	3	0	0	9	13	1	22	0	0	36	318
3:15 PM	19	113	6	0	0	138	2	152	2	0	0	156	11	1	4	0	0	16	4	0	17	0	0	21	331
3:30 PM	17	120	3	0	0	140	0	156	5	0	0	161	5	1	2	0	0	8	3	1	10	0	0	14	323
3:45 PM	11	107	7	0	0	125	4	166	3	0	0	173	7	1	7	0	0	15	5	2	15	0	0	22	335
Hourly Total	65	450	21	0	0	536	9	606	15	0	0	630	28	4	16	0	0	48	25	4	64	0	0	93	1307
4:00 PM	7	115	5	0	0	127	0	228	7	0	0	235	6	2	5	0	0	13	8	0	36	0	0	44	419
4:15 PM	12	97	3	0	0	112	0	239	2	0	0	241	3	1	6	0	0	10	4	0	26	0	0	30	393
4:30 PM	12	114	1	0	0	127	0	232	3	0	0	235	7	1	4	0	0	12	14	2	39	0	0	55	429
4:45 PM	12	108	1	0	0	121	2	214	3	0	0	219	6	1	6	0	0	13	9	0	22	0	0	31	384
Hourly Total	43	434	10	0	0	487	2	913	15	0	0	930	22	5	21	0	0	48	35	2	123	0	0	160	1625
5:00 PM	14	114	1	0	0	129	0	228	4	0	0	232	16	3	10	0	0	29	13	1	19	0	0	33	423
5:15 PM	9	111	0	0	0	120	0	247	1	0	0	248	14	2	3	0	0	19	6	0	21	0	0	27	414
5:30 PM	14	80	0	0	0	94	0	225	3	0	0	228	7	0	2	0	0	9	2	0	13	0	0	15	346
5:45 PM	7	63	1	0	0	71	0	268	3	0	0	271	3	0	3	0	0	6	2	0	21	0	0	23	371

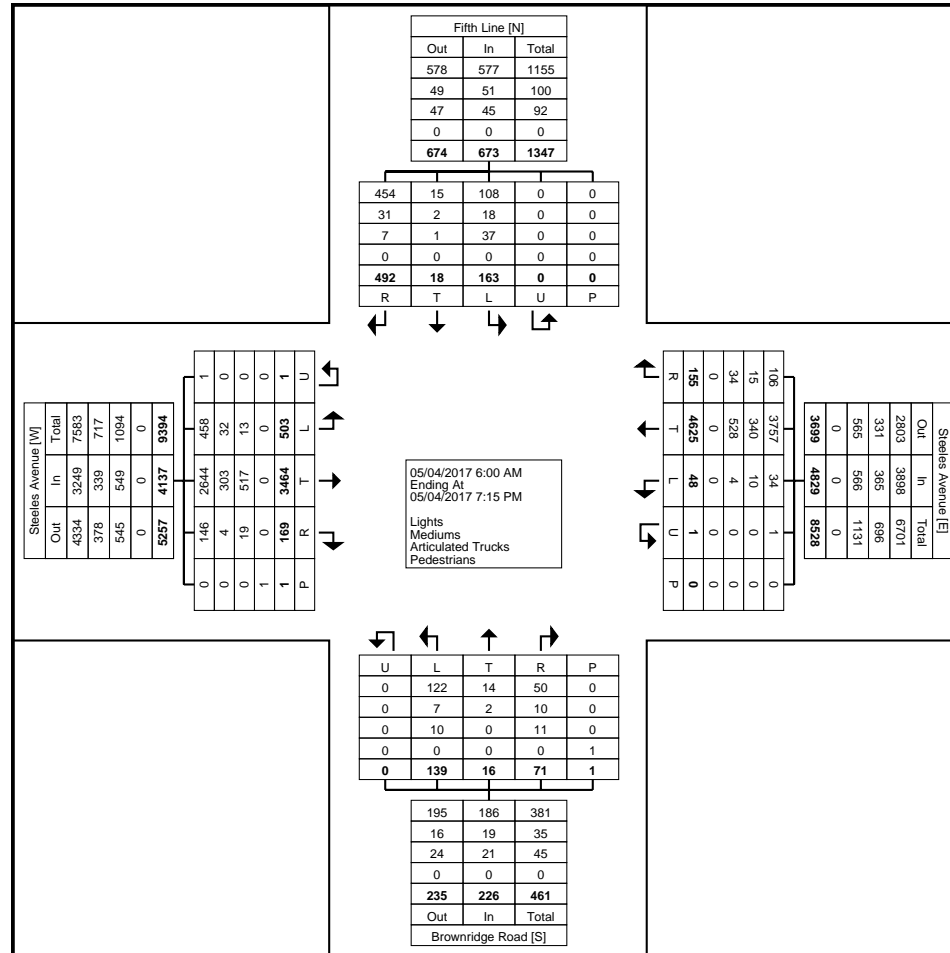




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Count Name: Steeles Avenue & Fifth  
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Turning Movement Data Plot

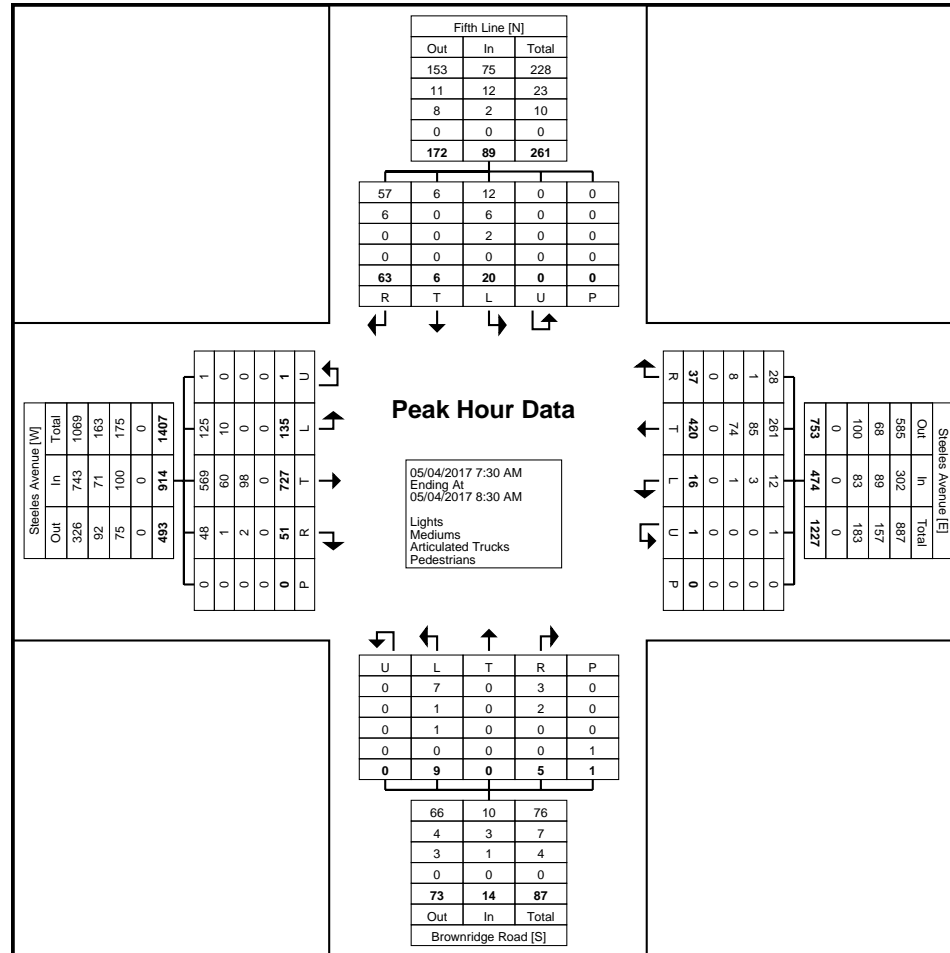




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Turning Movement Peak Hour Data Plot (7:30 AM)

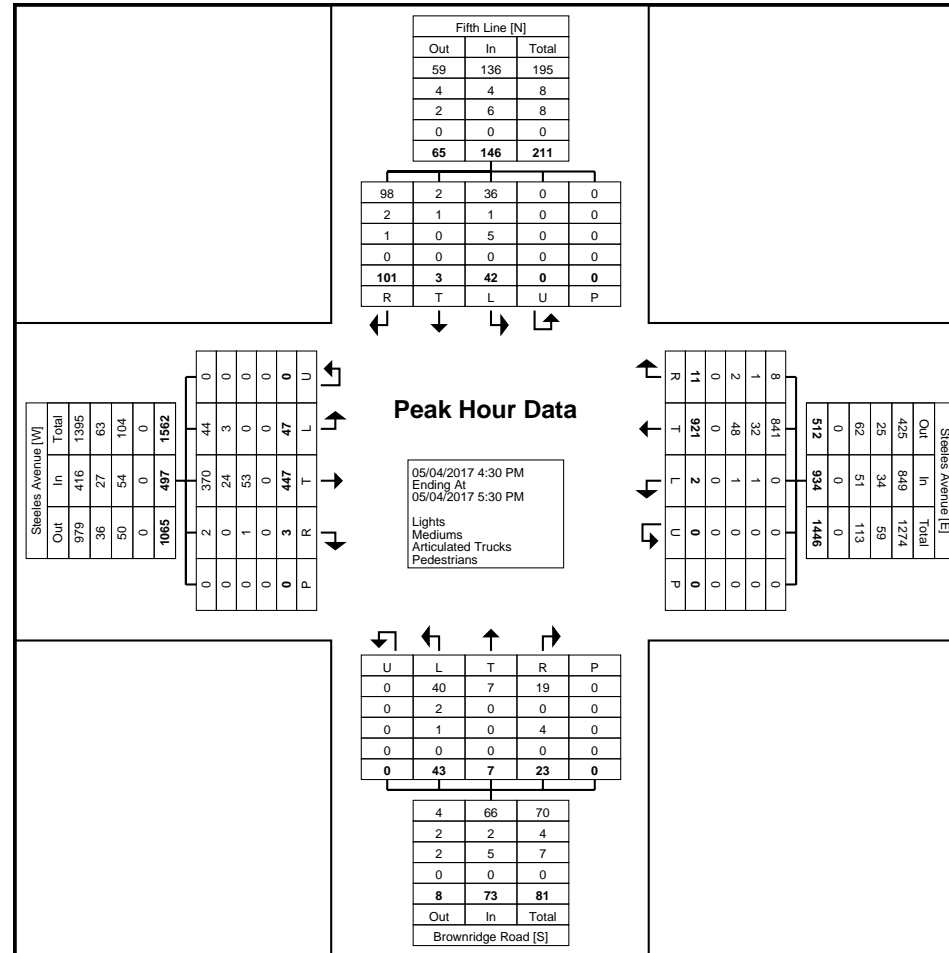




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Count Name: Steeles Avenue & Fifth  
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Page No: 7



Turning Movement Peak Hour Data Plot (4:30 PM)





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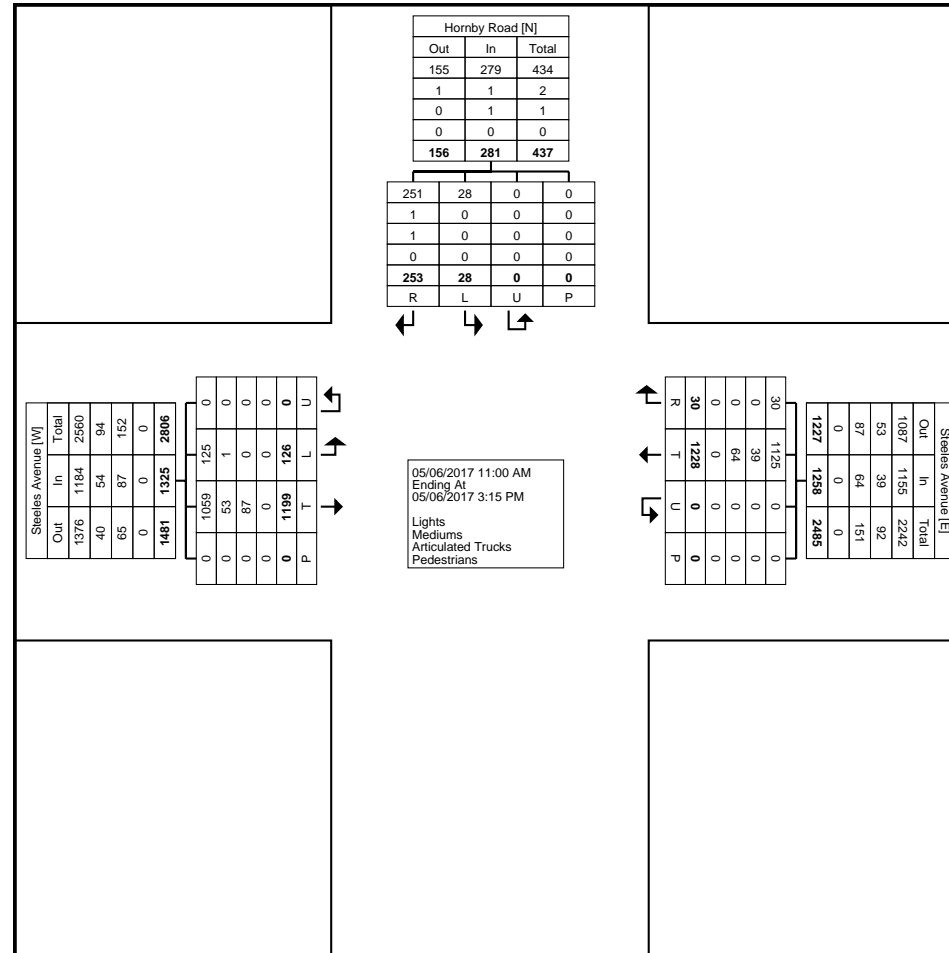




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Count Name: Steeles Avenue & Hornby Road - Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 2



Turning Movement Data Plot

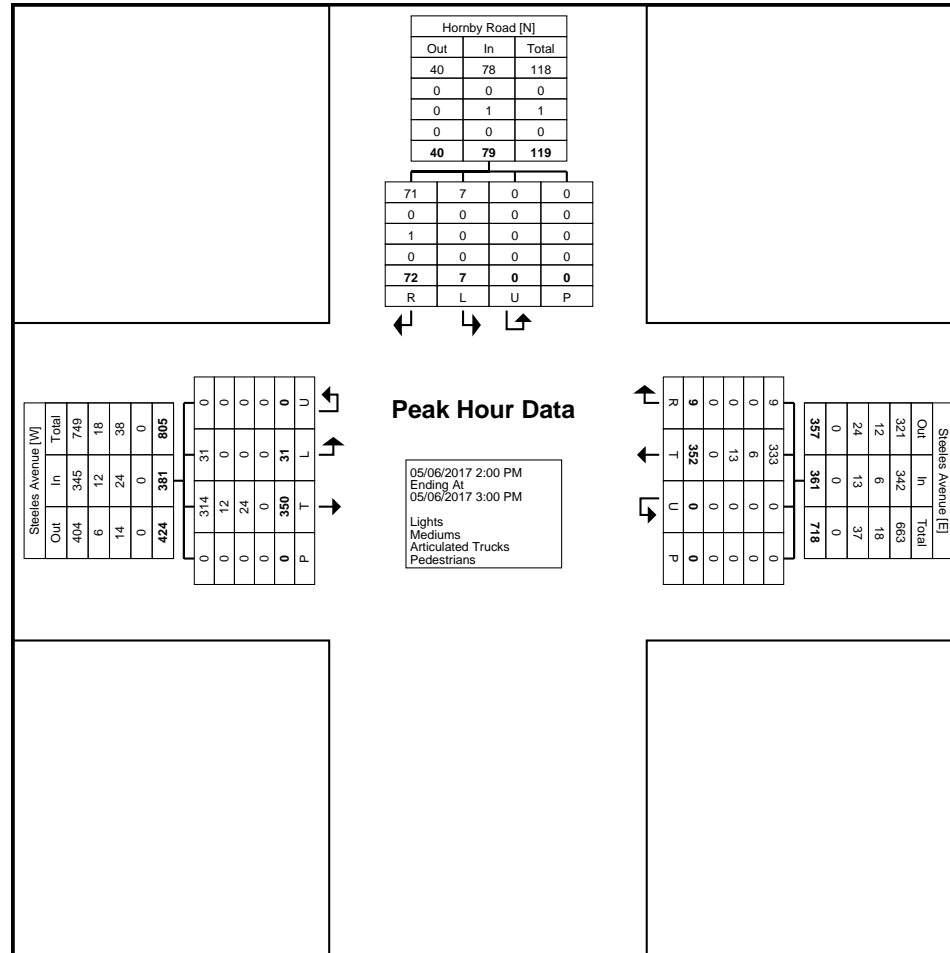




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Count Name: Steeles Avenue & Hornby Road - Saturday  
Site Code:  
Start Date: 05/06/2017  
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Turning Movement Peak Hour Data Plot (2:00 PM)



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Count Name: Steeles Avenue & Hornby Road -  
Saturday  
Site Code:  
Start Date: 05/06/2017  
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Count Name: Steeles Avenue & Hornby Road -  
Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 1

### Turning Movement Data

Start Time	Steeles Avenue Eastbound					Steeles Avenue Westbound					Hornby Road Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
6:00 AM	2	51	0	0	53	54	1	0	0	55	2	4	0	0	6	114
6:15 AM	4	58	0	0	62	81	6	0	0	87	0	6	0	0	6	155
6:30 AM	6	74	0	0	80	81	0	0	0	81	1	3	0	0	4	165
6:45 AM	3	102	0	0	105	84	1	0	0	85	3	6	0	0	9	199
Hourly Total	15	285	0	0	300	300	8	0	0	308	6	19	0	0	25	633
7:00 AM	0	114	0	0	114	82	4	0	0	86	1	9	0	0	10	210
7:15 AM	7	161	0	0	168	87	3	0	0	90	4	7	0	0	11	269
7:30 AM	2	191	0	0	193	110	1	0	0	111	4	5	0	0	9	313
7:45 AM	4	174	0	0	178	113	1	0	0	114	2	7	0	0	9	301
Hourly Total	13	640	0	0	653	392	9	0	0	401	11	28	0	0	39	1093
8:00 AM	1	179	0	0	180	100	4	0	0	104	3	5	0	0	8	292
8:15 AM	2	178	0	0	180	113	5	0	0	118	1	4	0	0	5	303
8:30 AM	7	188	0	0	195	100	5	0	0	105	0	14	0	0	14	314
8:45 AM	9	116	0	0	125	88	7	0	0	95	1	9	0	0	10	230
Hourly Total	19	661	0	0	680	401	21	0	0	422	5	32	0	0	37	1139
9:00 AM	5	96	0	0	101	94	1	0	0	95	4	8	0	0	12	208
9:15 AM	2	79	0	0	81	87	0	0	0	87	3	10	0	0	13	181
9:30 AM	4	87	0	0	91	74	7	0	0	81	2	8	0	0	10	182
9:45 AM	1	71	0	0	72	92	0	0	0	92	7	9	0	0	16	180
Hourly Total	12	333	0	0	345	347	8	0	0	355	16	35	0	0	51	751
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	6	124	0	0	130	133	4	0	0	137	1	12	0	0	13	280
3:15 PM	7	116	0	0	123	142	2	0	0	144	5	6	0	0	11	278
3:30 PM	3	119	0	0	122	140	3	0	0	143	2	11	0	0	13	278
3:45 PM	4	115	0	0	119	183	3	0	0	186	1	13	0	0	14	319
Hourly Total	20	474	0	0	494	598	12	0	0	610	9	42	0	0	51	1155
4:00 PM	7	110	0	0	117	205	2	0	0	207	1	11	0	0	12	336
4:15 PM	7	101	0	0	108	208	3	0	0	211	1	20	0	0	21	340
4:30 PM	5	135	0	0	140	220	4	0	0	224	1	10	0	0	11	375
4:45 PM	9	105	0	0	114	212	4	0	0	216	1	16	0	0	17	347
Hourly Total	28	451	0	0	479	845	13	0	0	858	4	57	0	0	61	1398
5:00 PM	12	114	0	0	126	224	2	0	0	226	1	10	0	0	11	363
5:15 PM	4	123	0	0	127	245	4	0	0	249	0	14	0	0	14	390
5:30 PM	6	88	0	0	94	225	1	0	0	226	1	12	0	0	13	333
5:45 PM	4	72	0	0	76	250	2	0	0	252	1	19	0	0	20	348
Hourly Total	26	397	0	0	423	944	9	0	0	953	3	55	0	0	58	1434







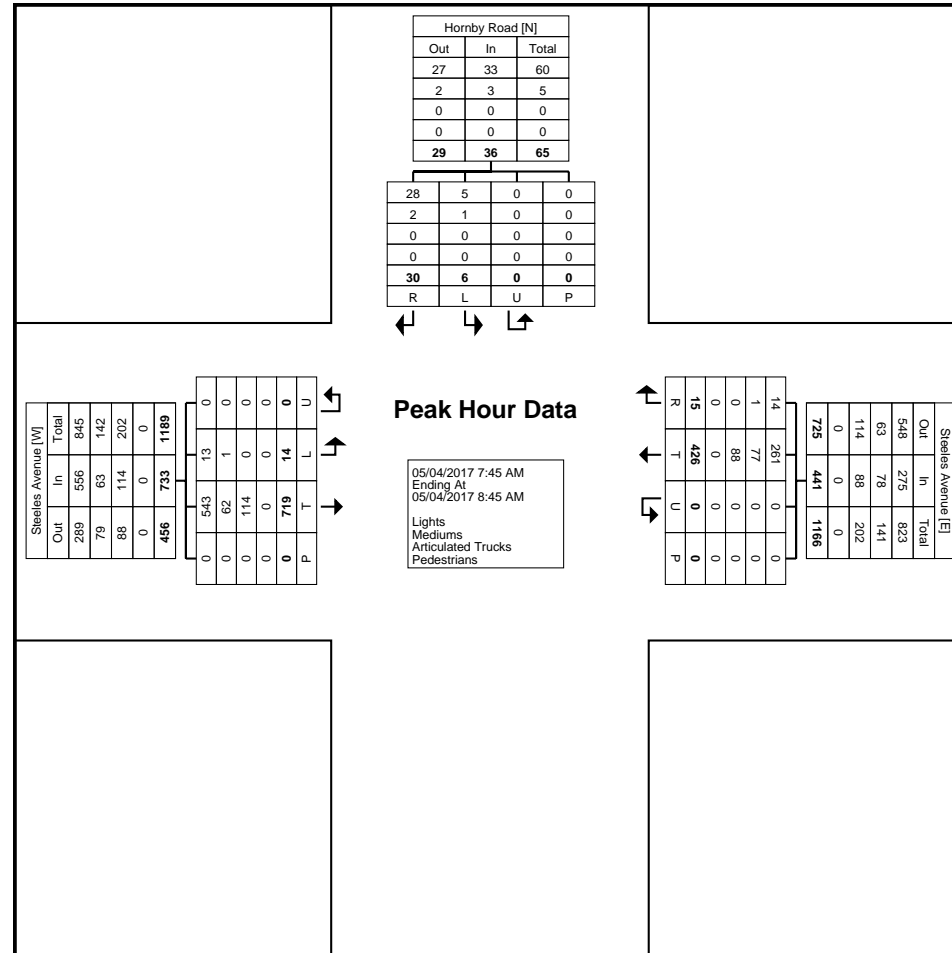




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Count Name: Steeles Avenue & Hornby Road - Weekday  
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Turning Movement Peak Hour Data Plot (7:45 AM)

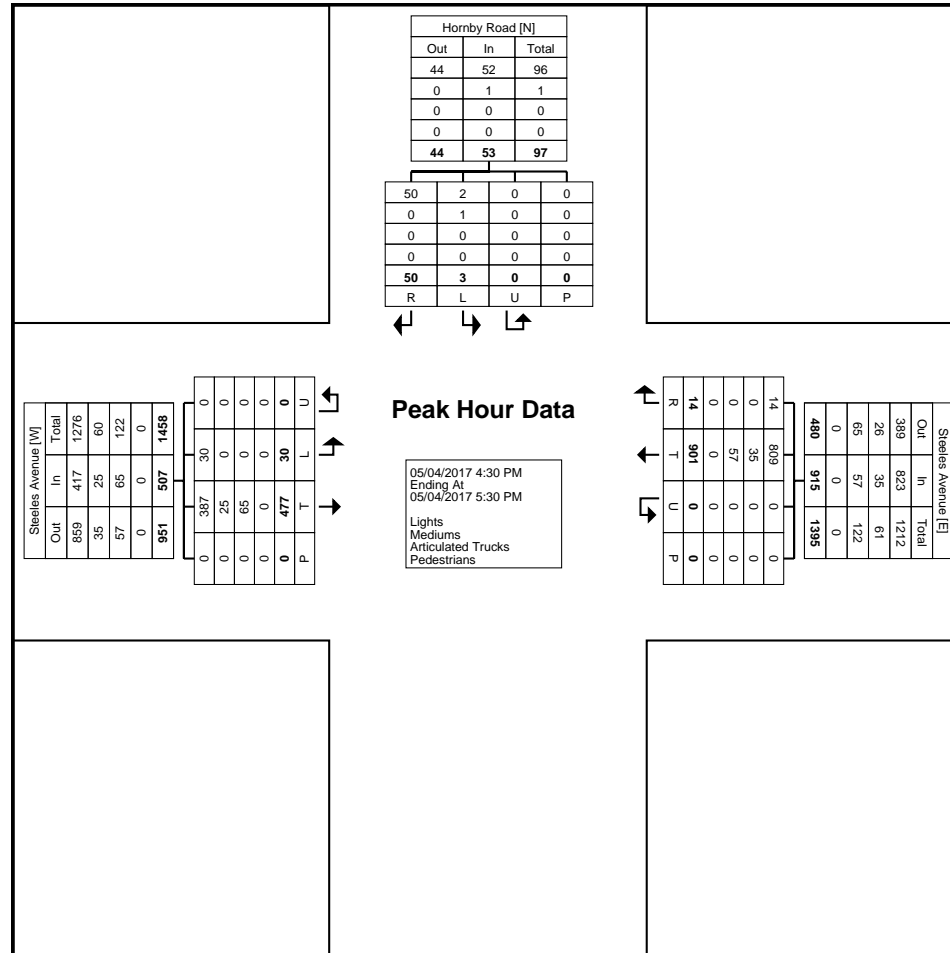




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Turning Movement Peak Hour Data Plot (4:30 PM)



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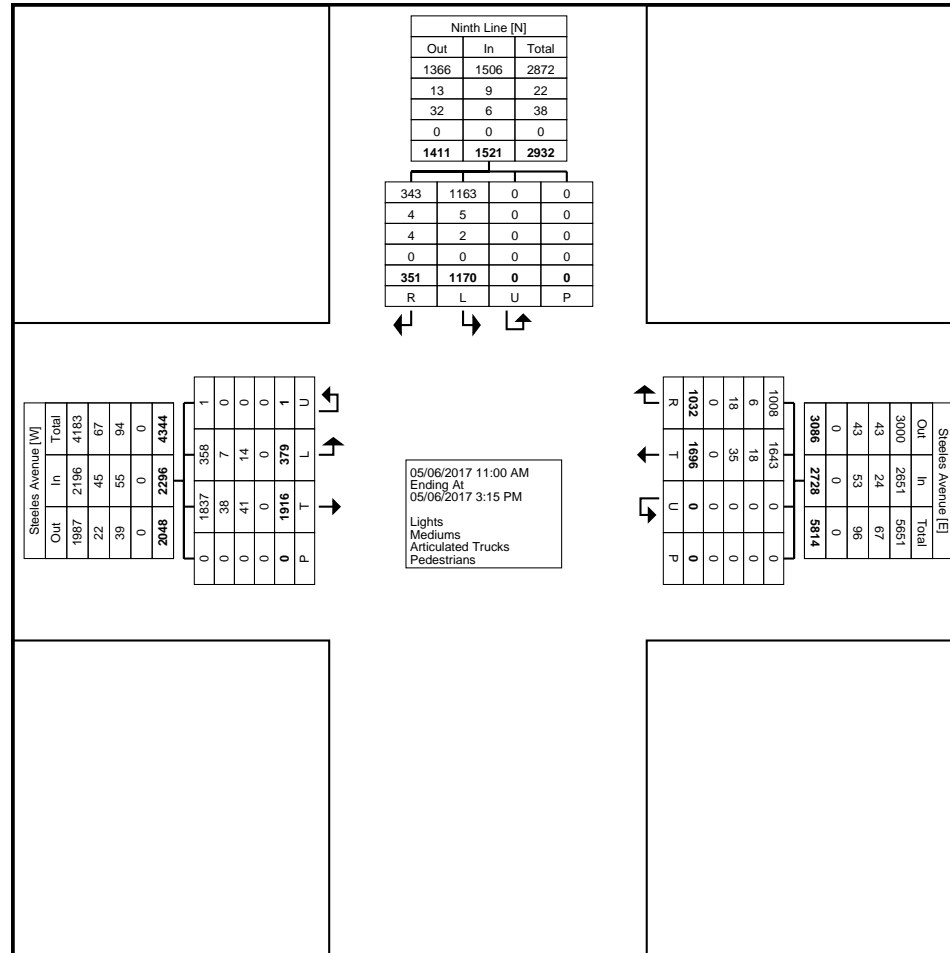




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Count Name: Steeles Avenue & Ninth Line  
(North) - Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 2



Turning Movement Data Plot



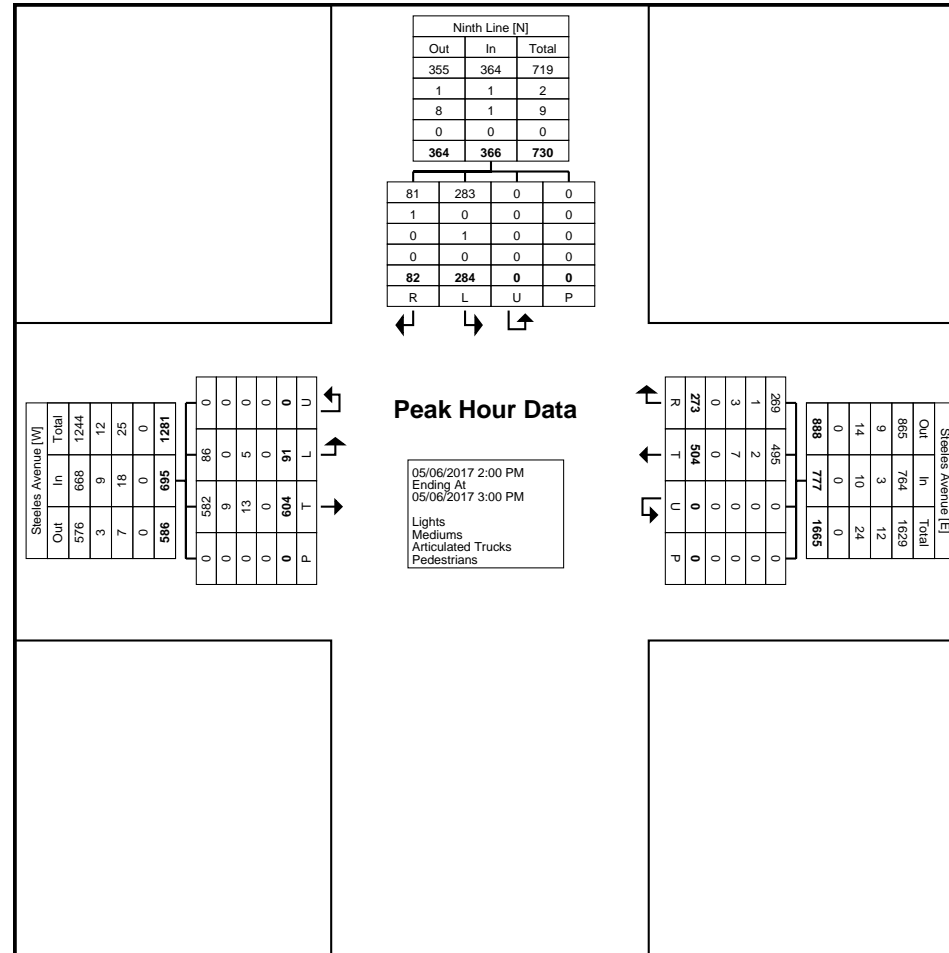




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Count Name: Steeles Avenue & Ninth Line  
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Turning Movement Peak Hour Data Plot (2:00 PM)



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Count Name: Steeles Avenue & Ninth Line  
(North) - Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 1

### Turning Movement Data

Start Time	Steeles Avenue Eastbound					Steeles Avenue Westbound					Ninth Line Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
6:00 AM	15	85	0	0	100	72	12	0	0	84	79	20	0	0	99	283
6:15 AM	13	89	0	0	102	79	14	0	0	93	117	14	0	0	131	326
6:30 AM	13	134	0	0	147	99	31	0	0	130	117	18	0	0	135	412
6:45 AM	17	135	0	0	152	88	35	0	0	123	127	13	0	0	140	415
Hourly Total	58	443	0	0	501	338	92	0	0	430	440	65	0	0	505	1436
7:00 AM	20	183	0	0	203	109	22	0	0	131	160	16	0	0	176	510
7:15 AM	16	218	0	0	234	93	39	0	0	132	161	20	0	0	181	547
7:30 AM	13	249	0	0	262	112	41	0	0	153	142	18	0	0	160	575
7:45 AM	24	256	0	0	280	106	52	0	0	158	160	24	0	0	184	622
Hourly Total	73	906	0	0	979	420	154	0	0	574	623	78	0	0	701	2254
8:00 AM	21	247	0	0	268	106	62	0	0	168	125	12	0	0	137	573
8:15 AM	17	249	0	0	266	121	58	0	0	179	141	19	0	0	160	605
8:30 AM	19	191	0	0	210	118	52	0	0	170	135	15	0	0	150	530
8:45 AM	26	159	0	0	185	100	40	0	0	140	102	14	0	0	116	441
Hourly Total	83	846	0	0	929	445	212	0	0	657	503	60	0	0	563	2149
9:00 AM	21	115	0	0	136	89	38	0	0	127	110	21	0	0	131	394
9:15 AM	20	108	0	0	128	71	35	0	0	106	94	20	0	0	114	348
9:30 AM	25	90	0	0	115	97	35	0	0	132	88	17	0	0	105	352
9:45 AM	28	72	0	0	100	93	29	0	0	122	71	25	0	0	96	318
Hourly Total	94	385	0	0	479	350	137	0	0	487	363	83	0	0	446	1412
10:00 AM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1
3:00 PM	18	121	0	0	139	128	97	0	0	225	51	17	0	0	68	432
3:15 PM	28	152	0	0	180	161	110	0	0	271	46	19	0	0	65	516
3:30 PM	20	143	1	0	164	217	123	0	0	340	63	20	0	0	83	587
3:45 PM	25	142	0	0	167	291	139	0	0	430	69	28	0	0	97	694
Hourly Total	91	558	1	0	650	797	469	0	0	1266	229	84	0	0	313	2229
4:00 PM	37	160	1	0	198	294	163	0	0	457	52	11	0	0	63	718
4:15 PM	21	134	0	0	155	335	134	0	0	469	81	15	0	0	96	720
4:30 PM	28	147	0	0	175	360	156	0	0	516	80	17	0	0	97	788
4:45 PM	25	143	0	0	168	328	186	0	0	514	61	20	0	0	81	763
Hourly Total	111	584	1	0	696	1317	639	0	0	1956	274	63	0	0	337	2989
5:00 PM	26	191	0	0	217	298	188	0	0	486	59	13	0	0	72	775
5:15 PM	21	183	0	0	204	331	169	0	0	500	66	17	0	0	83	787
5:30 PM	14	151	0	0	165	335	169	0	0	504	63	18	0	0	81	750
5:45 PM	27	123	0	0	150	276	187	0	0	463	55	14	0	0	69	682
Hourly Total	88	648	0	0	736	1240	713	0	0	1953	243	62	0	0	305	2994

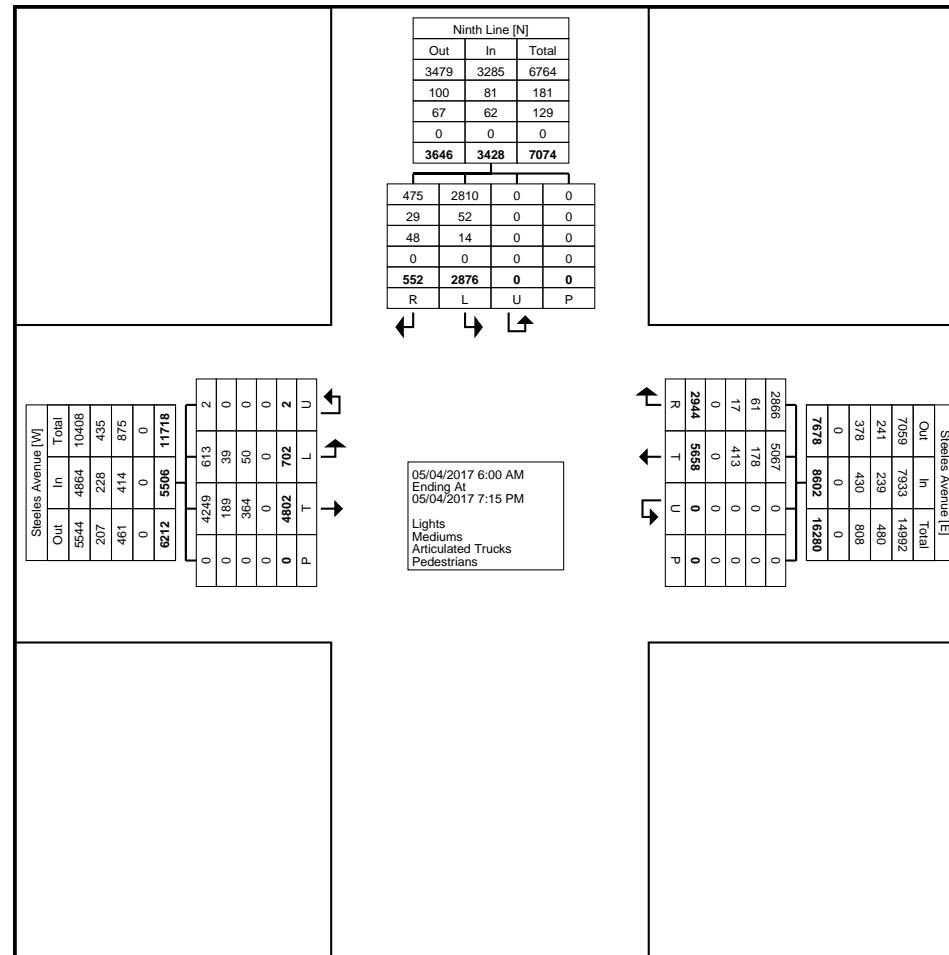




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Count Name: Steeles Avenue & Ninth Line  
(North) - Weekday  
Site Code:  
Start Date: 05/04/2017  
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Turning Movement Data Plot

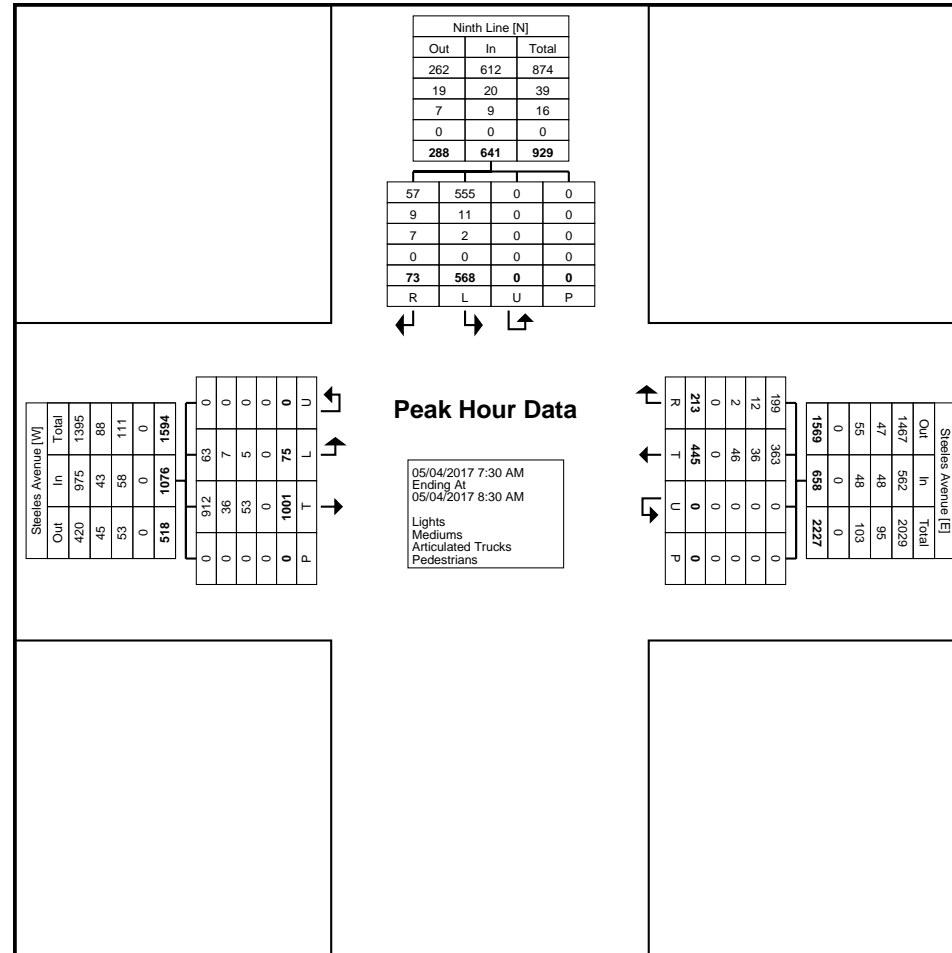




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Turning Movement Peak Hour Data Plot (7:30 AM)



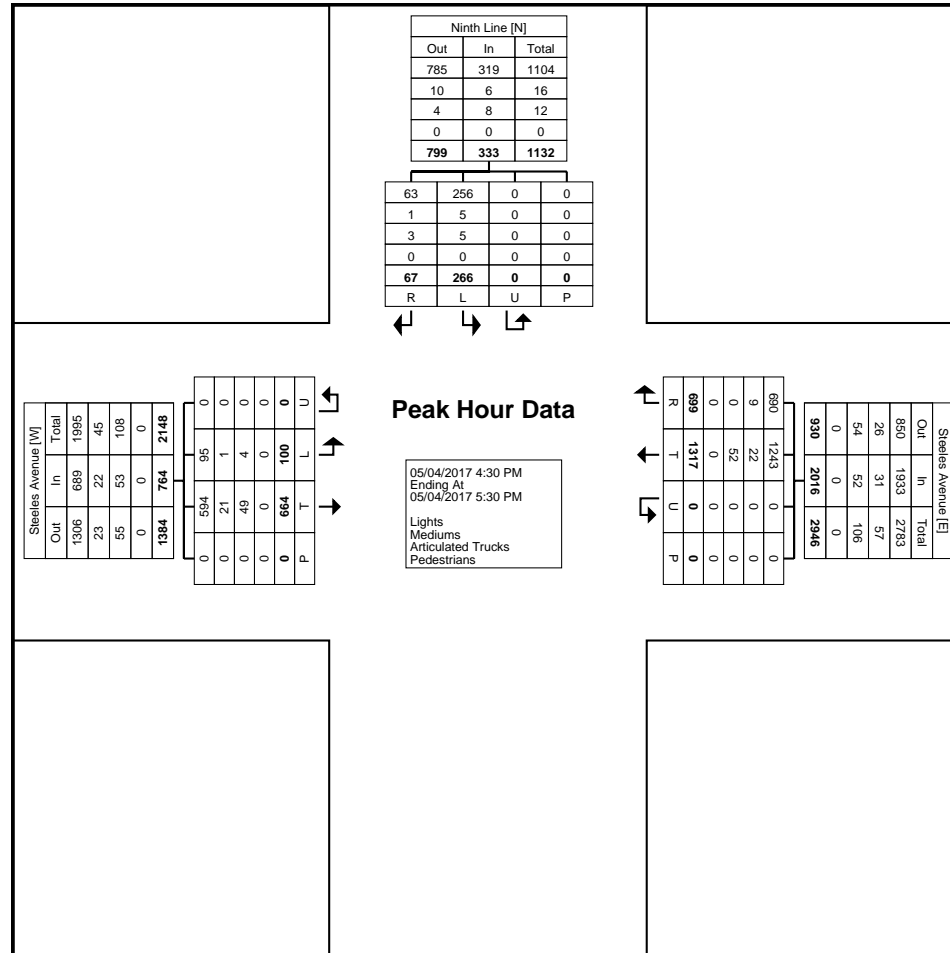




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Turning Movement Peak Hour Data Plot (4:30 PM)



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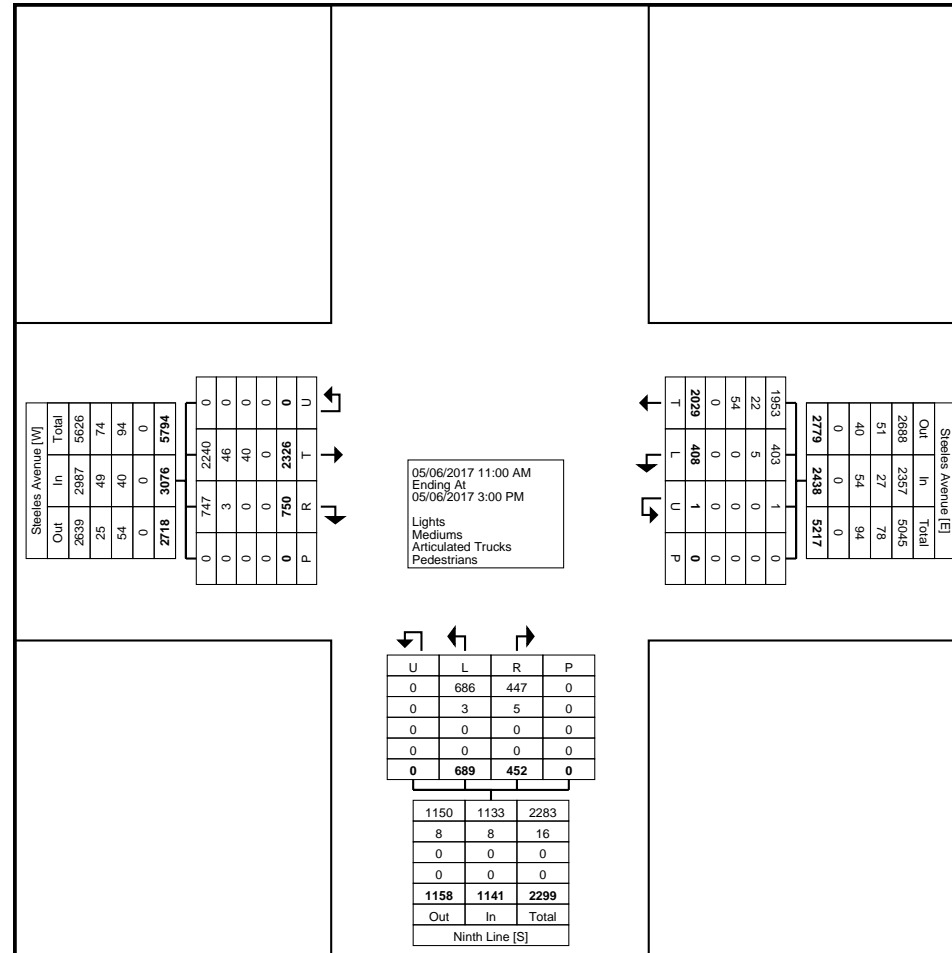




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Count Name: Steeles Avenue & Ninth Line  
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Site Code:  
Start Date: 05/06/2017  
Page No: 2



Turning Movement Data Plot

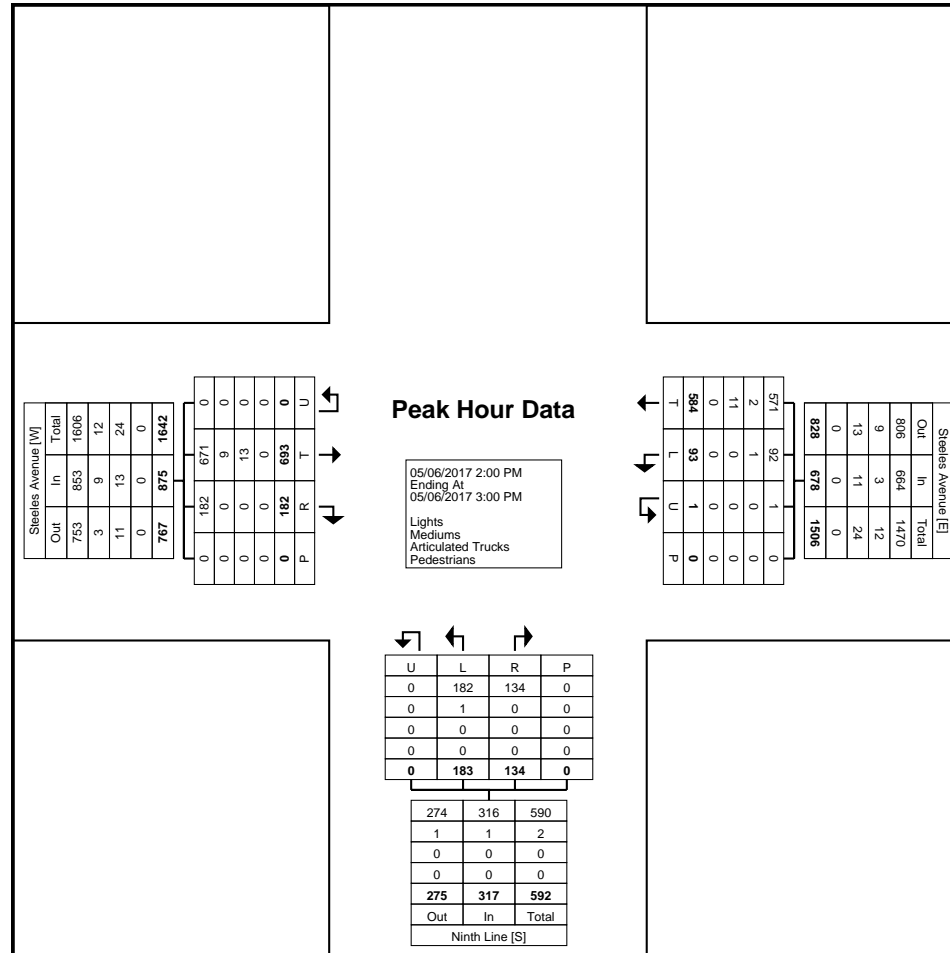




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Start Date: 05/06/2017  
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Turning Movement Peak Hour Data Plot (2:00 PM)



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Site Code:  
Start Date: 05/06/2017  
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Count Name: Steeles Avenue & Ninth Line  
(South) - Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 1

### Turning Movement Data

Start Time	Steeles Avenue Eastbound					Steeles Avenue Westbound					Ninth Line Northbound					Int. Total
	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
6:00 AM	140	20	0	0	160	10	77	0	0	87	9	16	0	0	25	272
6:15 AM	164	42	0	0	206	35	78	0	0	113	10	27	0	0	37	356
6:30 AM	178	55	0	0	233	50	102	0	0	152	20	39	0	0	59	444
6:45 AM	207	73	0	0	280	34	98	0	0	132	23	47	0	0	70	482
Hourly Total	689	190	0	0	879	129	355	0	0	484	62	129	0	0	191	1554
7:00 AM	250	85	0	0	335	58	116	0	0	174	15	30	0	0	45	554
7:15 AM	270	89	0	0	359	68	106	0	0	174	30	48	0	0	78	611
7:30 AM	278	113	0	0	391	72	132	0	0	204	24	75	0	0	99	694
7:45 AM	276	128	0	0	404	73	117	0	0	190	42	79	0	0	121	715
Hourly Total	1074	415	0	0	1489	271	471	0	0	742	111	232	0	0	343	2574
8:00 AM	260	92	0	0	352	82	109	0	0	191	50	55	0	0	105	648
8:15 AM	314	98	0	0	412	84	138	0	0	222	41	74	0	0	115	749
8:30 AM	245	73	0	0	318	75	131	0	0	206	37	70	0	0	107	631
8:45 AM	201	73	0	0	274	57	109	0	0	166	29	61	0	0	90	530
Hourly Total	1020	336	0	0	1356	298	487	0	0	785	157	260	0	0	417	2558
9:00 AM	163	60	0	0	223	39	93	1	0	133	35	37	0	0	72	428
9:15 AM	154	55	0	0	209	39	79	0	0	118	27	29	0	0	56	383
9:30 AM	133	49	0	0	182	25	117	0	0	142	21	24	0	0	45	369
9:45 AM	108	32	0	0	140	24	94	0	0	118	32	29	0	0	61	319
Hourly Total	558	196	0	0	754	127	383	1	0	511	115	119	0	0	234	1499
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	146	39	0	0	185	46	176	0	0	222	54	35	0	0	89	496
3:15 PM	158	38	0	0	196	50	216	0	0	266	58	54	0	0	112	574
3:30 PM	144	55	0	0	199	60	259	0	0	319	77	49	0	0	126	644
3:45 PM	173	43	0	0	216	57	342	0	0	399	89	57	0	0	146	761
Hourly Total	621	175	0	0	796	213	993	0	0	1206	278	195	0	0	473	2475
4:00 PM	166	36	0	0	202	58	350	0	0	408	102	61	0	0	163	773
4:15 PM	153	61	0	0	214	77	370	0	0	447	100	78	0	0	178	839
4:30 PM	168	55	0	0	223	66	407	0	0	473	116	77	0	0	193	889
4:45 PM	148	56	0	0	204	78	381	0	0	459	121	80	0	0	201	864
Hourly Total	635	208	0	0	843	279	1508	0	0	1787	439	296	0	0	735	3365
5:00 PM	170	59	0	0	229	64	367	0	0	431	127	83	0	0	210	870
5:15 PM	193	62	0	0	255	80	395	0	0	475	123	78	0	0	201	931
5:30 PM	176	59	0	0	235	44	367	0	0	411	126	73	0	0	199	845
5:45 PM	125	39	0	0	164	56	343	0	0	399	123	93	0	0	216	779
Hourly Total	664	219	0	0	883	244	1472	0	0	1716	499	327	0	0	826	3425

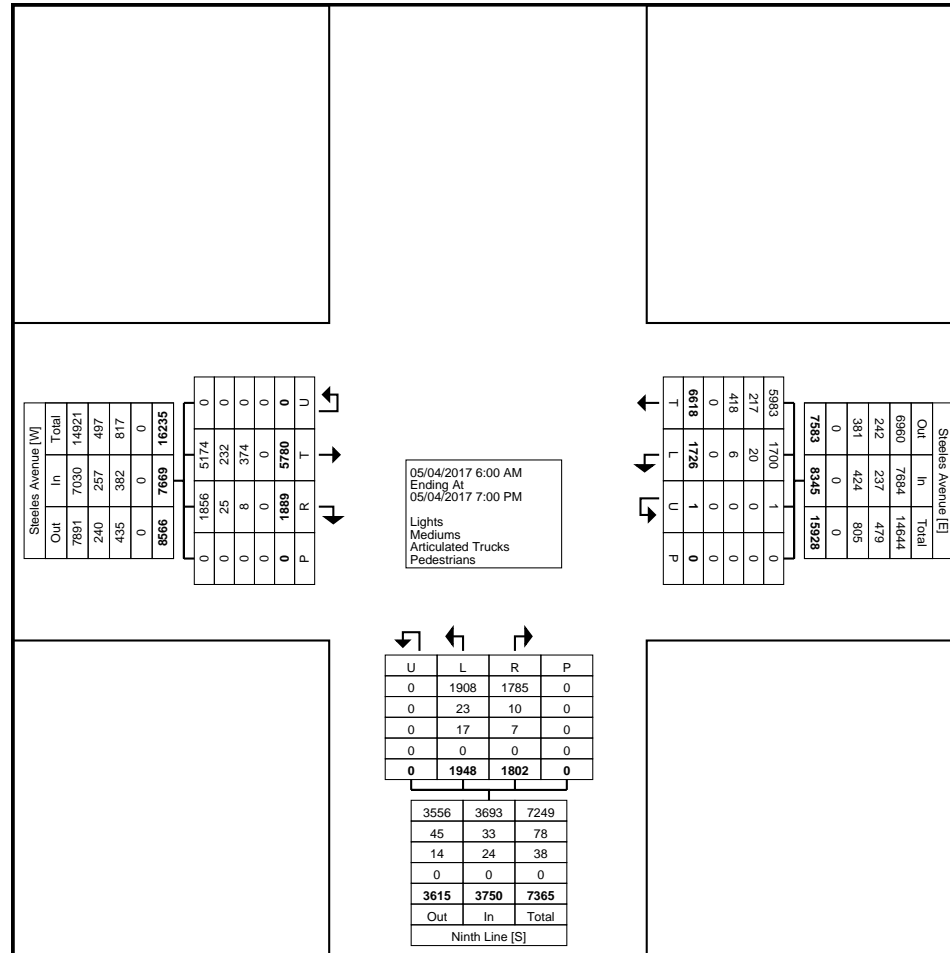




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Count Name: Steeles Avenue & Ninth Line  
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Turning Movement Data Plot

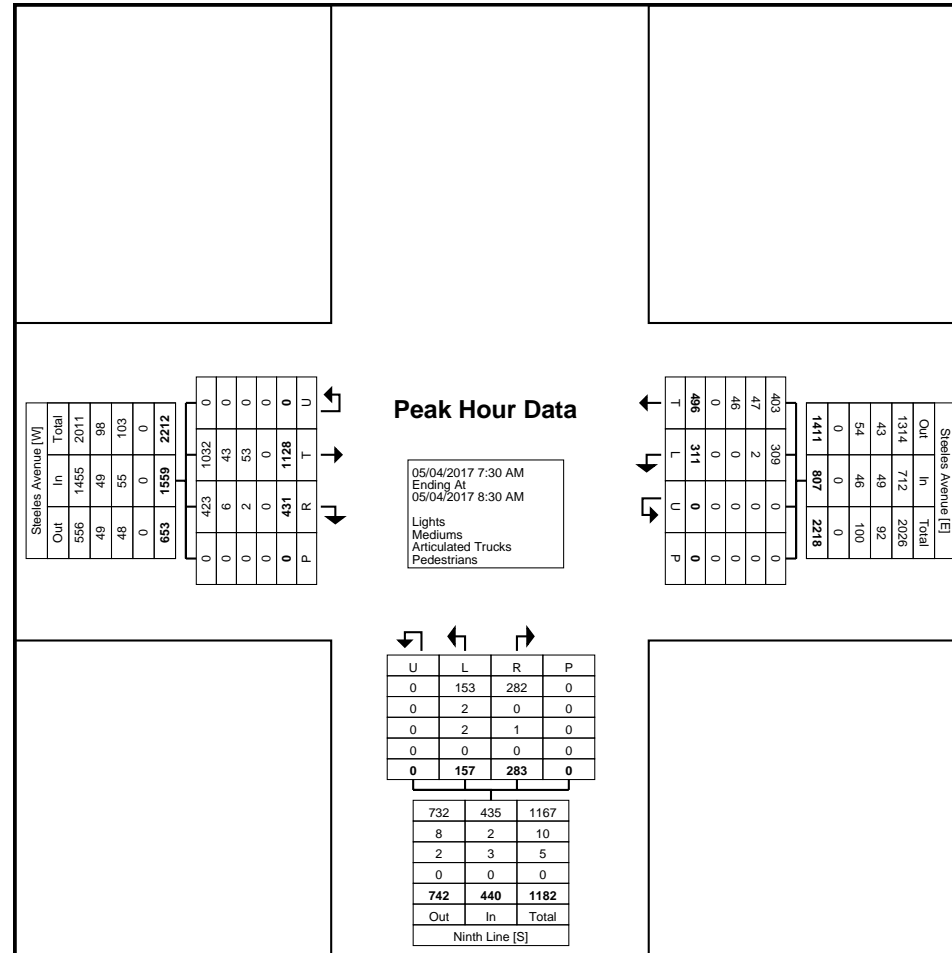




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Turning Movement Peak Hour Data Plot (7:30 AM)

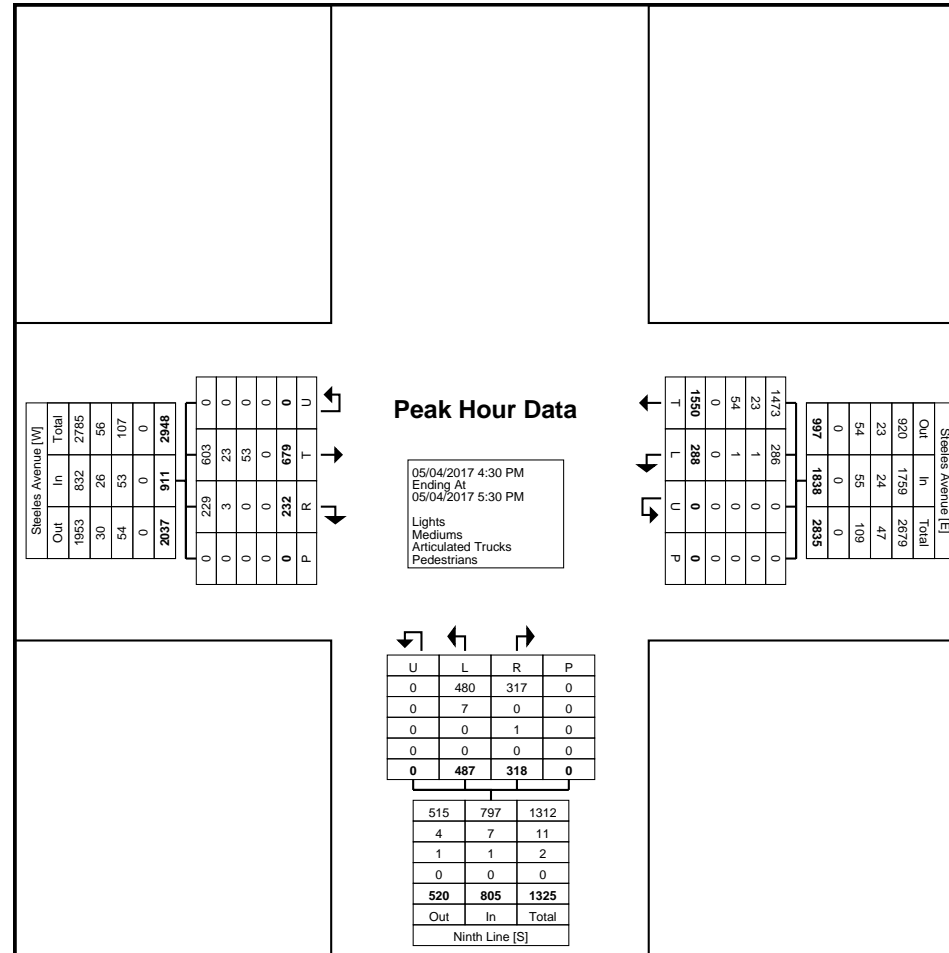




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Turning Movement Peak Hour Data Plot (4:30 PM)



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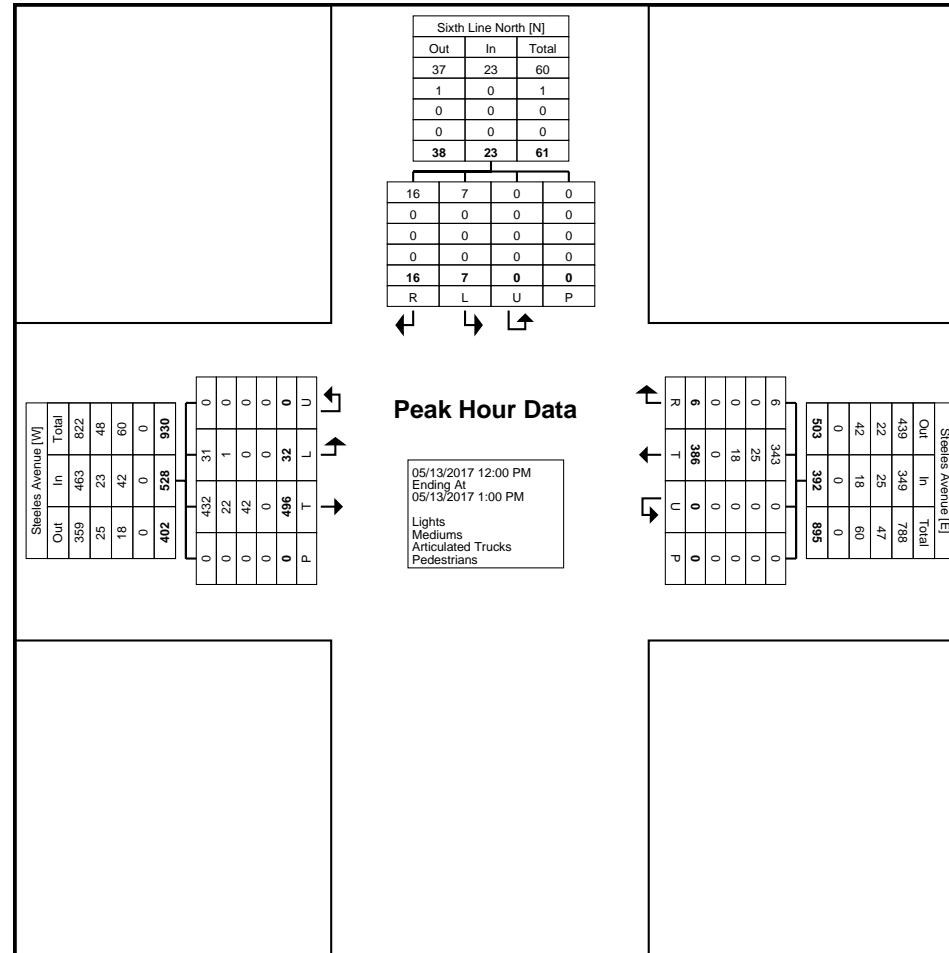




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Count Name: Steeles Avenue & Sixth Line  
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Site Code:  
Start Date: 05/13/2017  
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Turning Movement Peak Hour Data Plot (12:00 PM)



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Count Name: Steeles Avenue & Sixth Line  
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Start Date: 05/13/2017  
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Count Name: Steeles Avenue & Sixth Line  
(North) - Weekday  
Site Code:  
Start Date: 05/16/2017  
Page No: 1

### Turning Movement Data

Start Time	Steeles Avenue Eastbound					Steeles Avenue Westbound					Sixth Line North Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
6:00 AM	3	77	0	0	80	56	1	0	0	57	3	1	0	0	4	141
6:15 AM	3	77	0	0	80	69	0	0	0	69	2	1	0	0	3	152
6:30 AM	3	77	0	0	80	89	1	0	0	90	0	6	0	0	6	176
6:45 AM	3	115	0	0	118	73	3	0	0	76	2	0	0	0	2	196
Hourly Total	12	346	0	0	358	287	5	0	0	292	7	8	0	0	15	665
7:00 AM	7	156	0	0	163	78	0	1	0	79	4	6	0	0	10	252
7:15 AM	8	197	0	0	205	82	0	0	0	82	4	6	0	0	10	297
7:30 AM	3	182	0	0	185	134	0	0	0	134	5	4	0	0	9	328
7:45 AM	10	212	0	0	222	120	1	0	0	121	0	11	0	0	11	354
Hourly Total	28	747	0	0	775	414	1	1	0	416	13	27	0	0	40	1231
8:00 AM	6	220	0	0	226	99	1	0	0	100	2	9	0	0	11	337
8:15 AM	15	187	0	0	202	101	0	0	0	101	1	9	0	0	10	313
8:30 AM	9	189	0	0	198	94	6	0	0	100	2	8	0	0	10	308
8:45 AM	5	137	0	0	142	108	2	0	0	110	4	8	0	0	12	264
Hourly Total	35	733	0	0	768	402	9	0	0	411	9	34	0	0	43	1222
9:00 AM	6	124	0	0	130	91	2	0	0	93	2	3	0	0	5	228
9:15 AM	4	112	0	0	116	106	1	0	0	107	1	5	0	0	6	229
9:30 AM	0	95	0	0	95	90	4	0	0	94	2	2	0	0	4	193
9:45 AM	4	120	0	0	124	96	2	0	0	98	1	4	0	0	5	227
Hourly Total	14	451	0	0	465	383	9	0	0	392	6	14	0	0	20	877
10:00 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1
3:00 PM	8	109	0	0	117	109	0	0	0	109	2	6	0	0	8	234
3:15 PM	6	115	0	0	121	126	8	0	0	134	4	7	0	0	11	266
3:30 PM	3	117	0	0	120	158	6	0	0	164	2	5	0	0	7	291
3:45 PM	5	120	0	0	125	161	5	0	0	166	0	4	0	0	4	295
Hourly Total	22	461	0	0	483	554	19	0	0	573	8	22	0	0	30	1086
4:00 PM	12	143	0	0	155	214	4	0	0	218	3	9	0	0	12	385
4:15 PM	8	123	0	0	131	234	3	0	0	237	1	2	0	0	3	371
4:30 PM	12	176	0	0	188	226	6	0	0	232	2	4	0	1	6	426
4:45 PM	18	187	0	0	205	212	5	0	0	217	1	12	0	0	13	435
Hourly Total	50	629	0	0	679	886	18	0	0	904	7	27	0	1	34	1617
5:00 PM	10	193	0	0	203	259	3	0	0	262	2	9	0	0	11	476
5:15 PM	10	194	0	0	204	238	6	0	0	244	2	4	0	0	6	454
5:30 PM	6	179	0	0	185	263	4	0	0	267	0	6	0	0	6	458
5:45 PM	8	157	0	0	165	225	2	0	0	227	0	6	0	0	6	398
Hourly Total	34	723	0	0	757	985	15	0	0	1000	4	25	0	0	29	1786

6:00 PM	4	138	0	0	142	220	0	0	0	220	1	7	0	0	8	370
6:15 PM	3	138	0	0	141	212	2	0	0	214	1	7	0	0	8	363
6:30 PM	6	98	0	0	104	167	2	0	0	169	2	9	0	0	11	284
6:45 PM	6	73	0	0	79	162	1	0	0	163	3	3	0	0	6	248
Hourly Total	19	447	0	0	466	761	5	0	0	766	7	26	0	0	33	1265
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	214	4538	0	0	4752	4672	81	1	0	4754	61	183	0	1	244	9750
Approach %	4.5	95.5	0.0	-	-	98.3	1.7	0.0	-	-	25.0	75.0	0.0	-	-	-
Total %	2.2	46.5	0.0	-	48.7	47.9	0.8	0.0	-	48.8	0.6	1.9	0.0	-	2.5	-
Lights	207	3331	0	-	3538	3675	63	1	-	3739	48	176	0	-	224	7501
% Lights	96.7	73.4	-	-	74.5	78.7	77.8	100.0	-	78.6	78.7	96.2	-	-	91.8	76.9
Mediums	6	539	0	-	545	453	18	0	-	471	11	7	0	-	18	1034
% Mediums	2.8	11.9	-	-	11.5	9.7	22.2	0.0	-	9.9	18.0	3.8	-	-	7.4	10.6
Articulated Trucks	1	668	0	-	669	544	0	0	-	544	2	0	0	-	2	1215
% Articulated Trucks	0.5	14.7	-	-	14.1	11.6	0.0	0.0	-	11.4	3.3	0.0	-	-	0.8	12.5
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-





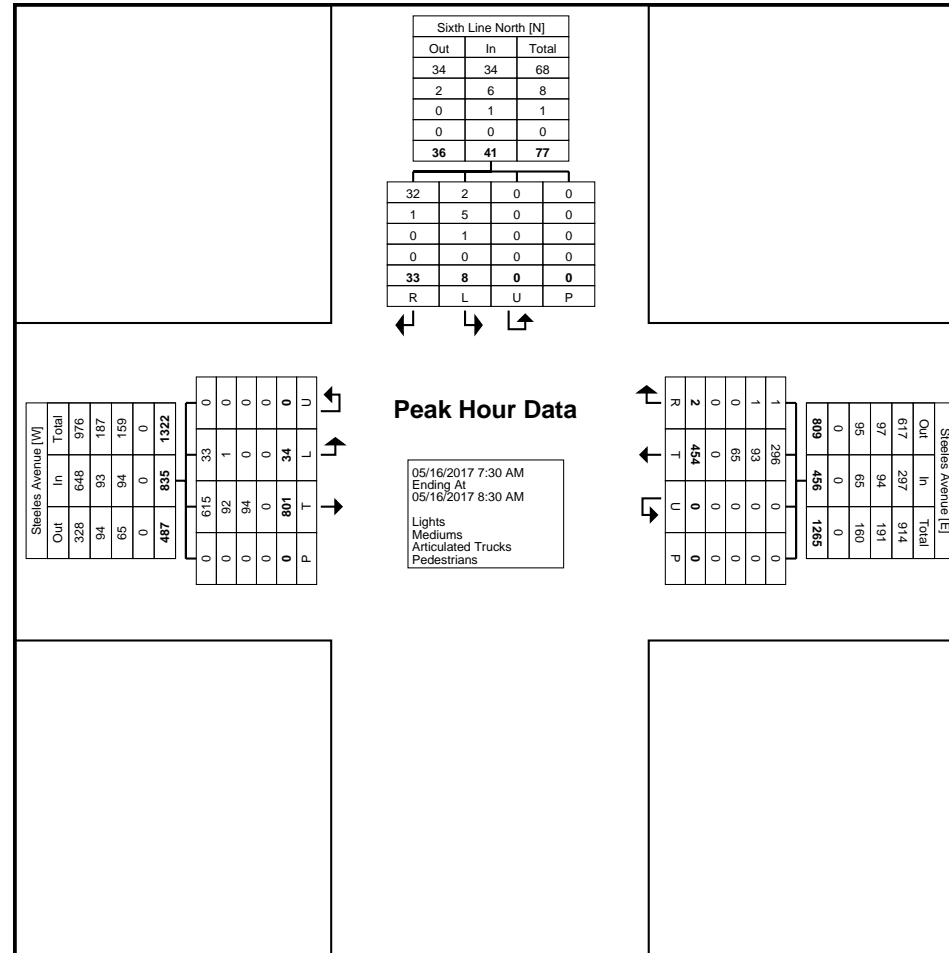




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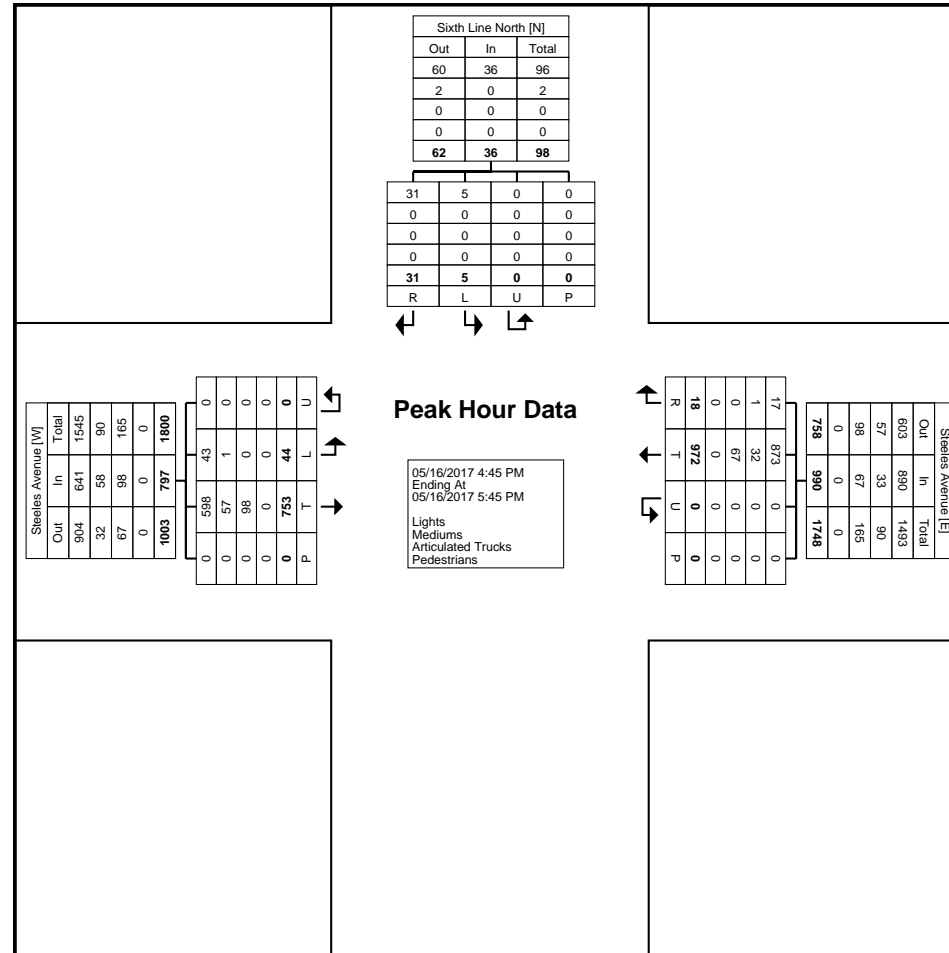




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Turning Movement Peak Hour Data Plot (4:45 PM)



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Count Name: Steeles Avenue & Trafalgar Road -  
Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 1

### Turning Movement Data

Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Trafalgar Road Northbound						Trafalgar Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
11:00 AM	3	45	12	0	0	60	70	52	6	0	0	128	6	44	171	1	0	222	43	105	0	0	0	148	558
11:15 AM	6	46	11	0	0	63	81	45	9	0	0	135	23	85	177	0	0	285	22	70	1	0	0	93	576
11:30 AM	3	48	18	0	0	69	109	51	10	0	0	170	11	77	197	0	0	285	11	133	6	0	0	150	674
11:45 AM	7	58	13	0	0	78	106	60	7	0	0	173	14	72	207	0	0	293	16	109	5	0	0	130	674
Hourly Total	19	197	54	0	0	270	366	208	32	0	0	606	54	278	752	1	0	1085	92	417	12	0	0	521	2482
12:00 PM	7	51	12	0	1	70	108	47	8	0	0	163	14	75	176	0	1	265	18	93	3	0	1	114	612
12:15 PM	11	53	21	0	0	85	124	75	18	0	0	217	13	79	189	2	0	283	19	88	0	0	0	107	692
12:30 PM	6	44	16	0	0	66	112	55	14	0	0	181	22	87	221	0	0	330	20	113	0	0	0	133	710
12:45 PM	3	66	11	0	0	80	129	58	10	0	0	197	32	105	244	0	3	381	17	73	0	0	0	90	748
Hourly Total	27	214	60	0	1	301	473	235	50	0	0	758	81	346	830	2	4	1259	74	367	3	0	1	444	2762
1:00 PM	5	45	12	0	0	62	139	71	10	0	0	220	13	104	231	0	3	348	16	79	3	0	0	98	728
1:15 PM	4	69	13	0	0	86	138	70	11	0	0	219	19	97	204	0	4	320	17	94	7	0	0	118	743
1:30 PM	8	57	15	0	0	80	132	51	12	0	0	195	16	110	206	0	2	332	13	80	4	0	0	97	704
1:45 PM	11	51	19	0	0	81	174	84	6	0	0	264	21	104	196	0	5	321	20	80	4	0	0	104	770
Hourly Total	28	222	59	0	0	309	583	276	39	0	0	898	69	415	837	0	14	1321	66	333	18	0	0	417	2945
2:00 PM	7	63	17	0	0	87	157	86	16	0	0	259	8	73	201	1	4	283	19	110	0	0	0	129	758
2:15 PM	10	62	22	0	0	94	166	65	14	0	0	245	16	78	212	0	5	306	20	101	4	0	0	125	770
2:30 PM	3	61	15	0	0	79	178	85	16	0	0	279	16	84	224	0	3	324	17	94	1	0	0	112	794
2:45 PM	11	79	18	0	0	108	144	69	13	0	1	226	13	82	201	0	3	296	13	91	1	0	0	105	735
Hourly Total	31	265	72	0	0	368	645	305	59	0	1	1009	53	317	838	1	15	1209	69	396	6	0	0	471	3057
3:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1
Grand Total	105	898	245	0	1	1248	2067	1024	180	0	1	3271	257	1356	3258	4	33	4875	301	1513	39	0	1	1853	11247
Approach %	8.4	72.0	19.6	0.0	-	-	63.2	31.3	5.5	0.0	-	-	5.3	27.8	66.8	0.1	-	-	16.2	81.7	2.1	0.0	-	-	-
Total %	0.9	8.0	2.2	0.0	-	11.1	18.4	9.1	1.6	0.0	-	29.1	2.3	12.1	29.0	0.0	-	43.3	2.7	13.5	0.3	0.0	-	16.5	-
Lights	96	835	174	0	-	1105	2050	971	180	0	-	3201	211	1323	3226	4	-	4764	294	1481	34	0	-	1809	10879
% Lights	91.4	93.0	71.0	-	-	88.5	99.2	94.8	100.0	-	-	97.9	82.1	97.6	99.0	100.0	-	97.7	97.7	97.9	87.2	-	-	97.6	96.7
Mediums	6	28	20	0	-	54	8	22	0	0	-	30	16	22	14	0	-	52	5	25	2	0	-	32	168
% Mediums	5.7	3.1	8.2	-	-	4.3	0.4	2.1	0.0	-	-	0.9	6.2	1.6	0.4	0.0	-	1.1	1.7	1.7	5.1	-	-	1.7	1.5
Articulated Trucks	3	35	51	0	-	89	9	31	0	0	-	40	30	11	18	0	-	59	2	7	3	0	-	12	200
% Articulated Trucks	2.9	3.9	20.8	-	-	7.1	0.4	3.0	0.0	-	-	1.2	11.7	0.8	0.6	0.0	-	1.2	0.7	0.5	7.7	-	-	0.6	1.8
Pedestrians	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	33	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-











Paradigm Transportation Solutions Limited  
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Paradigm Transportation Solutions Limited  
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Saturday  
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### Turning Movement Data

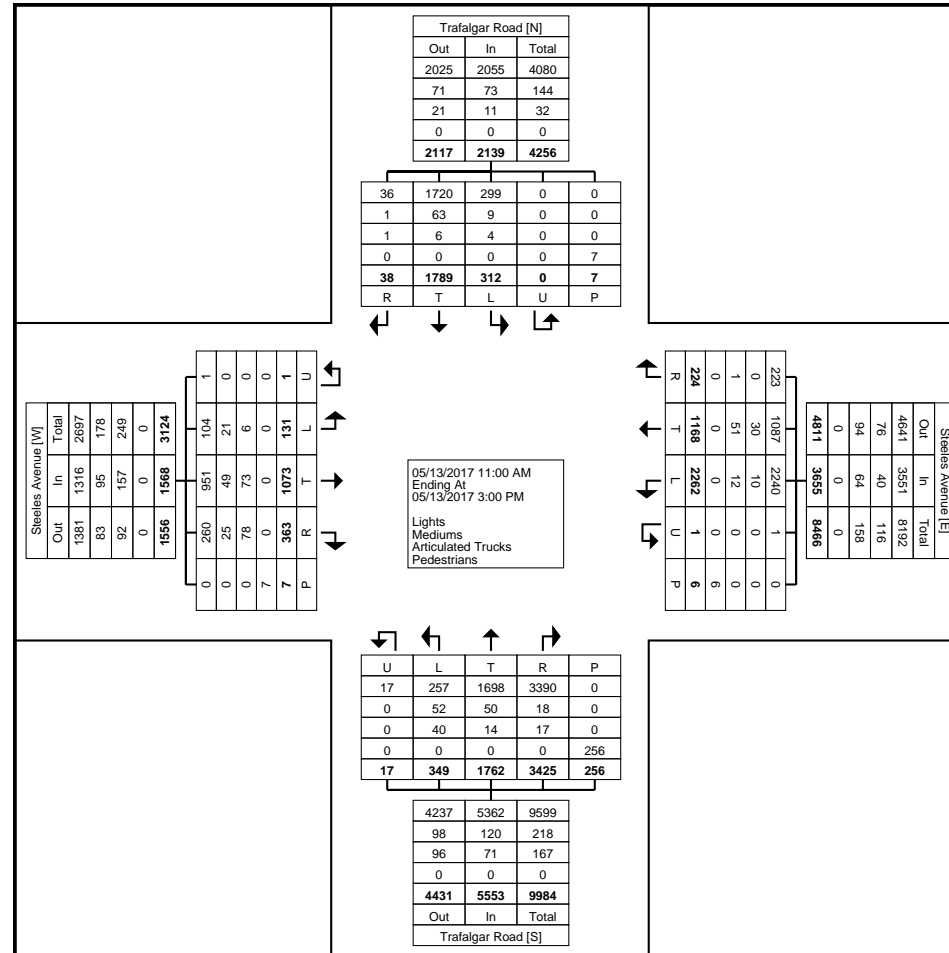
Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Trafalgar Road Northbound						Trafalgar Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
11:00 AM	7	45	18	0	0	70	110	47	12	0	0	169	14	113	237	0	3	364	17	107	0	0	0	124	727
11:15 AM	6	47	22	0	0	75	115	73	15	0	1	203	25	74	182	0	0	281	17	120	4	0	1	141	700
11:30 AM	9	56	22	0	0	87	130	54	11	0	1	195	19	109	196	0	0	324	17	104	1	0	1	122	728
11:45 AM	6	73	18	0	0	97	130	70	18	0	1	218	20	90	212	0	3	322	17	112	2	0	0	131	768
Hourly Total	28	221	80	0	0	329	485	244	56	0	3	785	78	386	827	0	6	1291	68	443	7	0	2	518	2923
12:00 PM	9	79	28	0	0	116	154	75	18	0	0	247	20	96	215	0	2	331	15	97	3	0	0	115	809
12:15 PM	11	90	24	0	0	125	128	81	12	0	0	221	18	93	193	4	2	308	18	121	3	0	0	142	796
12:30 PM	7	86	24	0	1	117	142	62	15	0	0	219	19	116	206	2	11	343	22	117	2	0	0	141	820
12:45 PM	8	81	22	0	0	111	128	83	20	1	0	232	24	100	231	1	9	356	23	120	5	0	0	148	847
Hourly Total	35	336	98	0	1	469	552	301	65	1	0	919	81	405	845	7	24	1338	78	455	13	0	0	546	3272
1:00 PM	9	72	30	0	0	111	141	68	9	0	1	218	32	93	209	1	9	335	24	120	0	0	1	144	808
1:15 PM	9	71	26	0	2	106	144	63	17	0	0	224	22	107	208	0	13	337	25	113	4	0	0	142	809
1:30 PM	9	60	20	1	0	90	186	69	21	0	0	276	20	148	215	1	22	384	19	107	3	0	0	129	879
1:45 PM	11	62	18	0	4	91	137	101	8	0	0	246	22	122	233	1	16	378	16	109	3	0	0	128	843
Hourly Total	38	265	94	1	6	398	608	301	55	0	1	964	96	470	865	3	60	1434	84	449	10	0	1	543	3339
2:00 PM	7	59	24	0	0	90	161	74	8	0	0	243	20	123	230	1	20	374	21	112	2	0	0	135	842
2:15 PM	8	61	30	0	0	99	144	90	17	0	0	251	25	104	220	1	29	350	13	112	3	0	2	128	828
2:30 PM	9	66	17	0	0	92	162	74	9	0	0	245	18	139	222	1	52	380	17	92	2	0	0	111	828
2:45 PM	6	65	20	0	0	91	150	84	14	0	2	248	31	135	216	4	65	386	31	126	1	0	2	158	883
Hourly Total	30	251	91	0	0	372	617	322	48	0	2	987	94	501	888	7	166	1490	82	442	8	0	4	532	3381
Grand Total	131	1073	363	1	7	1568	2262	1168	224	1	6	3655	349	1762	3425	17	256	5553	312	1789	38	0	7	2139	12915
Approach %	8.4	68.4	23.2	0.1	-	-	61.9	32.0	6.1	0.0	-	-	6.3	31.7	61.7	0.3	-	-	14.6	83.6	1.8	0.0	-	-	-
Total %	1.0	8.3	2.8	0.0	-	12.1	17.5	9.0	1.7	0.0	-	28.3	2.7	13.6	26.5	0.1	-	43.0	2.4	13.9	0.3	0.0	-	16.6	-
Lights	104	951	260	1	-	1316	2240	1087	223	1	-	3551	257	1698	3390	17	-	5362	299	1720	36	0	-	2055	12284
% Lights	79.4	88.6	71.6	100.0	-	83.9	99.0	93.1	99.6	100.0	-	97.2	73.6	96.4	99.0	100.0	-	96.6	95.8	96.1	94.7	-	-	96.1	95.1
Mediums	21	49	25	0	-	95	10	30	0	0	-	40	52	50	18	0	-	120	9	63	1	0	-	73	328
% Mediums	16.0	4.6	6.9	0.0	-	6.1	0.4	2.6	0.0	0.0	-	1.1	14.9	2.8	0.5	0.0	-	2.2	2.9	3.5	2.6	-	-	3.4	2.5
Articulated Trucks	6	73	78	0	-	157	12	51	1	0	-	64	40	14	17	0	-	71	4	6	1	0	-	11	303
% Articulated Trucks	4.6	6.8	21.5	0.0	-	10.0	0.5	4.4	0.4	0.0	-	1.8	11.5	0.8	0.5	0.0	-	1.3	1.3	0.3	2.6	-	-	0.5	2.3
Pedestrians	-	-	-	-	7	-	-	-	-	-	6	-	-	-	-	-	256	-	-	-	-	-	7	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: Steeles Avenue & Trafalgar Road - Saturday  
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Turning Movement Data Plot



Paradigm Transportation Solutions Limited  
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519-896-3163 cbowness@ptsl.com

Count Name: Steeles Avenue & Trafalgar Road -  
Saturday  
Site Code:  
Start Date: 05/13/2017  
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### Turning Movement Peak Hour Data (1:30 PM)

Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Trafalgar Road Northbound						Trafalgar Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
1:30 PM	9	60	20	1	0	90	186	69	21	0	0	276	20	148	215	1	22	384	19	107	3	0	0	129	879
1:45 PM	11	62	18	0	4	91	137	101	8	0	0	246	22	122	233	1	16	378	16	109	3	0	0	128	843
2:00 PM	7	59	24	0	0	90	161	74	8	0	0	243	20	123	230	1	20	374	21	112	2	0	0	135	842
2:15 PM	8	61	30	0	0	99	144	90	17	0	0	251	25	104	220	1	29	350	13	112	3	0	2	128	828
Total	35	242	92	1	4	370	628	334	54	0	0	1016	87	497	898	4	87	1486	69	440	11	0	2	520	3392
Approach %	9.5	65.4	24.9	0.3	-	-	61.8	32.9	5.3	0.0	-	-	5.9	33.4	60.4	0.3	-	-	13.3	84.6	2.1	0.0	-	-	-
Total %	1.0	7.1	2.7	0.0	-	10.9	18.5	9.8	1.6	0.0	-	30.0	2.6	14.7	26.5	0.1	-	43.8	2.0	13.0	0.3	0.0	-	15.3	-
PHF	0.795	0.976	0.767	0.250	-	0.934	0.844	0.827	0.643	0.000	-	0.920	0.870	0.840	0.964	1.000	-	0.967	0.821	0.982	0.917	0.000	-	0.963	0.965
Lights	25	215	66	1	-	307	625	317	54	0	-	996	62	482	888	4	-	1436	66	423	10	0	-	499	3238
% Lights	71.4	88.8	71.7	100.0	-	83.0	99.5	94.9	100.0	-	-	98.0	71.3	97.0	98.9	100.0	-	96.6	95.7	96.1	90.9	-	-	96.0	95.5
Mediums	7	7	6	0	-	20	1	6	0	0	-	7	17	12	7	0	-	36	2	14	0	0	-	16	79
% Mediums	20.0	2.9	6.5	0.0	-	5.4	0.2	1.8	0.0	-	-	0.7	19.5	2.4	0.8	0.0	-	2.4	2.9	3.2	0.0	-	-	3.1	2.3
Articulated Trucks	3	20	20	0	-	43	2	11	0	0	-	13	8	3	3	0	-	14	1	3	1	0	-	5	75
% Articulated Trucks	8.6	8.3	21.7	0.0	-	11.6	0.3	3.3	0.0	-	-	1.3	9.2	0.6	0.3	0.0	-	0.9	1.4	0.7	9.1	-	-	1.0	2.2
Pedestrians	-	-	-	-	4	-	-	-	-	-	0	-	-	-	-	-	87	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-





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Saturday  
Site Code:  
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Paradigm Transportation Solutions Limited  
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Waterloo, Ontario, Canada N2J 1N8  
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Count Name: Steeles Avenue & Trafalgar Road -  
Weekday  
Site Code:  
Start Date: 05/04/2017  
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### Turning Movement Data

Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Trafalgar Road Northbound						Trafalgar Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
6:00 AM	6	41	16	0	0	63	70	50	6	0	0	126	12	25	58	0	0	95	19	116	1	0	0	136	420
6:15 AM	2	36	14	0	0	52	75	60	7	0	0	142	24	42	76	1	0	143	18	151	6	0	0	175	512
6:30 AM	5	53	18	0	0	76	92	66	6	0	0	164	23	41	67	0	0	131	21	187	1	0	0	209	580
6:45 AM	5	64	17	0	0	86	90	54	2	0	0	146	20	62	82	0	1	164	34	179	3	0	0	216	612
Hourly Total	18	194	65	0	0	277	327	230	21	0	0	578	79	170	283	1	1	533	92	633	11	0	0	736	2124
7:00 AM	8	84	25	0	1	117	98	59	13	0	0	170	27	44	97	0	0	168	26	165	0	0	0	191	646
7:15 AM	7	103	37	1	0	148	132	59	9	0	0	200	23	59	91	0	0	173	24	208	4	0	1	236	757
7:30 AM	7	127	45	0	0	179	119	75	14	0	0	208	29	64	80	0	0	173	31	256	1	0	0	288	848
7:45 AM	9	129	43	0	1	181	106	73	6	0	1	185	29	62	90	0	1	181	43	265	5	0	2	313	860
Hourly Total	31	443	150	1	2	625	455	266	42	0	1	763	108	229	358	0	1	695	124	894	10	0	3	1028	3111
8:00 AM	5	121	52	0	0	178	100	63	7	1	0	171	39	58	90	0	0	187	40	240	3	0	1	283	819
8:15 AM	6	126	41	0	0	173	124	94	8	0	0	226	21	80	76	0	0	177	31	234	1	0	0	266	842
8:30 AM	10	110	46	0	0	166	106	73	12	0	0	191	32	69	89	0	0	190	22	185	0	0	0	207	754
8:45 AM	11	82	34	0	0	127	69	73	5	0	0	147	25	46	89	0	0	160	16	154	3	0	0	173	607
Hourly Total	32	439	173	0	0	644	399	303	32	1	0	735	117	253	344	0	0	714	109	813	7	0	1	929	3022
9:00 AM	10	50	35	0	0	95	68	62	15	0	0	145	33	58	75	0	0	166	23	141	8	0	0	172	578
9:15 AM	4	49	28	0	0	81	77	59	17	0	0	153	26	59	83	0	0	168	25	135	4	0	1	164	566
9:30 AM	5	47	30	0	0	82	66	51	11	0	0	128	27	62	79	0	0	168	20	113	5	0	0	138	516
9:45 AM	5	49	23	0	0	77	51	54	8	0	0	113	33	61	97	0	0	191	16	96	3	0	1	115	496
Hourly Total	24	195	116	0	0	335	262	226	51	0	0	539	119	240	334	0	0	693	84	485	20	0	2	589	2156
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1
3:00 PM	8	73	23	0	0	104	91	81	13	0	0	185	49	117	112	0	0	278	15	88	8	0	0	111	678
3:15 PM	13	89	27	0	0	129	128	100	26	0	0	254	38	129	130	0	0	297	16	82	0	0	0	98	778
3:30 PM	5	73	24	1	0	103	118	111	22	0	0	251	49	135	129	0	0	313	9	64	1	0	0	74	741
3:45 PM	10	72	14	0	0	96	136	156	21	0	0	313	25	126	128	0	0	279	12	58	5	0	0	75	763
Hourly Total	36	307	88	1	0	432	473	448	82	0	0	1003	161	507	499	0	0	1167	52	292	14	0	0	358	2960
4:00 PM	9	61	35	0	0	105	152	164	39	0	0	355	44	125	126	0	0	295	12	69	2	0	0	83	838
4:15 PM	6	69	22	0	0	97	145	187	44	0	0	376	33	116	115	0	0	264	4	74	5	0	0	83	820
4:30 PM	9	90	32	0	0	131	150	196	36	0	0	382	32	141	115	1	0	289	9	73	2	0	0	84	886
4:45 PM	4	79	27	0	0	110	140	176	36	0	0	352	37	158	139	0	0	334	9	66	5	0	0	80	876
Hourly Total	28	299	116	0	0	443	587	723	155	0	0	1465	146	540	495	1	0	1182	34	282	14	0	0	330	3420
5:00 PM	7	81	31	0	0	119	153	174	43	0	0	370	37	163	157	0	0	357	7	74	3	0	0	84	930
5:15 PM	5	96	18	0	0	119	127	200	35	0	0	362	49	146	128	1	0	324	13	67	4	0	0	84	889
5:30 PM	6	62	9	0	0	77	132	186	41	0	0	359	36	154	119	1	0	310	15	79	3	0	0	97	843
5:45 PM	6	51	19	0	0	76	124	195	29	0	0	348	47	161	135	0	0	343	10	71	3	0	0	84	851



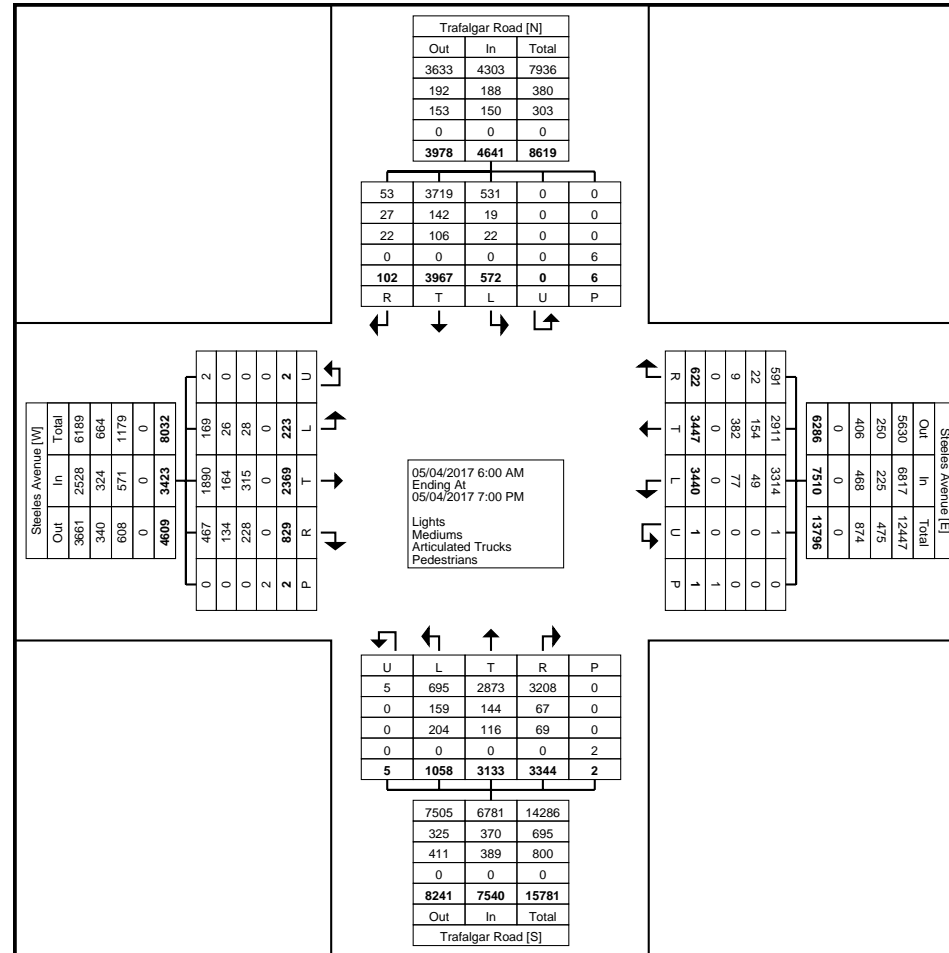
Hourly Total	24	290	77	0	0	391	536	755	148	0	0	1439	169	624	539	2	0	1334	45	291	13	0	0	349	3513
6:00 PM	6	62	10	0	0	78	115	163	28	0	0	306	36	149	129	0	0	314	6	69	4	0	0	79	777
6:15 PM	9	62	14	0	0	85	111	143	19	0	0	273	51	137	131	0	0	319	11	84	3	0	0	98	775
6:30 PM	9	45	8	0	0	62	88	96	21	0	0	205	37	160	125	0	0	322	8	68	4	0	0	80	669
6:45 PM	6	33	12	0	0	51	87	94	23	0	0	204	35	123	107	1	0	266	7	56	2	0	0	65	586
Hourly Total	30	202	44	0	0	276	401	496	91	0	0	988	159	569	492	1	0	1221	32	277	13	0	0	322	2807
Grand Total	223	2369	829	2	2	3423	3440	3447	622	1	1	7510	1058	3133	3344	5	2	7540	572	3967	102	0	6	4641	23114
Approach %	6.5	69.2	24.2	0.1	-	-	45.8	45.9	8.3	0.0	-	-	14.0	41.6	44.4	0.1	-	-	12.3	85.5	2.2	0.0	-	-	-
Total %	1.0	10.2	3.6	0.0	-	14.8	14.9	14.9	2.7	0.0	-	32.5	4.6	13.6	14.5	0.0	-	32.6	2.5	17.2	0.4	0.0	-	20.1	-
Lights	169	1890	467	2	-	2528	3314	2911	591	1	-	6817	695	2873	3208	5	-	6781	531	3719	53	0	-	4303	20429
% Lights	75.8	79.8	56.3	100.0	-	73.9	96.3	84.5	95.0	100.0	-	90.8	65.7	91.7	95.9	100.0	-	89.9	92.8	93.7	52.0	-	-	92.7	88.4
Mediums	26	164	134	0	-	324	49	154	22	0	-	225	159	144	67	0	-	370	19	142	27	0	-	188	1107
% Mediums	11.7	6.9	16.2	0.0	-	9.5	1.4	4.5	3.5	0.0	-	3.0	15.0	4.6	2.0	0.0	-	4.9	3.3	3.6	26.5	-	-	4.1	4.8
Articulated Trucks	28	315	228	0	-	571	77	382	9	0	-	468	204	116	69	0	-	389	22	106	22	0	-	150	1578
% Articulated Trucks	12.6	13.3	27.5	0.0	-	16.7	2.2	11.1	1.4	0.0	-	6.2	19.3	3.7	2.1	0.0	-	5.2	3.8	2.7	21.6	-	-	3.2	6.8
Pedestrians	-	-	-	-	2	-	-	-	-	-	1	-	-	-	-	-	2	-	-	-	-	-	6	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited  
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Count Name: Steeles Avenue & Trafalgar Road -  
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Site Code:  
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Turning Movement Data Plot



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Count Name: Steeles Avenue & Trafalgar Road -  
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### Turning Movement Peak Hour Data (7:30 AM)

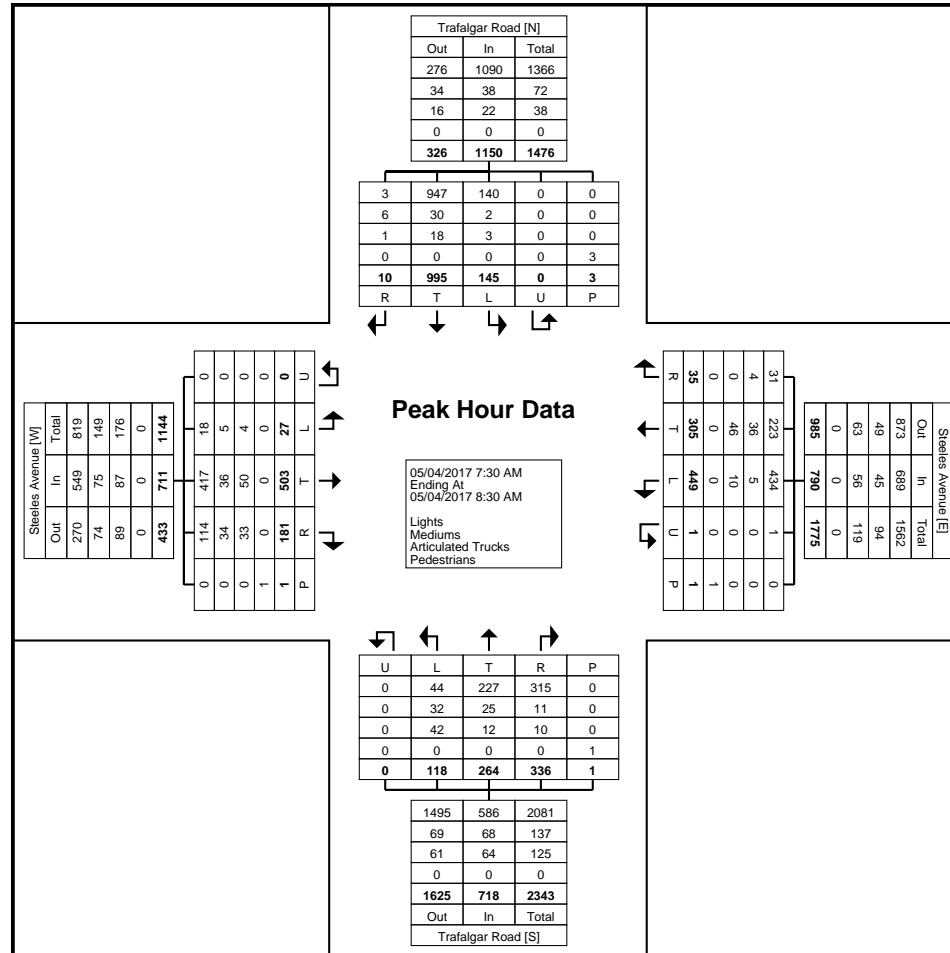
Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Trafalgar Road Northbound						Trafalgar Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	7	127	45	0	0	179	119	75	14	0	0	208	29	64	80	0	0	173	31	256	1	0	0	288	848
7:45 AM	9	129	43	0	1	181	106	73	6	0	1	185	29	62	90	0	1	181	43	265	5	0	2	313	860
8:00 AM	5	121	52	0	0	178	100	63	7	1	0	171	39	58	90	0	0	187	40	240	3	0	1	283	819
8:15 AM	6	126	41	0	0	173	124	94	8	0	0	226	21	80	76	0	0	177	31	234	1	0	0	266	842
Total	27	503	181	0	1	711	449	305	35	1	1	790	118	264	336	0	1	718	145	995	10	0	3	1150	3369
Approach %	3.8	70.7	25.5	0.0	-	-	56.8	38.6	4.4	0.1	-	-	16.4	36.8	46.8	0.0	-	-	12.6	86.5	0.9	0.0	-	-	-
Total %	0.8	14.9	5.4	0.0	-	21.1	13.3	9.1	1.0	0.0	-	23.4	3.5	7.8	10.0	0.0	-	21.3	4.3	29.5	0.3	0.0	-	34.1	-
PHF	0.750	0.975	0.870	0.000	-	0.982	0.905	0.811	0.625	0.250	-	0.874	0.756	0.825	0.933	0.000	-	0.960	0.843	0.939	0.500	0.000	-	0.919	0.979
Lights	18	417	114	0	-	549	434	223	31	1	-	689	44	227	315	0	-	586	140	947	3	0	-	1090	2914
% Lights	66.7	82.9	63.0	-	-	77.2	96.7	73.1	88.6	100.0	-	87.2	37.3	86.0	93.8	-	-	81.6	96.6	95.2	30.0	-	-	94.8	86.5
Mediums	5	36	34	0	-	75	5	36	4	0	-	45	32	25	11	0	-	68	2	30	6	0	-	38	226
% Mediums	18.5	7.2	18.8	-	-	10.5	1.1	11.8	11.4	0.0	-	5.7	27.1	9.5	3.3	-	-	9.5	1.4	3.0	60.0	-	-	3.3	6.7
Articulated Trucks	4	50	33	0	-	87	10	46	0	0	-	56	42	12	10	0	-	64	3	18	1	0	-	22	229
% Articulated Trucks	14.8	9.9	18.2	-	-	12.2	2.2	15.1	0.0	0.0	-	7.1	35.6	4.5	3.0	-	-	8.9	2.1	1.8	10.0	-	-	1.9	6.8
Pedestrians	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	3	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Turning Movement Peak Hour Data Plot (7:30 AM)







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Count Name: Steeles Avenue & Trafalgar Road -  
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Site Code:  
Start Date: 05/16/2017  
Page No: 1

### Turning Movement Data

Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Trafalgar Road Northbound						Trafalgar Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
6:00 AM	3	56	25	0	0	84	66	38	7	0	0	111	24	30	59	1	0	114	18	127	3	0	1	148	457
6:15 AM	1	47	19	0	0	67	85	47	11	0	0	143	23	49	95	0	0	167	31	156	2	0	1	189	566
6:30 AM	6	55	15	0	0	76	97	52	12	0	0	161	32	47	97	2	1	178	32	183	1	0	0	216	631
6:45 AM	10	72	27	0	0	109	93	46	16	0	0	155	23	55	116	0	0	194	27	168	3	0	0	198	656
Hourly Total	20	230	86	0	0	336	341	183	46	0	0	570	102	181	367	3	1	653	108	634	9	0	2	751	2310
7:00 AM	5	99	26	0	1	130	102	44	9	0	0	155	24	60	94	0	1	178	27	213	1	0	0	241	704
7:15 AM	10	115	37	0	0	162	116	62	11	0	0	189	32	71	84	0	1	187	29	198	0	0	0	227	765
7:30 AM	6	124	42	0	0	172	112	80	7	0	0	199	41	87	77	1	0	206	37	272	3	0	0	312	889
7:45 AM	8	119	59	2	1	188	133	92	5	0	0	230	28	61	84	0	2	173	43	228	4	0	1	275	866
Hourly Total	29	457	164	2	2	652	463	278	32	0	0	773	125	279	339	1	4	744	136	911	8	0	1	1055	3224
8:00 AM	11	127	51	0	0	189	135	74	16	0	0	225	36	60	101	0	0	197	40	270	1	0	0	311	922
8:15 AM	6	129	46	0	0	181	120	72	12	0	1	204	33	61	74	0	0	168	32	271	0	0	1	303	856
8:30 AM	9	116	56	0	0	181	101	72	12	0	0	185	33	65	78	0	0	176	34	184	1	0	0	219	761
8:45 AM	12	120	48	0	0	180	84	79	11	0	0	174	21	63	100	0	0	184	30	172	4	0	0	206	744
Hourly Total	38	492	201	0	0	731	440	297	51	0	1	788	123	249	353	0	0	725	136	897	6	0	1	1039	3283
9:00 AM	10	71	25	0	1	106	80	59	9	0	0	148	38	58	86	0	0	182	17	136	5	0	2	158	594
9:15 AM	11	61	33	0	0	105	74	56	8	0	3	138	22	61	85	0	1	168	21	129	5	0	2	155	566
9:30 AM	11	76	29	0	2	116	63	48	13	0	0	124	30	68	86	0	3	184	23	113	7	0	0	143	567
9:45 AM	8	66	34	0	0	108	57	61	10	1	0	129	30	60	92	1	0	183	11	94	4	0	0	109	529
Hourly Total	40	274	121	0	3	435	274	224	40	1	3	539	120	247	349	1	4	717	72	472	21	0	4	565	2256
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	5	61	36	1	0	103	109	81	14	1	0	205	25	103	121	0	0	249	10	79	5	0	0	94	651
3:15 PM	6	55	35	0	0	96	110	89	24	0	0	223	38	150	141	0	0	329	20	80	2	0	0	102	750
3:30 PM	15	64	37	0	0	116	123	103	19	0	0	245	52	148	131	1	0	332	14	55	1	0	1	70	763
3:45 PM	10	77	33	0	0	120	125	122	22	0	0	269	40	121	150	0	0	311	11	80	7	0	0	98	798
Hourly Total	36	257	141	1	0	435	467	395	79	1	0	942	155	522	543	1	0	1221	55	294	15	0	1	364	2962
4:00 PM	5	77	49	0	0	131	132	156	30	1	0	319	57	154	150	0	0	361	12	101	2	0	0	115	926
4:15 PM	15	62	31	0	0	108	130	180	31	0	0	341	36	145	137	0	0	318	16	102	5	0	0	123	890
4:30 PM	8	94	36	0	0	138	107	171	28	0	0	306	49	180	118	0	0	347	9	87	2	0	0	98	889
4:45 PM	11	110	31	0	0	152	148	217	35	0	0	400	38	156	156	0	1	350	18	66	5	0	0	89	991
Hourly Total	39	343	147	0	0	529	517	724	124	1	0	1366	180	635	561	0	1	1376	55	356	14	0	0	425	3696
5:00 PM	16	133	65	0	1	214	153	145	37	0	0	335	52	157	167	0	1	376	19	99	5	0	0	123	1048
5:15 PM	19	131	46	0	0	196	160	180	30	0	0	370	42	158	142	0	0	342	14	91	4	0	0	109	1017
5:30 PM	9	123	43	0	0	175	150	186	43	0	0	379	60	196	150	0	0	406	19	99	2	0	0	120	1080
5:45 PM	10	104	31	0	0	145	147	144	47	0	0	338	45	182	147	0	0	374	18	88	4	0	1	110	967



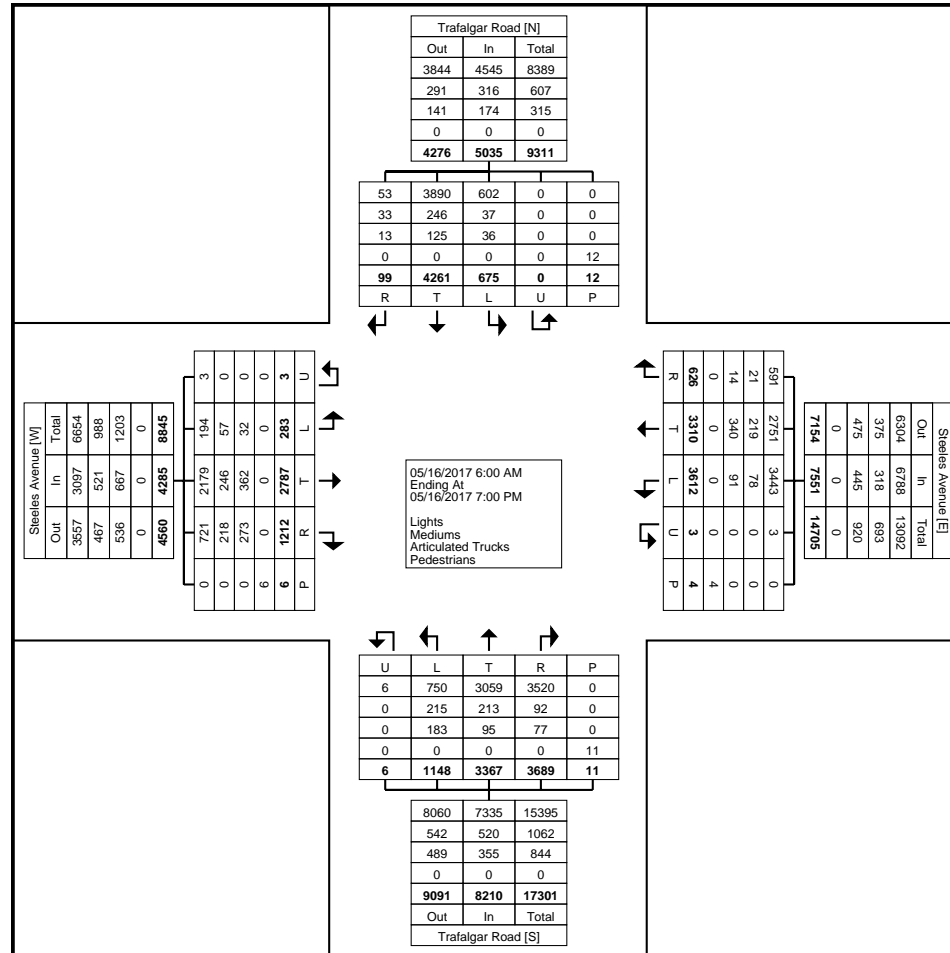
Hourly Total	54	491	185	0	1	730	610	655	157	0	0	1422	199	693	606	0	1	1498	70	377	15	0	1	462	4112
6:00 PM	10	66	47	0	0	123	144	171	28	0	0	343	37	169	155	0	0	361	7	81	4	0	0	92	919
6:15 PM	8	65	67	0	0	140	122	150	27	0	0	299	47	142	163	0	0	352	19	84	1	0	1	104	895
6:30 PM	5	52	35	0	0	92	112	114	29	0	0	255	31	140	114	0	0	285	10	66	2	0	1	78	710
6:45 PM	4	60	18	0	0	82	122	119	13	0	0	254	29	110	139	0	0	278	7	89	4	0	0	100	714
Hourly Total	27	243	167	0	0	437	500	554	97	0	0	1151	144	561	571	0	0	1276	43	320	11	0	2	374	3238
Grand Total	283	2787	1212	3	6	4285	3612	3310	626	3	4	7551	1148	3367	3689	6	11	8210	675	4261	99	0	12	5035	25081
Approach %	6.6	65.0	28.3	0.1	-	-	47.8	43.8	8.3	0.0	-	-	14.0	41.0	44.9	0.1	-	-	13.4	84.6	2.0	0.0	-	-	-
Total %	1.1	11.1	4.8	0.0	-	17.1	14.4	13.2	2.5	0.0	-	30.1	4.6	13.4	14.7	0.0	-	32.7	2.7	17.0	0.4	0.0	-	20.1	-
Lights	194	2179	721	3	-	3097	3443	2751	591	3	-	6788	750	3059	3520	6	-	7335	602	3890	53	0	-	4545	21765
% Lights	68.6	78.2	59.5	100.0	-	72.3	95.3	83.1	94.4	100.0	-	89.9	65.3	90.9	95.4	100.0	-	89.3	89.2	91.3	53.5	-	-	90.3	86.8
Mediums	57	246	218	0	-	521	78	219	21	0	-	318	215	213	92	0	-	520	37	246	33	0	-	316	1675
% Mediums	20.1	8.8	18.0	0.0	-	12.2	2.2	6.6	3.4	0.0	-	4.2	18.7	6.3	2.5	0.0	-	6.3	5.5	5.8	33.3	-	-	6.3	6.7
Articulated Trucks	32	362	273	0	-	667	91	340	14	0	-	445	183	95	77	0	-	355	36	125	13	0	-	174	1641
% Articulated Trucks	11.3	13.0	22.5	0.0	-	15.6	2.5	10.3	2.2	0.0	-	5.9	15.9	2.8	2.1	0.0	-	4.3	5.3	2.9	13.1	-	-	3.5	6.5
Pedestrians	-	-	-	-	6	-	-	-	-	-	4	-	-	-	-	-	11	-	-	-	-	-	12	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Turning Movement Data Plot



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Count Name: Steeles Avenue & Trafalgar Road -  
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### Turning Movement Peak Hour Data (7:30 AM)

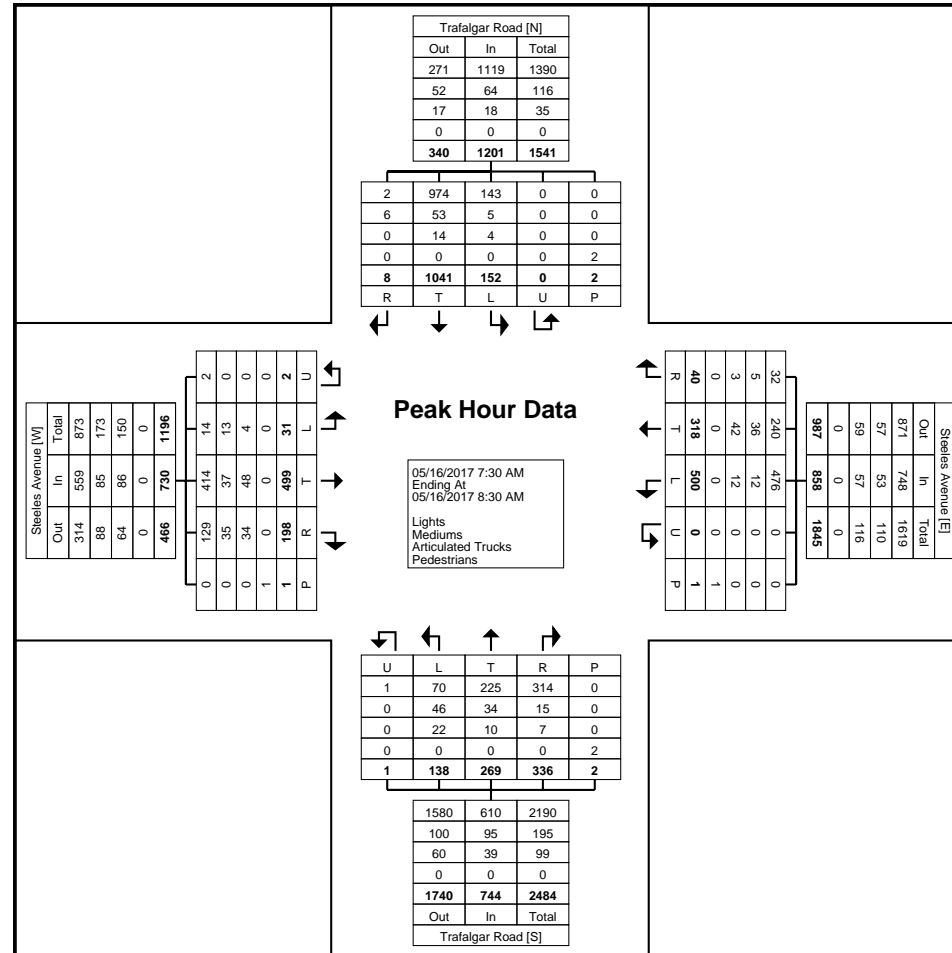
Start Time	Steeles Avenue Eastbound						Steeles Avenue Westbound						Trafalgar Road Northbound						Trafalgar Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	6	124	42	0	0	172	112	80	7	0	0	199	41	87	77	1	0	206	37	272	3	0	0	312	889
7:45 AM	8	119	59	2	1	188	133	92	5	0	0	230	28	61	84	0	2	173	43	228	4	0	1	275	866
8:00 AM	11	127	51	0	0	189	135	74	16	0	0	225	36	60	101	0	0	197	40	270	1	0	0	311	922
8:15 AM	6	129	46	0	0	181	120	72	12	0	1	204	33	61	74	0	0	168	32	271	0	0	1	303	856
Total	31	499	198	2	1	730	500	318	40	0	1	858	138	269	336	1	2	744	152	1041	8	0	2	1201	3533
Approach %	4.2	68.4	27.1	0.3	-	-	58.3	37.1	4.7	0.0	-	-	18.5	36.2	45.2	0.1	-	-	12.7	86.7	0.7	0.0	-	-	-
Total %	0.9	14.1	5.6	0.1	-	20.7	14.2	9.0	1.1	0.0	-	24.3	3.9	7.6	9.5	0.0	-	21.1	4.3	29.5	0.2	0.0	-	34.0	-
PHF	0.705	0.967	0.839	0.250	-	0.966	0.926	0.864	0.625	0.000	-	0.933	0.841	0.773	0.832	0.250	-	0.903	0.884	0.957	0.500	0.000	-	0.962	0.958
Lights	14	414	129	2	-	559	476	240	32	0	-	748	70	225	314	1	-	610	143	974	2	0	-	1119	3036
% Lights	45.2	83.0	65.2	100.0	-	76.6	95.2	75.5	80.0	-	-	87.2	50.7	83.6	93.5	100.0	-	82.0	94.1	93.6	25.0	-	-	93.2	85.9
Mediums	13	37	35	0	-	85	12	36	5	0	-	53	46	34	15	0	-	95	5	53	6	0	-	64	297
% Mediums	41.9	7.4	17.7	0.0	-	11.6	2.4	11.3	12.5	-	-	6.2	33.3	12.6	4.5	0.0	-	12.8	3.3	5.1	75.0	-	-	5.3	8.4
Articulated Trucks	4	48	34	0	-	86	12	42	3	0	-	57	22	10	7	0	-	39	4	14	0	0	-	18	200
% Articulated Trucks	12.9	9.6	17.2	0.0	-	11.8	2.4	13.2	7.5	-	-	6.6	15.9	3.7	2.1	0.0	-	5.2	2.6	1.3	0.0	-	-	1.5	5.7
Pedestrians	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	2	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Turning Movement Peak Hour Data Plot (7:30 AM)

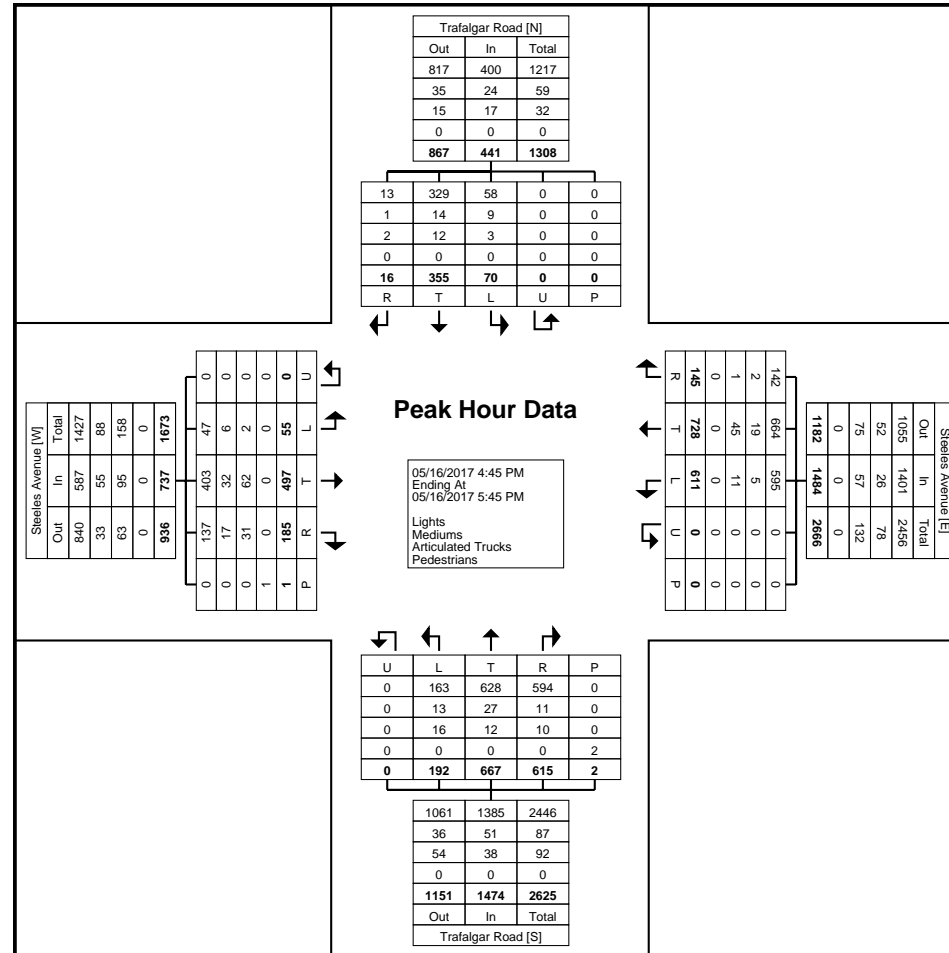




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Count Name: Steeles Avenue & Trafalgar Road -  
Weekday  
Site Code:  
Start Date: 05/16/2017  
Page No: 7



Turning Movement Peak Hour Data Plot (4:45 PM)



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Count Name: Steeles Avenue & Trafalgar Road -  
Weekday  
Site Code:  
Start Date: 05/16/2017  
Page No: 8



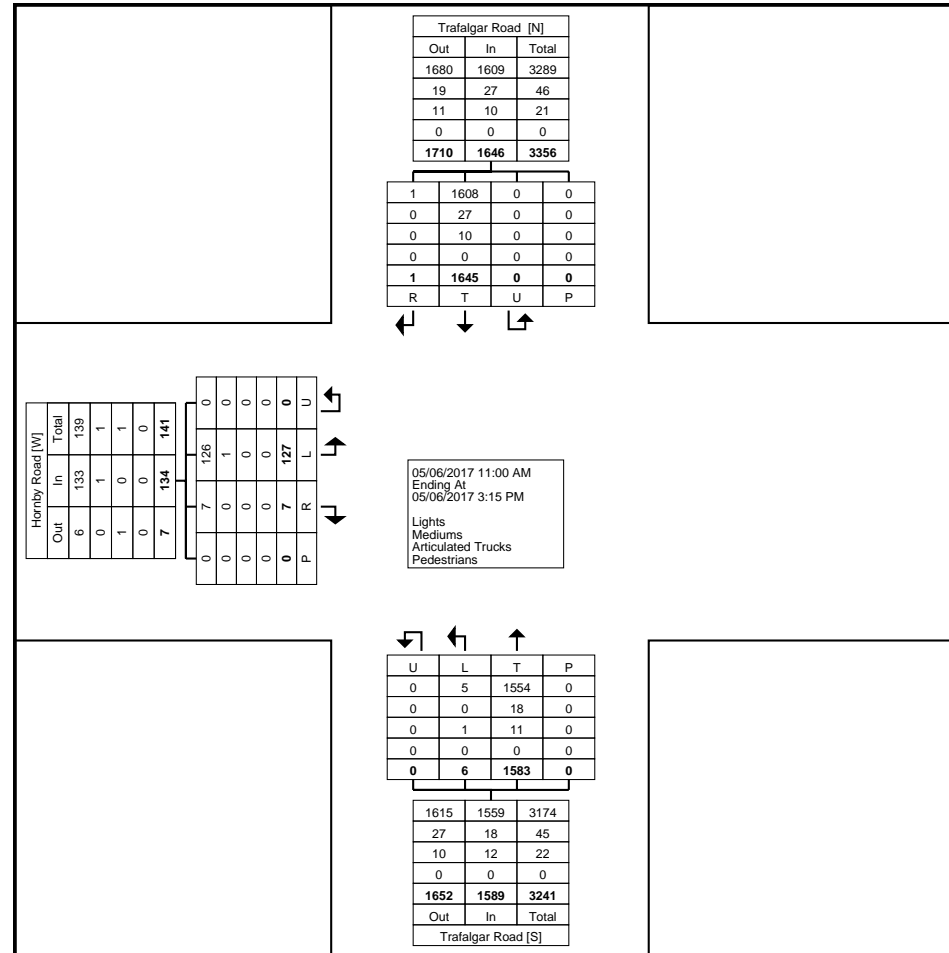




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Count Name: Trafalgar Road & Hornby Road - Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 2



Turning Movement Data Plot

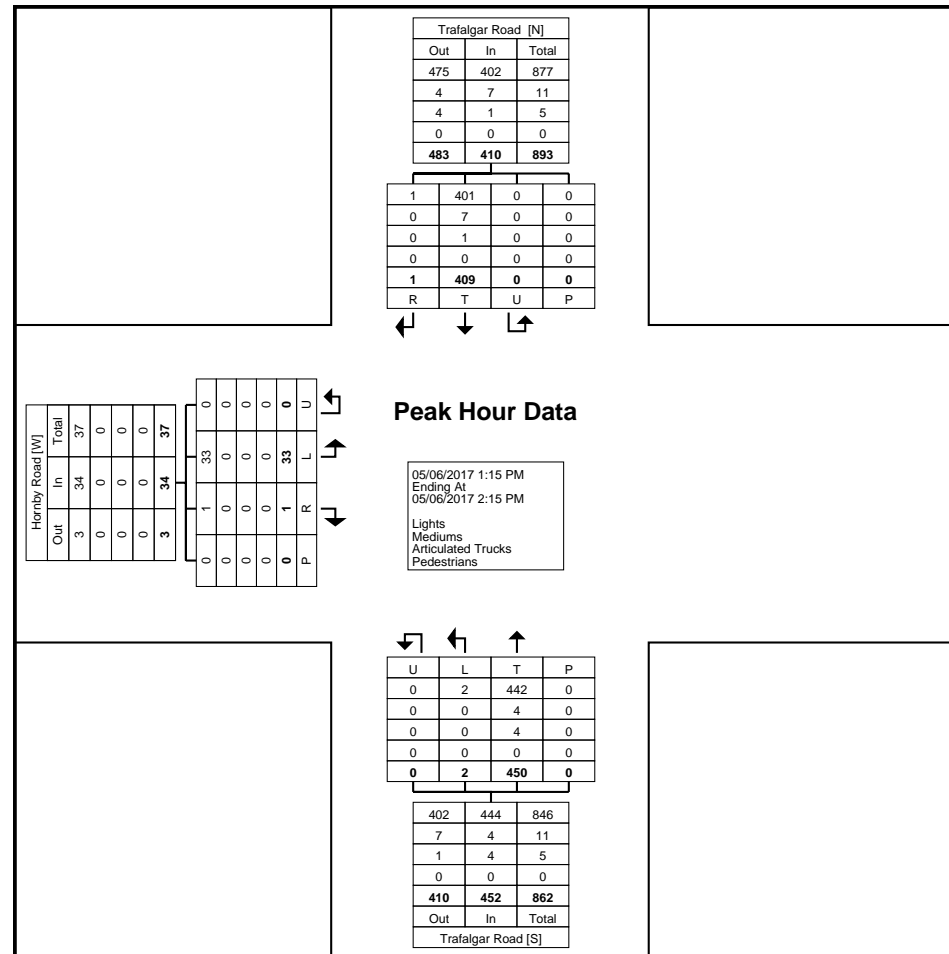




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Count Name: Trafalgar Road & Hornby Road -  
Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 4



Turning Movement Peak Hour Data Plot (1:15 PM)



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Count Name: Trafalgar Road & Hornby Road -  
Saturday  
Site Code:  
Start Date: 05/06/2017  
Page No: 5



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Waterloo, Ontario, Canada N2J 1N8  
519-896-3163 cbowness@ptsl.com

Count Name: Trafalgar Road & Hornby Road -  
Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 1

### Turning Movement Data

Start Time	Hornby Road Eastbound					Trafalgar Road Northbound					Trafalgar Road Southbound					Int. Total
	Left	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	
6:00 AM	1	1	0	0	2	0	20	0	0	20	127	0	0	0	127	149
6:15 AM	2	0	0	0	2	1	42	0	0	43	167	0	0	0	167	212
6:30 AM	5	2	0	0	7	0	49	0	0	49	240	0	0	0	240	296
6:45 AM	3	0	0	0	3	0	65	0	0	65	183	0	0	0	183	251
Hourly Total	11	3	0	0	14	1	176	0	0	177	717	0	0	0	717	908
7:00 AM	0	3	0	2	3	4	49	0	0	53	216	0	0	2	216	272
7:15 AM	8	1	0	1	9	0	69	0	0	69	249	0	0	0	249	327
7:30 AM	2	1	0	0	3	0	72	0	0	72	263	1	0	1	264	339
7:45 AM	4	3	0	0	7	0	61	0	0	61	283	0	0	1	283	351
Hourly Total	14	8	0	3	22	4	251	0	0	255	1011	1	0	4	1012	1289
8:00 AM	3	2	0	0	5	1	73	0	0	74	277	0	0	0	277	356
8:15 AM	3	0	0	0	3	1	86	0	0	87	265	0	0	0	265	355
8:30 AM	6	2	0	0	8	0	74	0	0	74	183	1	0	0	184	266
8:45 AM	12	0	0	0	12	2	63	0	1	65	188	0	0	0	188	265
Hourly Total	24	4	0	0	28	4	296	0	1	300	913	1	0	0	914	1242
9:00 AM	4	2	0	0	6	0	61	0	0	61	162	1	0	0	163	230
9:15 AM	4	1	0	0	5	0	69	0	0	69	134	0	0	2	134	208
9:30 AM	7	3	0	0	10	0	77	0	0	77	132	0	0	0	132	219
9:45 AM	2	1	0	0	3	0	70	0	0	70	103	0	0	0	103	176
Hourly Total	17	7	0	0	24	0	277	0	0	277	531	1	0	2	532	833
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	5	2	0	2	7	1	132	0	0	133	86	0	0	1	86	226
3:15 PM	10	0	0	0	10	1	154	0	0	155	76	0	0	0	76	241
3:30 PM	4	0	0	0	4	0	161	0	0	161	62	0	0	0	62	227
3:45 PM	4	0	0	0	4	0	143	0	0	143	58	0	0	0	58	205
Hourly Total	23	2	0	2	25	2	590	0	0	592	282	0	0	1	282	899
4:00 PM	9	2	0	0	11	1	181	0	0	182	75	0	0	0	75	268
4:15 PM	7	0	0	0	7	1	151	0	0	152	54	0	0	0	54	213
4:30 PM	9	0	0	0	9	0	190	0	0	190	73	0	0	0	73	272
4:45 PM	8	2	0	0	10	0	190	0	0	190	69	0	0	0	69	269
Hourly Total	33	4	0	0	37	2	712	0	0	714	271	0	0	0	271	1022
5:00 PM	14	1	0	0	15	0	202	0	0	202	64	0	0	0	64	281
5:15 PM	5	1	0	0	6	2	185	0	0	187	74	0	0	0	74	267
5:30 PM	6	2	0	0	8	1	199	0	0	200	82	0	0	0	82	290
5:45 PM	6	1	0	0	7	0	190	0	0	190	73	0	0	0	73	270
Hourly Total	31	5	0	0	36	3	776	0	0	779	293	0	0	0	293	1108

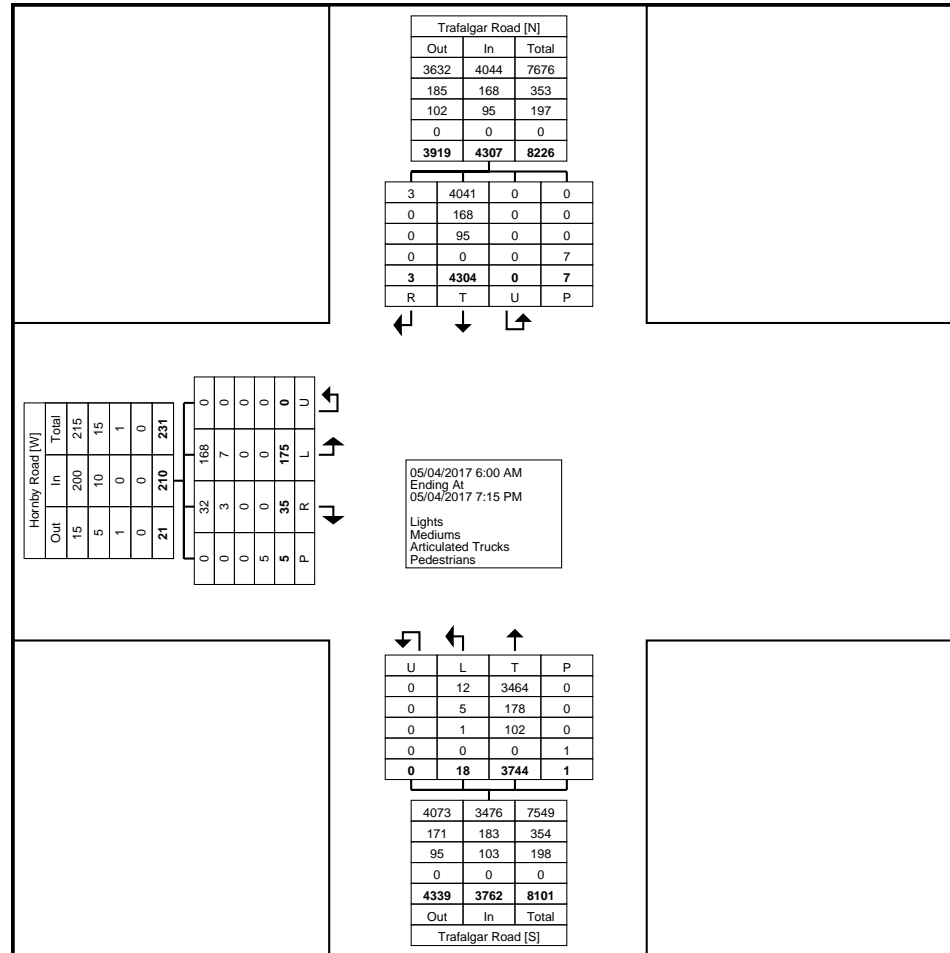
6:00 PM	5	1	0	0	6	1	188	0	0	189	73	0	0	0	73	268
6:15 PM	10	1	0	0	11	1	143	0	0	144	83	0	0	0	83	238
6:30 PM	5	0	0	0	5	0	193	0	0	193	64	0	0	0	64	262
6:45 PM	2	0	0	0	2	0	142	0	0	142	66	0	0	0	66	210
Hourly Total	22	2	0	0	24	2	666	0	0	668	286	0	0	0	286	978
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	175	35	0	5	210	18	3744	0	1	3762	4304	3	0	7	4307	8279
Approach %	83.3	16.7	0.0	-	-	0.5	99.5	0.0	-	-	99.9	0.1	0.0	-	-	-
Total %	2.1	0.4	0.0	-	2.5	0.2	45.2	0.0	-	45.4	52.0	0.0	0.0	-	52.0	-
Lights	168	32	0	-	200	12	3464	0	-	3476	4041	3	0	-	4044	7720
% Lights	96.0	91.4	-	-	95.2	66.7	92.5	-	-	92.4	93.9	100.0	-	-	93.9	93.2
Mediums	7	3	0	-	10	5	178	0	-	183	168	0	0	-	168	361
% Mediums	4.0	8.6	-	-	4.8	27.8	4.8	-	-	4.9	3.9	0.0	-	-	3.9	4.4
Articulated Trucks	0	0	0	-	0	1	102	0	-	103	95	0	0	-	95	198
% Articulated Trucks	0.0	0.0	-	-	0.0	5.6	2.7	-	-	2.7	2.2	0.0	-	-	2.2	2.4
Pedestrians	-	-	-	5	-	-	-	-	1	-	-	-	-	7	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-



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Count Name: Trafalgar Road & Hornby Road -  
Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 3



Turning Movement Data Plot



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519-896-3163 cbowness@pts.com

Count Name: Trafalgar Road & Hornby Road -  
Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 4

### Turning Movement Peak Hour Data (7:30 AM)

Start Time	Hornby Road Eastbound					Trafalgar Road Northbound					Trafalgar Road Southbound					Int. Total
	Left	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	2	1	0	0	3	0	72	0	0	72	263	1	0	1	264	339
7:45 AM	4	3	0	0	7	0	61	0	0	61	283	0	0	1	283	351
8:00 AM	3	2	0	0	5	1	73	0	0	74	277	0	0	0	277	356
8:15 AM	3	0	0	0	3	1	86	0	0	87	265	0	0	0	265	355
Total	12	6	0	0	18	2	292	0	0	294	1088	1	0	2	1089	1401
Approach %	66.7	33.3	0.0	-	-	0.7	99.3	0.0	-	-	99.9	0.1	0.0	-	-	-
Total %	0.9	0.4	0.0	-	1.3	0.1	20.8	0.0	-	21.0	77.7	0.1	0.0	-	77.7	-
PHF	0.750	0.500	0.000	-	0.643	0.500	0.849	0.000	-	0.845	0.961	0.250	0.000	-	0.962	0.984
Lights	9	5	0	-	14	1	244	0	-	245	1036	1	0	-	1037	1296
% Lights	75.0	83.3	-	-	77.8	50.0	83.6	-	-	83.3	95.2	100.0	-	-	95.2	92.5
Mediums	3	1	0	-	4	1	37	0	-	38	36	0	0	-	36	78
% Mediums	25.0	16.7	-	-	22.2	50.0	12.7	-	-	12.9	3.3	0.0	-	-	3.3	5.6
Articulated Trucks	0	0	0	-	0	0	11	0	-	11	16	0	0	-	16	27
% Articulated Trucks	0.0	0.0	-	-	0.0	0.0	3.8	-	-	3.7	1.5	0.0	-	-	1.5	1.9
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-

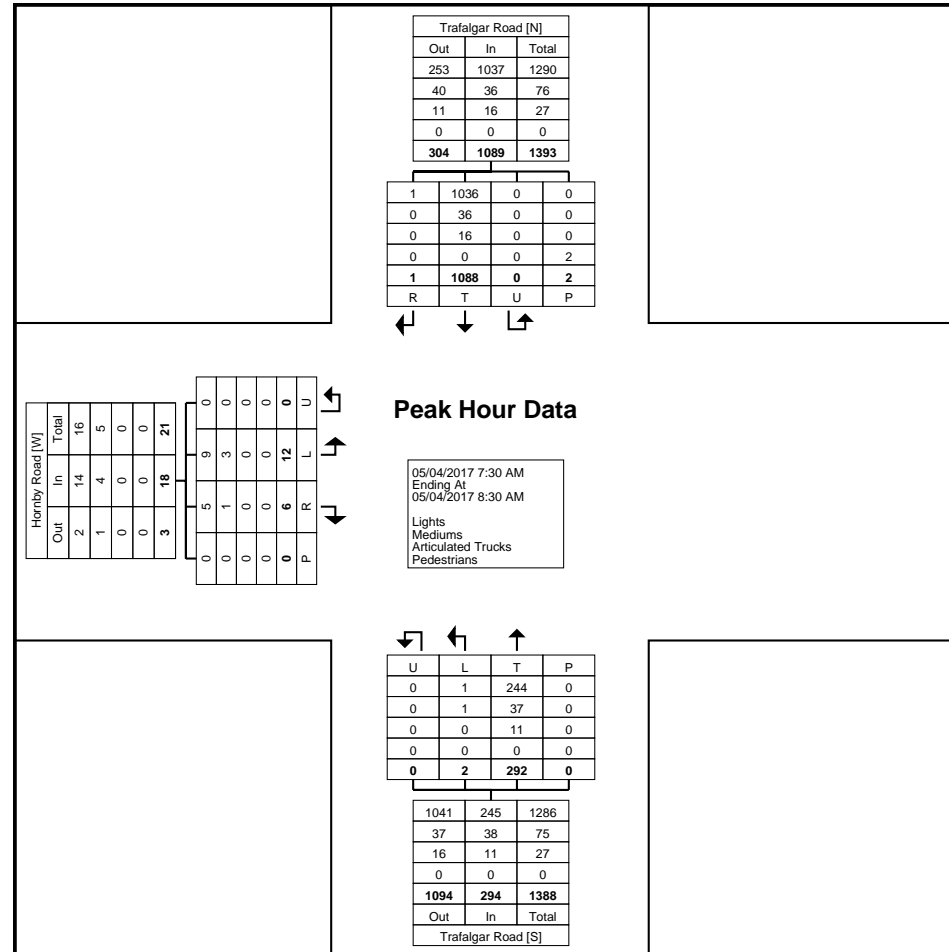




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Count Name: Trafalgar Road & Hornby Road -  
Weekday  
Site Code:  
Start Date: 05/04/2017  
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Turning Movement Peak Hour Data Plot (7:30 AM)

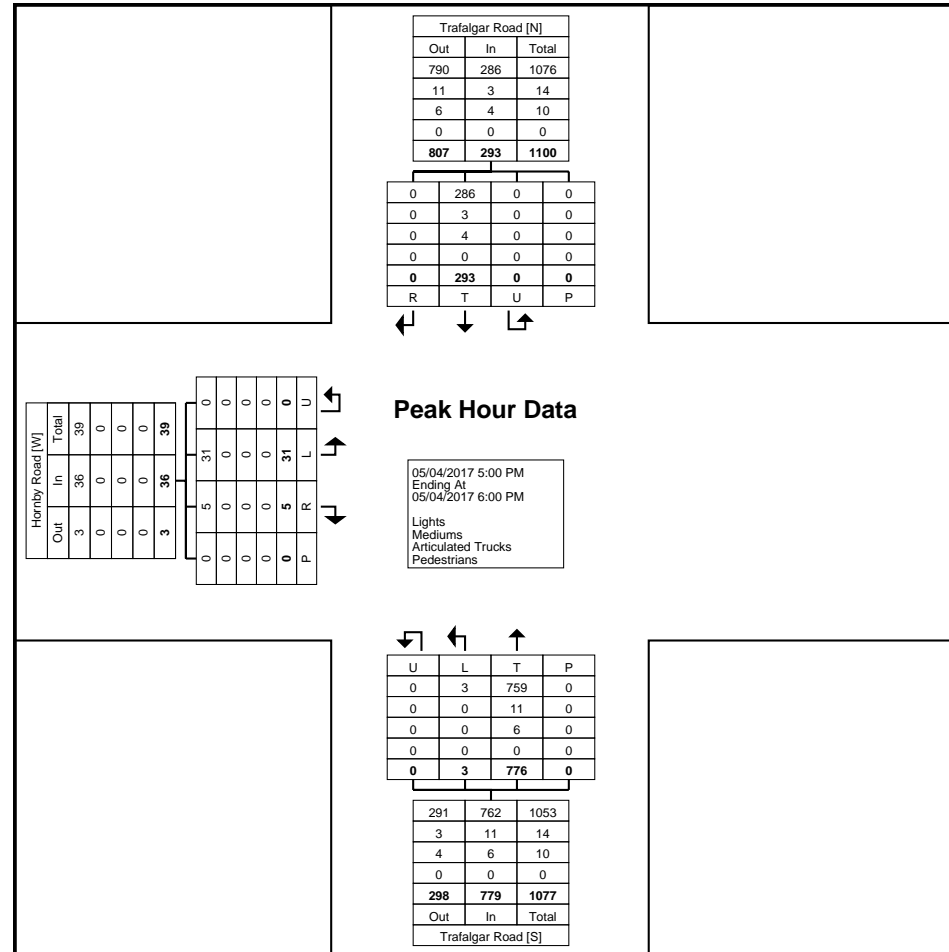




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Weekday  
Site Code:  
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Turning Movement Peak Hour Data Plot (5:00 PM)



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Count Name: Trafalgar Road & Hornby Road -  
Weekday  
Site Code:  
Start Date: 05/04/2017  
Page No: 8

# Appendix C

## Existing Traffic Operations Reports



HCM Unsignalized Intersection Capacity Analysis  
1: Fifth Line & 5 Side Road

Base Year - AM Peak Hour  
Premier Gateway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	10	563	11	22	160	3	4	20	19	36	46	25
Future Volume (Veh/h)	10	563	11	22	160	3	4	20	19	36	46	25
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	10	586	11	23	167	3	4	21	20	38	48	26
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	170			597			876	828	592	856	832	168
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	170			597			876	828	592	856	832	168
tC, single (s)	4.2			4.2			7.3	6.7	6.3	7.2	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.3			2.3			3.7	4.2	3.4	3.6	4.0	3.3
p0 queue free %	99			98			98	92	96	84	84	97
cM capacity (veh/h)	1366			946			204	279	492	239	293	881
<b>Direction, Lane #</b>	<b>EB 1</b>	<b>WB 1</b>	<b>NB 1</b>	<b>SB 1</b>								
Volume Total	607	193	45	112								
Volume Left	10	23	4	38								
Volume Right	11	3	20	26								
cSH	1366	946	332	318								
Volume to Capacity	0.01	0.02	0.14	0.35								
Queue Length 95th (m)	0.2	0.6	3.7	12.3								
Control Delay (s)	0.2	1.3	17.5	22.3								
Lane LOS	A	A	C	C								
Approach Delay (s)	0.2	1.3	17.5	22.3								
Approach LOS			C	C								
<b>Intersection Summary</b>												
Average Delay			3.8									
Intersection Capacity Utilization			51.4%		ICU Level of Service				A			
Analysis Period (min)			15									

# HCM Unsignalized Intersection Capacity Analysis

## 2: Sixth Line & 5 Side Road

Base Year - AM Peak Hour  
Premier Gateway



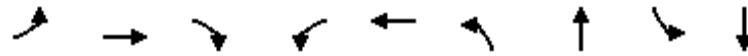
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	7	595	1	8	171	3	1	9	30	20	17	16
Future Volume (Veh/h)	7	595	1	8	171	3	1	9	30	20	17	16
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	7	620	1	8	178	3	1	9	31	21	18	17
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	181			621			856	832	620	866	830	180
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	181			621			856	832	620	866	830	180
tC, single (s)	4.2			4.1			7.1	6.5	6.4	7.2	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.3			2.2			3.5	4.0	3.4	3.6	4.0	3.3
p0 queue free %	99			99			100	97	93	91	94	98
cM capacity (veh/h)	1331			969			259	303	463	239	304	869
<b>Direction, Lane #</b>	<b>EB 1</b>	<b>WB 1</b>	<b>NB 1</b>	<b>SB 1</b>								
Volume Total	628	189	41	56								
Volume Left	7	8	1	21								
Volume Right	1	3	31	17								
cSH	1331	969	408	336								
Volume to Capacity	0.01	0.01	0.10	0.17								
Queue Length 95th (m)	0.1	0.2	2.7	4.7								
Control Delay (s)	0.1	0.4	14.8	17.8								
Lane LOS	A	A	B	C								
Approach Delay (s)	0.1	0.4	14.8	17.8								
Approach LOS			B	C								
<b>Intersection Summary</b>												
Average Delay			2.0									
Intersection Capacity Utilization			50.1%		ICU Level of Service				A			
Analysis Period (min)			15									

## Queues

Base Year - AM Peak Hour

## 3: Trafalgar Rd &amp; 5 Side Road

Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	50	350	253	59	135	16	377	28	986
v/c Ratio	0.16	0.73	0.46	0.44	0.29	0.05	0.24	0.06	0.53
Control Delay	28.6	41.5	9.9	40.9	29.1	8.0	12.8	7.9	15.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	28.6	41.5	9.9	40.9	29.1	8.0	12.8	7.9	15.5
Queue Length 50th (m)	7.7	63.6	6.9	9.8	21.0	1.1	21.1	2.0	54.2
Queue Length 95th (m)	17.3	95.6	27.5	23.2	37.2	3.8	31.9	5.6	96.2
Internal Link Dist (m)		593.5			641.2		240.1		238.0
Turn Bay Length (m)	40.0		40.0	40.0		40.0		50.0	
Base Capacity (vph)	379	587	623	166	571	300	1579	506	1866
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.60	0.41	0.36	0.24	0.05	0.24	0.06	0.53

## Intersection Summary



# HCM Signalized Intersection Capacity Analysis

## 3: Trafalgar Rd & 5 Side Road

Base Year - AM Peak Hour

Premier Gateway



















Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	49	343	248	58	124	8	16	328	41	27	924	42
Future Volume (vph)	49	343	248	58	124	8	16	328	41	27	924	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.4	6.4	6.4	6.4	6.4		4.0	6.0		4.0	6.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	1.00	0.85	1.00	0.99		1.00	0.98		1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1703	1863	1524	1656	1806		1456	2907		1378	3314	
Flt Permitted	0.67	1.00	1.00	0.30	1.00		0.23	1.00		0.51	1.00	
Satd. Flow (perm)	1202	1863	1524	528	1806		358	2907		747	3314	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	50	350	253	59	127	8	16	335	42	28	943	43
RTOR Reduction (vph)	0	0	156	0	2	0	0	9	0	0	3	0
Lane Group Flow (vph)	50	350	97	59	133	0	16	368	0	28	983	0
Heavy Vehicles (%)	6%	2%	6%	9%	1%	56%	24%	23%	15%	31%	8%	13%
Turn Type	Perm	NA	Perm	Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8			2			6		
Actuated Green, G (s)	23.6	23.6	23.6	23.6	23.6		52.9	50.4		55.5	51.7	
Effective Green, g (s)	23.6	23.6	23.6	23.6	23.6		52.9	50.4		55.5	51.7	
Actuated g/C Ratio	0.25	0.25	0.25	0.25	0.25		0.56	0.54		0.59	0.55	
Clearance Time (s)	6.4	6.4	6.4	6.4	6.4		4.0	6.0		4.0	6.0	
Vehicle Extension (s)	5.0	5.0	5.0	5.0	5.0		3.0	5.0		3.0	5.0	
Lane Grp Cap (vph)	301	466	381	132	452		230	1555		465	1818	
v/s Ratio Prot		c0.19			0.07		0.00	0.13		c0.00	c0.30	
v/s Ratio Perm	0.04		0.06	0.11			0.04			0.03		
v/c Ratio	0.17	0.75	0.25	0.45	0.29		0.07	0.24		0.06	0.54	
Uniform Delay, d1	27.6	32.6	28.3	29.8	28.6		9.7	11.7		8.1	13.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.5	7.9	0.7	5.0	0.8		0.1	0.4		0.1	1.2	
Delay (s)	28.2	40.5	29.0	34.8	29.3		9.8	12.0		8.2	14.8	
Level of Service	C	D	C	C	C		A	B		A	B	
Approach Delay (s)		35.1			31.0			11.9			14.6	
Approach LOS		D			C			B			B	

Intersection Summary			
HCM 2000 Control Delay	21.5	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.59		
Actuated Cycle Length (s)	94.2	Sum of lost time (s)	16.4
Intersection Capacity Utilization	73.1%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

HCM Unsignalized Intersection Capacity Analysis  
4: Eighth Line & 5 Side Road

Base Year - AM Peak Hour  
Premier Gateway

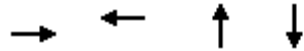
												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	24	365	9	24	113	12	1	81	22	65	395	67
Future Volume (vph)	24	365	9	24	113	12	1	81	22	65	395	67
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	25	380	9	25	118	13	1	84	23	68	411	70
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	414	156	108	549								
Volume Left (vph)	25	25	1	68								
Volume Right (vph)	9	13	23	70								
Hadj (s)	0.04	0.08	-0.03	-0.02								
Departure Headway (s)	6.5	7.2	7.1	6.1								
Degree Utilization, x	0.74	0.31	0.21	0.93								
Capacity (veh/h)	541	471	450	585								
Control Delay (s)	26.0	13.4	12.1	45.4								
Approach Delay (s)	26.0	13.4	12.1	45.4								
Approach LOS	D	B	B	E								
Intersection Summary												
Delay			31.8									
Level of Service			D									
Intersection Capacity Utilization			65.1%	ICU Level of Service	C							
Analysis Period (min)			15									

## Queues

Base Year - AM Peak Hour

## 5: Ninth Line &amp; 5 Side Road

Premier Gateway



Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	462	130	209	706
v/c Ratio	0.82	0.23	0.24	0.93
Control Delay	36.8	18.2	11.4	39.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	36.8	18.2	11.4	39.3
Queue Length 50th (m)	62.0	13.1	15.8	93.5
Queue Length 95th (m)	97.0	25.4	31.6	#185.5
Internal Link Dist (m)	556.9	434.3	3096.2	305.9
Turn Bay Length (m)				
Base Capacity (vph)	688	691	874	762
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.67	0.19	0.24	0.93

## Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 5: Ninth Line & 5 Side Road

Base Year - AM Peak Hour  
Premier Gateway



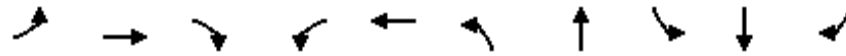
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	24	396	19	1	110	12	5	181	12	259	386	26
Future Volume (vph)	24	396	19	1	110	12	5	181	12	259	386	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0			6.0			6.0			6.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.99			0.99			0.99			0.99	
Flt Protected		1.00			1.00			1.00			0.98	
Satd. Flow (prot)		1807			1778			1681			1790	
Flt Permitted		0.98			1.00			0.99			0.79	
Satd. Flow (perm)		1774			1774			1660			1450	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	25	417	20	1	116	13	5	191	13	273	406	27
RTOR Reduction (vph)	0	2	0	0	5	0	0	3	0	0	1	0
Lane Group Flow (vph)	0	460	0	0	125	0	0	206	0	0	705	0
Heavy Vehicles (%)	8%	4%	5%	0%	6%	0%	40%	12%	0%	0%	6%	4%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		23.7			23.7			39.5			39.5	
Effective Green, g (s)		23.7			23.7			39.5			39.5	
Actuated g/C Ratio		0.32			0.32			0.53			0.53	
Clearance Time (s)		6.0			6.0			6.0			6.0	
Vehicle Extension (s)		3.5			3.5			5.5			5.5	
Lane Grp Cap (vph)		559			559			871			761	
v/s Ratio Prot												
v/s Ratio Perm		c0.26			0.07			0.12			c0.49	
v/c Ratio		0.82			0.22			0.24			0.93	
Uniform Delay, d1		23.8			19.0			9.7			16.5	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		9.8			0.2			0.6			18.9	
Delay (s)		33.6			19.2			10.3			35.4	
Level of Service		C			B			B			D	
Approach Delay (s)		33.6			19.2			10.3			35.4	
Approach LOS		C			B			B			D	

### Intersection Summary

HCM 2000 Control Delay	29.9	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.89		
Actuated Cycle Length (s)	75.2	Sum of lost time (s)	12.0
Intersection Capacity Utilization	102.8%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
6: Brownridge Road/Fifth Line & Steeles Avenue

Base Year - AM Peak Hour  
Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	53	786	141	17	515	9	5	21	6	66
v/c Ratio	0.09	0.35	0.12	0.04	0.26	0.06	0.01	0.16	0.02	0.26
Control Delay	4.5	5.0	1.1	4.3	4.3	29.2	0.0	32.5	28.3	10.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	4.5	5.0	1.1	4.3	4.3	29.2	0.0	32.5	28.3	10.8
Queue Length 50th (m)	2.3	22.5	0.0	0.7	12.8	1.4	0.0	3.2	0.9	0.0
Queue Length 95th (m)	5.8	31.8	4.7	2.7	19.3	5.3	0.0	9.1	4.0	10.2
Internal Link Dist (m)		462.3			679.6		261.2		67.4	
Turn Bay Length (m)	145.0		65.0	30.0		20.0		25.0		25.0
Base Capacity (vph)	615	2239	1198	410	1988	268	513	220	417	378
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.35	0.12	0.04	0.26	0.03	0.01	0.10	0.01	0.17

Intersection Summary

# HCM Signalized Intersection Capacity Analysis

## 6: Brownridge Road/Fifth Line & Steeles Avenue

Base Year - AM Peak Hour  
Premier Gateway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	51	755	135	16	454	40	9	0	5	20	6	63
Future Volume (vph)	51	755	135	16	454	40	9	0	5	20	6	63
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.0	8.0	8.0	8.0	8.0		6.0	6.0		6.0	6.0	6.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		1.00	1.00		1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	0.99		1.00	0.85		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1671	2959	1538	1456	2622		1543	1615		1262	1900	1482
Flt Permitted	0.46	1.00	1.00	0.35	1.00		0.75	1.00		0.75	1.00	1.00
Satd. Flow (perm)	813	2959	1538	543	2622		1224	1615		1002	1900	1482
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	53	786	141	17	473	42	9	0	5	21	6	66
RTOR Reduction (vph)	0	0	39	0	6	0	0	5	0	0	0	59
Lane Group Flow (vph)	53	786	102	17	509	0	9	0	0	21	6	7
Heavy Vehicles (%)	8%	22%	5%	24%	37%	25%	17%	0%	0%	43%	0%	9%
Turn Type	Perm	NA	Perm	Perm	NA		Perm	NA		Perm	NA	Perm
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8			2			6		6
Actuated Green, G (s)	57.2	57.2	57.2	57.2	57.2		7.9	7.9		7.9	7.9	7.9
Effective Green, g (s)	57.2	57.2	57.2	57.2	57.2		7.9	7.9		7.9	7.9	7.9
Actuated g/C Ratio	0.72	0.72	0.72	0.72	0.72		0.10	0.10		0.10	0.10	0.10
Clearance Time (s)	8.0	8.0	8.0	8.0	8.0		6.0	6.0		6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	587	2139	1112	392	1896		122	161		100	189	148
v/s Ratio Prot		c0.27			0.19			0.00				0.00
v/s Ratio Perm	0.07		0.07	0.03			0.01			c0.02		0.00
v/c Ratio	0.09	0.37	0.09	0.04	0.27		0.07	0.00		0.21	0.03	0.04
Uniform Delay, d1	3.2	4.1	3.2	3.1	3.8		32.3	32.1		32.7	32.1	32.2
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.3	0.5	0.2	0.2	0.3		0.3	0.0		1.0	0.1	0.1
Delay (s)	3.5	4.6	3.4	3.3	4.1		32.5	32.1		33.8	32.2	32.3
Level of Service	A	A	A	A	A		C	C		C	C	C
Approach Delay (s)		4.4			4.1			32.4			32.6	
Approach LOS		A			A			C			C	

### Intersection Summary

HCM 2000 Control Delay	6.2	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.35		
Actuated Cycle Length (s)	79.1	Sum of lost time (s)	14.0
Intersection Capacity Utilization	68.3%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
7: Fifth Line South & Steeles Avenue

Base Year - AM Peak Hour  
Premier Gateway



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	807	22	6	536	6	3
v/c Ratio	0.29	0.01	0.01	0.21	0.03	0.01
Control Delay	1.5	1.0	1.8	1.4	33.2	23.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	1.5	1.0	1.8	1.4	33.2	23.7
Queue Length 50th (m)	0.0	0.0	0.0	0.0	0.8	0.0
Queue Length 95th (m)	31.5	1.7	1.2	19.9	4.8	2.7
Internal Link Dist (m)	679.6			455.7	532.9	
Turn Bay Length (m)		30.0	60.0		15.0	
Base Capacity (vph)	2780	1472	624	2514	278	293
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.01	0.01	0.21	0.02	0.01

Intersection Summary

# HCM Signalized Intersection Capacity Analysis

## 7: Fifth Line South & Steeles Avenue

Base Year - AM Peak Hour  
Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑
Traffic Volume (vph)	759	21	6	504	6	3
Future Volume (vph)	759	21	6	504	6	3
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.0	8.0	8.0	8.0	6.0	6.0
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	2935	1553	1805	2654	1543	1615
Flt Permitted	1.00	1.00	0.35	1.00	0.95	1.00
Satd. Flow (perm)	2935	1553	659	2654	1543	1615
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	807	22	6	536	6	3
RTOR Reduction (vph)	0	4	0	0	0	3
Lane Group Flow (vph)	807	18	6	536	6	0
Heavy Vehicles (%)	23%	4%	0%	36%	17%	0%
Turn Type	NA	Perm	Perm	NA	Perm	Perm
Protected Phases	4			8		
Permitted Phases		4	8		2	2
Actuated Green, G (s)	67.4	67.4	67.4	67.4	1.7	1.7
Effective Green, g (s)	67.4	67.4	67.4	67.4	1.7	1.7
Actuated g/C Ratio	0.81	0.81	0.81	0.81	0.02	0.02
Clearance Time (s)	8.0	8.0	8.0	8.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	2380	1259	534	2152	31	33
v/s Ratio Prot	c0.27			0.20		
v/s Ratio Perm		0.01	0.01		c0.00	0.00
v/c Ratio	0.34	0.01	0.01	0.25	0.19	0.00
Uniform Delay, d1	2.0	1.5	1.5	1.9	40.0	39.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.4	0.0	0.0	0.3	3.0	0.0
Delay (s)	2.4	1.5	1.5	2.1	43.1	39.9
Level of Service	A	A	A	A	D	D
Approach Delay (s)	2.4			2.1	42.0	
Approach LOS	A			A	D	

### Intersection Summary

HCM 2000 Control Delay	2.6	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.34		
Actuated Cycle Length (s)	83.1	Sum of lost time (s)	14.0
Intersection Capacity Utilization	55.0%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			



# HCM Unsignalized Intersection Capacity Analysis

## 8: Steeles Avenue & Sixth Line

Base Year - AM Peak Hour  
Premier Gateway



Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations								
Traffic Volume (veh/h)	34	728	477	2	8	33		
Future Volume (Veh/h)	34	728	477	2	8	33		
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94		
Hourly flow rate (vph)	36	774	507	2	9	35		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type		None	None					
Median storage (veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	509				966	254		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	509				966	254		
tC, single (s)	4.2				8.1	7.0		
tC, 2 stage (s)								
tF (s)	2.2				4.2	3.3		
p0 queue free %	97				94	95		
cM capacity (veh/h)	1045				156	743		
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	SB 2
Volume Total	36	387	387	254	254	2	9	35
Volume Left	36	0	0	0	0	0	9	0
Volume Right	0	0	0	0	0	2	0	35
cSH	1045	1700	1700	1700	1700	1700	156	743
Volume to Capacity	0.03	0.23	0.23	0.15	0.15	0.00	0.06	0.05
Queue Length 95th (m)	0.9	0.0	0.0	0.0	0.0	0.0	1.5	1.2
Control Delay (s)	8.6	0.0	0.0	0.0	0.0	0.0	29.5	10.1
Lane LOS	A						D	B
Approach Delay (s)	0.4			0.0			14.1	
Approach LOS							B	
Intersection Summary								
Average Delay			0.7					
Intersection Capacity Utilization			30.1%		ICU Level of Service			A
Analysis Period (min)			15					

# HCM Unsignalized Intersection Capacity Analysis

## 9: Sixth Line South & Steeles Avenue

Base Year - AM Peak Hour  
Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↑↑	↑	↓	↑↑	↓		
Traffic Volume (veh/h)	733	3	2	477	2	0	
Future Volume (Veh/h)	733	3	2	477	2	0	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Hourly flow rate (vph)	772	3	2	502	2	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage (veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume			775		1027	386	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			775		1027	386	
tC, single (s)			4.1		6.8	6.9	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		99	100	
cM capacity (veh/h)			850		233	618	
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1
Volume Total	386	386	3	2	251	251	2
Volume Left	0	0	0	2	0	0	2
Volume Right	0	0	3	0	0	0	0
cSH	1700	1700	1700	850	1700	1700	233
Volume to Capacity	0.23	0.23	0.00	0.00	0.15	0.15	0.01
Queue Length 95th (m)	0.0	0.0	0.0	0.1	0.0	0.0	0.2
Control Delay (s)	0.0	0.0	0.0	9.2	0.0	0.0	20.6
Lane LOS				A	C		
Approach Delay (s)	0.0			0.0			20.6
Approach LOS							C
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization			30.3%	ICU Level of Service		A	
Analysis Period (min)			15				

HCM Unsignalized Intersection Capacity Analysis  
 10: Steeles Avenue & Hornby Road

Base Year - AM Peak Hour  
 Premier Gateway



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↗↗	↗↗	↖	↖	↖
Traffic Volume (veh/h)	14	719	449	15	6	30
Future Volume (Veh/h)	14	719	449	15	6	30
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	15	749	468	16	6	31
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	484				872	234
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	484				872	234
tC, single (s)	4.2				7.1	7.0
tC, 2 stage (s)						
tF (s)	2.3				3.7	3.4
p0 queue free %	99				98	96
cM capacity (veh/h)	1041				259	756

Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	SB 2
Volume Total	15	374	374	234	234	16	6	31
Volume Left	15	0	0	0	0	0	6	0
Volume Right	0	0	0	0	0	16	0	31
cSH	1041	1700	1700	1700	1700	1700	259	756
Volume to Capacity	0.01	0.22	0.22	0.14	0.14	0.01	0.02	0.04
Queue Length 95th (m)	0.4	0.0	0.0	0.0	0.0	0.0	0.6	1.0
Control Delay (s)	8.5	0.0	0.0	0.0	0.0	0.0	19.2	10.0
Lane LOS	A						C	A
Approach Delay (s)	0.2			0.0			11.5	
Approach LOS							B	

Intersection Summary		
Average Delay		0.4
Intersection Capacity Utilization	29.9%	ICU Level of Service
Analysis Period (min)	15	A

HCM Unsignalized Intersection Capacity Analysis  
 11: Trafalgar Rd & Hornby Rd

Base Year - AM Peak Hour  
 Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	23	6	2	338	1195	34
Future Volume (Veh/h)	23	6	2	338	1195	34
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Hourly flow rate (vph)	23	6	2	345	1219	35
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1413	627	1219			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1413	627	1219			
tC, single (s)	7.1	7.2	5.1			
tC, 2 stage (s)						
tF (s)	3.6	3.5	2.7			
p0 queue free %	80	98	99			
cM capacity (veh/h)	116	392	362			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	29	117	230	813	441	
Volume Left	23	2	0	0	0	
Volume Right	6	0	0	0	35	
cSH	136	362	1700	1700	1700	
Volume to Capacity	0.21	0.01	0.14	0.48	0.26	
Queue Length 95th (m)	6.2	0.1	0.0	0.0	0.0	
Control Delay (s)	38.6	0.3	0.0	0.0	0.0	
Lane LOS	E	A				
Approach Delay (s)	38.6	0.1		0.0		
Approach LOS	E					
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			44.1%	ICU Level of Service		A
Analysis Period (min)			15			

Queues  
12: Trafalgar Road & Steeles Avenue

Base Year - AM Peak Hour  
Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	32	520	206	521	331	42	144	280	350	158	1092
v/c Ratio	0.14	0.78	0.50	1.30	0.36	0.08	0.55	0.24	0.44	0.28	0.90
Control Delay	23.1	54.5	11.6	196.4	34.7	0.3	61.4	28.9	5.2	17.6	49.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	23.1	54.5	11.6	196.4	34.7	0.3	61.4	28.9	5.2	17.6	49.7
Queue Length 50th (m)	5.0	67.1	3.9	-89.0	36.4	0.0	18.4	25.8	0.0	20.3	141.4
Queue Length 95th (m)	10.9	81.6	25.1	#125.0	48.2	0.0	29.6	42.2	23.3	37.0	#213.1
Internal Link Dist (m)		443.0			287.3			749.5			265.5
Turn Bay Length (m)	115.0		40.0	130.0		70.0	100.0		65.0		
Base Capacity (vph)	265	937	499	400	997	537	263	1155	791	576	1213
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.55	0.41	1.30	0.33	0.08	0.55	0.24	0.44	0.27	0.90

























Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 12: Trafalgar Road & Steeles Avenue

Base Year - AM Peak Hour

Premier Gateway

														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations														
Traffic Volume (vph)	31	499	198	500	318	40	138	269	336	152	1041	8		
Future Volume (vph)	31	499	198	500	318	40	138	269	336	152	1041	8		
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Total Lost time (s)	4.0	7.0	7.0	5.0	7.0	7.0	5.0	8.0	8.0	4.0	8.0			
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.95			
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00			
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00			
Satd. Flow (prot)	1165	3085	1214	3335	2888	1324	2556	3034	1509	1703	3356			
Flt Permitted	0.55	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.58	1.00			
Satd. Flow (perm)	677	3085	1214	3335	2888	1324	2556	3034	1509	1039	3356			
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96		
Adj. Flow (vph)	32	520	206	521	331	42	144	280	350	158	1084	8		
RTOR Reduction (vph)	0	0	144	0	0	29	0	0	221	0	1	0		
Lane Group Flow (vph)	32	520	62	521	331	13	144	280	129	158	1091	0		
Heavy Vehicles (%)	55%	17%	33%	5%	25%	22%	37%	19%	7%	6%	7%	67%		
Turn Type	pm+pt	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA			
Protected Phases	7	4		3	8		5	2		1	6			
Permitted Phases	4		4			8			2	6				
Actuated Green, G (s)	33.8	28.6	28.6	15.0	39.4	39.4	12.8	46.0	46.0	55.0	43.6			
Effective Green, g (s)	33.8	28.6	28.6	15.0	39.4	39.4	12.8	46.0	46.0	55.0	43.6			
Actuated g/C Ratio	0.27	0.23	0.23	0.12	0.32	0.32	0.10	0.37	0.37	0.44	0.35			
Clearance Time (s)	4.0	7.0	7.0	5.0	7.0	7.0	5.0	8.0	8.0	4.0	8.0			
Vehicle Extension (s)	3.0	3.0	3.0	4.0	3.0	3.0	4.0	0.2	0.2	3.0	0.2			
Lane Grp Cap (vph)	203	705	277	400	910	417	261	1116	555	517	1170			
v/s Ratio Prot	0.01	c0.17		c0.16	0.11		c0.06	0.09		0.03	c0.33			
v/s Ratio Perm	0.04		0.05			0.01			0.09	0.11				
v/c Ratio	0.16	0.74	0.22	1.30	0.36	0.03	0.55	0.25	0.23	0.31	0.93			
Uniform Delay, d1	34.2	44.7	39.2	55.0	33.1	29.6	53.4	27.5	27.3	21.6	39.3			
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Incremental Delay, d2	0.4	4.0	0.4	153.3	0.2	0.0	3.1	0.5	1.0	0.3	14.4			
Delay (s)	34.6	48.8	39.6	208.3	33.4	29.6	56.5	28.0	28.3	21.9	53.7			
Level of Service	C	D	D	F	C	C	E	C	C	C	D			
Approach Delay (s)		45.7			135.1			33.4			49.7			
Approach LOS		D			F			C			D			
<b>Intersection Summary</b>														
HCM 2000 Control Delay			66.2									HCM 2000 Level of Service	E	
HCM 2000 Volume to Capacity ratio			0.88											
Actuated Cycle Length (s)			125.0								25.0			
Intersection Capacity Utilization			87.5%										ICU Level of Service	E
Analysis Period (min)			15											
c	Critical Lane Group													

Queues  
13: Toronto Premier Outlets & Steeles Avenue

Base Year - AM Peak Hour  
Premier Gateway



Lane Group	EBT	EBR	WBT	NBL	NBR
Lane Group Flow (vph)	983	42	855	39	10
v/c Ratio	0.55	0.05	0.49	0.03	0.02
Control Delay	16.6	3.6	15.5	22.2	11.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	16.6	3.6	15.5	22.2	11.8
Queue Length 50th (m)	65.5	0.0	54.3	2.7	0.0
Queue Length 95th (m)	84.4	4.9	70.5	6.4	3.7
Internal Link Dist (m)	287.3		176.7	95.1	
Turn Bay Length (m)		130.0			40.0
Base Capacity (vph)	1772	849	1756	1133	472
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.55	0.05	0.49	0.03	0.02
<b>Intersection Summary</b>					

HCM Signalized Intersection Capacity Analysis  
13: Toronto Premier Outlets & Steeles Avenue

Base Year - AM Peak Hour  
Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↑	↑↑	↑↑	↑
Traffic Volume (vph)	944	40	0	821	37	10
Future Volume (vph)	944	40	0	821	37	10
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0		6.0	6.0	6.0
Lane Util. Factor	0.95	1.00		0.95	0.97	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		1.00	0.95	1.00
Satd. Flow (prot)	3282	1538		3252	3335	1369
Flt Permitted	1.00	1.00		1.00	0.95	1.00
Satd. Flow (perm)	3282	1538		3252	3335	1369
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	983	42	0	855	39	10
RTOR Reduction (vph)	0	19	0	0	0	7
Lane Group Flow (vph)	983	23	0	855	39	3
Heavy Vehicles (%)	10%	5%	0%	11%	5%	18%
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4		3	8	2	
Permitted Phases		4	8			2
Actuated Green, G (s)	54.0	54.0		54.0	34.0	34.0
Effective Green, g (s)	54.0	54.0		54.0	34.0	34.0
Actuated g/C Ratio	0.54	0.54		0.54	0.34	0.34
Clearance Time (s)	6.0	6.0		6.0	6.0	6.0
Vehicle Extension (s)	0.2	0.2		0.2	4.0	4.0
Lane Grp Cap (vph)	1772	830		1756	1133	465
v/s Ratio Prot	c0.30			0.26	c0.01	
v/s Ratio Perm		0.01				0.00
v/c Ratio	0.55	0.03		0.49	0.03	0.01
Uniform Delay, d1	15.1	10.7		14.4	22.0	21.8
Progression Factor	1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	1.3	0.1		1.0	0.1	0.0
Delay (s)	16.4	10.8		15.3	22.1	21.9
Level of Service	B	B		B	C	C
Approach Delay (s)	16.1			15.3	22.0	
Approach LOS	B			B	C	

Intersection Summary

HCM 2000 Control Delay	15.9	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.37		
Actuated Cycle Length (s)	100.0	Sum of lost time (s)	16.0
Intersection Capacity Utilization	44.4%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

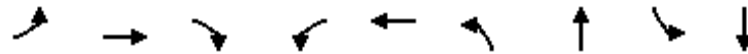


## Queues

Base Year - AM Peak Hour

## 14: Toronto Premium Outlets/Eighth Line &amp; Steeles Avenue

Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	79	922	14	33	518	3	10	188	382
v/c Ratio	0.14	0.50	0.02	0.08	0.34	0.01	0.03	0.79	0.66
Control Delay	6.8	13.2	0.0	6.8	13.8	34.3	14.3	58.9	10.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.8	13.2	0.0	6.8	13.8	34.3	14.3	58.9	10.4
Queue Length 50th (m)	3.3	30.1	0.0	1.4	22.6	0.2	0.1	27.7	0.6
Queue Length 95th (m)	13.0	94.5	0.0	6.7	51.0	1.7	3.9	#80.8	29.3
Internal Link Dist (m)		176.7			846.8		194.1		472.6
Turn Bay Length (m)	105.0		55.0	30.0				20.0	
Base Capacity (vph)	602	1861	869	497	1514	1134	1037	237	582
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.50	0.02	0.07	0.34	0.00	0.01	0.79	0.66

## Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 14: Toronto Premium Outlets/Eighth Line & Steeles Avenue

Base Year - AM Peak Hour

Premier Gateway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	74	867	13	31	464	23	3	1	8	177	5	354
Future Volume (vph)	74	867	13	31	464	23	3	1	8	177	5	354
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	6.0	6.0	4.0	6.0		7.0	7.0		7.0	7.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.97	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.99		1.00	0.86		1.00	0.85	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	3252	1417	1752	2971		2633	1495		1770	1587	
Flt Permitted	0.41	1.00	1.00	0.26	1.00		0.95	1.00		0.75	1.00	
Satd. Flow (perm)	714	3252	1417	488	2971		2633	1495		1399	1587	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	79	922	14	33	494	24	3	1	9	188	5	377
RTOR Reduction (vph)	0	0	7	0	2	0	0	7	0	0	319	0
Lane Group Flow (vph)	79	922	7	33	516	0	3	3	0	188	63	0
Heavy Vehicles (%)	8%	11%	14%	3%	21%	14%	33%	0%	11%	2%	0%	2%
Turn Type	pm+pt	NA	Perm	pm+pt	NA		Prot	NA		Perm	NA	
Protected Phases	7	4		3	8		5	2			6	
Permitted Phases	4		4	8						6		
Actuated Green, G (s)	50.4	44.5	44.5	43.8	41.2		1.6	21.8		13.2	13.2	
Effective Green, g (s)	50.4	44.5	44.5	43.8	41.2		1.6	21.8		13.2	13.2	
Actuated g/C Ratio	0.59	0.52	0.52	0.51	0.48		0.02	0.25		0.15	0.15	
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		7.0	7.0		7.0	7.0	
Vehicle Extension (s)	3.0	0.2	0.2	3.0	0.2		4.0	4.0		3.0	3.0	
Lane Grp Cap (vph)	484	1684	734	287	1424		49	379		214	243	
v/s Ratio Prot	c0.01	c0.28		0.00	0.17		c0.00	0.00			0.04	
v/s Ratio Perm	0.08		0.01	0.06						c0.13		
v/c Ratio	0.16	0.55	0.01	0.11	0.36		0.06	0.01		0.88	0.26	
Uniform Delay, d1	7.9	13.9	10.0	10.7	14.1		41.4	24.0		35.6	32.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.2	1.3	0.0	0.2	0.7		0.7	0.0		36.5	2.6	
Delay (s)	8.0	15.2	10.1	10.9	14.8		42.1	24.0		72.0	34.6	
Level of Service	A	B	B	B	B		D	C		E	C	
Approach Delay (s)		14.6			14.6			28.2			47.0	
Approach LOS		B			B			C			D	

Intersection Summary			
HCM 2000 Control Delay	23.2	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.59		
Actuated Cycle Length (s)	85.9	Sum of lost time (s)	24.0
Intersection Capacity Utilization	66.1%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Unsignalized Intersection Capacity Analysis  
 15: Eighth Line South & Steeles Avenue

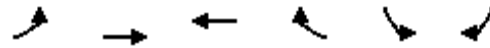
Base Year - AM Peak Hour  
 Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↑↑		↙	↑↑	↙	↗	
Traffic Volume (veh/h)	1051	1	1	517	1	0	
Future Volume (Veh/h)	1051	1	1	517	1	0	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	
Hourly flow rate (vph)	1095	1	1	539	1	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None		None				
Median storage (veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume			1096		1367	548	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			1096		1367	548	
tC, single (s)			4.1		6.8	6.9	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		99	100	
cM capacity (veh/h)			644		140	485	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1	NB 2
Volume Total	730	366	1	270	270	1	0
Volume Left	0	0	1	0	0	1	0
Volume Right	0	1	0	0	0	0	0
cSH	1700	1700	644	1700	1700	140	1700
Volume to Capacity	0.43	0.22	0.00	0.16	0.16	0.01	0.00
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.2	0.0
Control Delay (s)	0.0	0.0	10.6	0.0	0.0	30.8	0.0
Lane LOS			B			D	A
Approach Delay (s)	0.0		0.0			30.8	
Approach LOS						D	
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization			39.1%	ICU Level of Service		A	
Analysis Period (min)			15				

Queues  
16: Steeles Avenue & Ninth Line

Base Year - AM Peak Hour  
Premier Gateway



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	79	1027	468	224	598	77
v/c Ratio	0.25	0.80	0.51	0.37	0.72	0.12
Control Delay	19.9	32.6	32.0	5.7	27.3	4.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	19.9	32.6	32.0	5.7	27.3	4.0
Queue Length 50th (m)	9.6	95.7	42.7	0.0	94.4	0.0
Queue Length 95th (m)	19.2	122.5	59.4	17.6	137.7	7.8
Internal Link Dist (m)		501.4	674.5		3096.2	
Turn Bay Length (m)	65.0			75.0	90.0	
Base Capacity (vph)	319	1291	916	611	831	663
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.80	0.51	0.37	0.72	0.12

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 16: Steeles Avenue & Ninth Line

Base Year - AM Peak Hour  
 Premier Gateway



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	75	976	445	213	568	73
Future Volume (vph)	75	976	445	213	568	73
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	7.0	7.0	7.0	7.0	7.0
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Frt	1.00	1.00	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1556	3312	3034	1509	1770	1324
Flt Permitted	0.37	1.00	1.00	1.00	0.95	1.00
Satd. Flow (perm)	602	3312	3034	1509	1770	1324
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	79	1027	468	224	598	77
RTOR Reduction (vph)	0	0	0	157	0	41
Lane Group Flow (vph)	79	1027	468	67	598	36
Heavy Vehicles (%)	16%	9%	19%	7%	2%	22%
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	7	4	8		6	
Permitted Phases	4			8		6
Actuated Green, G (s)	39.8	39.8	30.2	30.2	47.0	47.0
Effective Green, g (s)	39.8	39.8	30.2	30.2	47.0	47.0
Actuated g/C Ratio	0.39	0.39	0.30	0.30	0.47	0.47
Clearance Time (s)	4.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	0.2	0.2	0.2	3.0	3.0
Lane Grp Cap (vph)	290	1307	908	452	825	617
v/s Ratio Prot	0.02	c0.31	0.15		c0.34	
v/s Ratio Perm	0.09			0.04		0.03
v/c Ratio	0.27	0.79	0.52	0.15	0.72	0.06
Uniform Delay, d1	19.8	26.8	29.2	25.9	21.7	14.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	4.8	2.1	0.7	5.5	0.2
Delay (s)	20.3	31.6	31.3	26.6	27.2	14.9
Level of Service	C	C	C	C	C	B
Approach Delay (s)		30.8	29.8		25.8	
Approach LOS		C	C		C	

Intersection Summary

HCM 2000 Control Delay	29.1	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.79		
Actuated Cycle Length (s)	100.8	Sum of lost time (s)	18.0
Intersection Capacity Utilization	70.1%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
17: Ninth Line (South) & Steeles Avenue

Base Year - AM Peak Hour  
Premier Gateway



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1188	454	331	533	167	301
v/c Ratio	0.86	0.49	0.83	0.28	0.46	0.53
Control Delay	31.0	3.7	38.4	7.2	39.4	8.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.0	3.7	38.4	7.2	39.4	8.2
Queue Length 50th (m)	102.0	0.0	40.6	19.6	29.0	0.0
Queue Length 95th (m)	137.5	17.6	#77.1	26.9	53.0	23.0
Internal Link Dist (m)	674.5			410.9	143.5	
Turn Bay Length (m)		75.0	145.0		60.0	
Base Capacity (vph)	1649	1016	485	2334	360	568
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.72	0.45	0.68	0.23	0.46	0.53

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 17: Ninth Line (South) & Steeles Avenue

Base Year - AM Peak Hour  
 Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑
Traffic Volume (vph)	1117	427	311	501	157	283
Future Volume (vph)	1117	427	311	501	157	283
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.0	7.0	4.0	7.0	7.0	7.0
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3312	1583	1787	3034	1770	1615
Flt Permitted	1.00	1.00	0.10	1.00	0.95	1.00
Satd. Flow (perm)	3312	1583	183	3034	1770	1615
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	1188	454	331	533	167	301
RTOR Reduction (vph)	0	263	0	0	0	240
Lane Group Flow (vph)	1188	191	331	533	167	61
Heavy Vehicles (%)	9%	2%	1%	19%	2%	0%
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4		3	8	2	
Permitted Phases		4	8			2
Actuated Green, G (s)	37.7	37.7	57.3	57.3	18.3	18.3
Effective Green, g (s)	37.7	37.7	57.3	57.3	18.3	18.3
Actuated g/C Ratio	0.42	0.42	0.64	0.64	0.20	0.20
Clearance Time (s)	7.0	7.0	4.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1393	666	396	1940	361	329
v/s Ratio Prot	0.36		c0.15	0.18	c0.09	
v/s Ratio Perm		0.12	c0.39			0.04
v/c Ratio	0.85	0.29	0.84	0.27	0.46	0.19
Uniform Delay, d1	23.4	17.1	23.3	7.1	31.3	29.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	5.3	0.2	14.1	0.1	4.2	1.3
Delay (s)	28.7	17.3	37.5	7.1	35.5	30.7
Level of Service	C	B	D	A	D	C
Approach Delay (s)	25.6			18.8	32.5	
Approach LOS	C			B	C	

Intersection Summary			
HCM 2000 Control Delay	24.7	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.77		
Actuated Cycle Length (s)	89.6	Sum of lost time (s)	18.0
Intersection Capacity Utilization	71.8%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

## Queues

Base Year - AM Peak Hour

## 18: James Snow Parkway &amp; Hwy 401 (Westbound Ramp)

Premier Gateway



Lane Group	WBL	WBR	NBT	SBT
Lane Group Flow (vph)	504	235	562	521
v/c Ratio	0.61	0.58	0.38	0.33
Control Delay	18.8	15.6	11.6	11.1
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	18.8	15.6	11.6	11.1
Queue Length 50th (m)	20.0	10.8	18.8	17.0
Queue Length 95th (m)	32.6	30.6	35.4	32.0
Internal Link Dist (m)	390.4		415.8	504.8
Turn Bay Length (m)				
Base Capacity (vph)	1893	810	1460	1557
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.27	0.29	0.38	0.33

## Intersection Summary



HCM Signalized Intersection Capacity Analysis  
 18: James Snow Parkway & Hwy 401 (Westbound Ramp)

Base Year - AM Peak Hour

Premier Gateway



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔↔	↗	↕↕			↕↕
Traffic Volume (vph)	348	354	534	0	0	495
Future Volume (vph)	348	354	534	0	0	495
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.2	8.2	9.3			9.3
Lane Util. Factor	0.97	0.91	0.95			0.95
Frt	0.96	0.85	1.00			1.00
Flt Protected	0.96	1.00	1.00			1.00
Satd. Flow (prot)	3274	1336	3195			3406
Flt Permitted	0.96	1.00	1.00			1.00
Satd. Flow (perm)	3274	1336	3195			3406
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	366	373	562	0	0	521
RTOR Reduction (vph)	72	96	0	0	0	0
Lane Group Flow (vph)	432	139	562	0	0	521
Heavy Vehicles (%)	2%	10%	13%	0%	0%	6%
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Actuated Green, G (s)	13.0	13.0	25.8			25.8
Effective Green, g (s)	13.0	13.0	25.8			25.8
Actuated g/C Ratio	0.23	0.23	0.46			0.46
Clearance Time (s)	8.2	8.2	9.3			9.3
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	755	308	1464			1560
v/s Ratio Prot	c0.13		c0.18			0.15
v/s Ratio Perm		0.10				
v/c Ratio	0.57	0.45	0.38			0.33
Uniform Delay, d1	19.2	18.6	10.0			9.8
Progression Factor	1.00	1.00	1.00			1.00
Incremental Delay, d2	1.1	1.1	0.8			0.6
Delay (s)	20.2	19.6	10.8			10.3
Level of Service	C	B	B			B
Approach Delay (s)	20.0		10.8			10.3
Approach LOS	C		B			B

Intersection Summary			
HCM 2000 Control Delay	14.4	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.45		
Actuated Cycle Length (s)	56.3	Sum of lost time (s)	17.5
Intersection Capacity Utilization	45.9%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

Queues

Base Year - AM Peak Hour

19: James Snow Parkway & Hwy 401 (Eastbound Ramp)

Premier Gateway



Lane Group	EBL	EBR	NBT	SBT
Lane Group Flow (vph)	283	134	622	672
v/c Ratio	0.49	0.39	0.30	0.31
Control Delay	16.7	8.7	6.3	6.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	16.7	8.7	6.3	6.4
Queue Length 50th (m)	9.2	0.0	15.5	16.9
Queue Length 95th (m)	19.1	13.8	26.1	28.2
Internal Link Dist (m)	305.5		1282.4	415.8
Turn Bay Length (m)				
Base Capacity (vph)	1163	604	2067	2148
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.24	0.22	0.30	0.31

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 19: James Snow Parkway & Hwy 401 (Eastbound Ramp)

Base Year - AM Peak Hour

Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	166	243	0	610	659	0
Future Volume (vph)	166	243	0	610	659	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0		7.4	7.4	
Lane Util. Factor	0.97	0.91		0.95	0.95	
Frt	0.94	0.85		1.00	1.00	
Flt Protected	0.97	1.00		1.00	1.00	
Satd. Flow (prot)	2822	1348		3374	3505	
Flt Permitted	0.97	1.00		1.00	1.00	
Satd. Flow (perm)	2822	1348		3374	3505	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	169	248	0	622	672	0
RTOR Reduction (vph)	95	111	0	0	0	0
Lane Group Flow (vph)	188	23	0	622	672	0
Heavy Vehicles (%)	26%	9%	0%	7%	3%	0%
Turn Type	Prot	Perm		NA	NA	
Protected Phases	4			2	6	
Permitted Phases		4				
Actuated Green, G (s)	10.6	10.6		38.0	38.0	
Effective Green, g (s)	10.6	10.6		38.0	38.0	
Actuated g/C Ratio	0.17	0.17		0.61	0.61	
Clearance Time (s)	6.0	6.0		7.4	7.4	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	482	230		2067	2148	
v/s Ratio Prot	c0.07			0.18	c0.19	
v/s Ratio Perm		0.02				
v/c Ratio	0.39	0.10		0.30	0.31	
Uniform Delay, d1	22.8	21.7		5.7	5.7	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.5	0.2		0.4	0.4	
Delay (s)	23.4	21.9		6.1	6.1	
Level of Service	C	C		A	A	
Approach Delay (s)	22.9			6.1	6.1	
Approach LOS	C			A	A	

Intersection Summary			
HCM 2000 Control Delay	10.2	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.33		
Actuated Cycle Length (s)	62.0	Sum of lost time (s)	13.4
Intersection Capacity Utilization	45.9%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
 20: Trafalgar Road & Hwy 401 (Westbound Ramp)

Base Year - AM Peak Hour  
 Premier Gateway



Lane Group	WBL	WBR	NBT	SBT
Lane Group Flow (vph)	261	145	468	1686
v/c Ratio	0.70	0.61	0.18	0.64
Control Delay	31.2	18.9	3.3	6.7
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	31.2	18.9	3.3	6.7
Queue Length 50th (m)	13.0	0.0	10.7	67.5
Queue Length 95th (m)	26.9	22.0	19.7	111.8
Internal Link Dist (m)	383.1		312.7	749.5
Turn Bay Length (m)				
Base Capacity (vph)	553	313	2572	2644
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.47	0.46	0.18	0.64
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
 20: Trafalgar Road & Hwy 401 (Westbound Ramp)

Base Year - AM Peak Hour  
 Premier Gateway



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔↔	↗	↕↕			↕↕
Traffic Volume (vph)	112	282	454	0	0	1635
Future Volume (vph)	112	282	454	0	0	1635
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	6.0			6.0
Lane Util. Factor	0.97	0.91	0.95			0.95
Frt	0.92	0.85	1.00			1.00
Flt Protected	0.98	1.00	1.00			1.00
Satd. Flow (prot)	2414	1081	3252			3343
Flt Permitted	0.98	1.00	1.00			1.00
Satd. Flow (perm)	2414	1081	3252			3343
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	115	291	468	0	0	1686
RTOR Reduction (vph)	131	130	0	0	0	0
Lane Group Flow (vph)	130	15	468	0	0	1686
Heavy Vehicles (%)	38%	36%	11%	0%	0%	8%
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Actuated Green, G (s)	11.3	11.3	88.1			88.1
Effective Green, g (s)	11.3	11.3	88.1			88.1
Actuated g/C Ratio	0.10	0.10	0.79			0.79
Clearance Time (s)	6.0	6.0	6.0			6.0
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	244	109	2571			2643
v/s Ratio Prot	c0.05		0.14			c0.50
v/s Ratio Perm		0.01				
v/c Ratio	0.53	0.13	0.18			0.64
Uniform Delay, d1	47.5	45.6	2.8			4.9
Progression Factor	1.00	1.00	1.00			1.00
Incremental Delay, d2	2.2	0.6	0.2			1.2
Delay (s)	49.8	46.2	3.0			6.1
Level of Service	D	D	A			A
Approach Delay (s)	48.5		3.0			6.1
Approach LOS	D		A			A

Intersection Summary			
HCM 2000 Control Delay	12.3	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.63		
Actuated Cycle Length (s)	111.4	Sum of lost time (s)	12.0
Intersection Capacity Utilization	61.4%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
 21: Trafalgar Road & Hwy 401 (Eastbound Ramp)

Base Year - AM Peak Hour  
 Premier Gateway



Lane Group	EBL	EBR	NBT	SBT
Lane Group Flow (vph)	141	94	1046	1287
v/c Ratio	0.47	0.50	0.38	0.48
Control Delay	23.3	19.3	3.2	3.8
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	23.3	19.3	3.2	3.8
Queue Length 50th (m)	5.1	0.0	23.3	32.5
Queue Length 95th (m)	15.0	17.5	38.2	53.0
Internal Link Dist (m)	204.3		1138.2	312.7
Turn Bay Length (m)				
Base Capacity (vph)	582	313	2764	2689
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.24	0.30	0.38	0.48
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
 21: Trafalgar Road & Hwy 401 (Eastbound Ramp)

Base Year - AM Peak Hour

Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	45	185	0	1025	1261	0
Future Volume (vph)	45	185	0	1025	1261	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0		6.0	6.0	
Lane Util. Factor	0.97	0.91		0.95	0.95	
Frt	0.90	0.85		1.00	1.00	
Flt Protected	0.98	1.00		1.00	1.00	
Satd. Flow (prot)	2957	1386		3374	3282	
Flt Permitted	0.98	1.00		1.00	1.00	
Satd. Flow (perm)	2957	1386		3374	3282	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	46	189	0	1046	1287	0
RTOR Reduction (vph)	88	87	0	0	0	0
Lane Group Flow (vph)	53	7	0	1046	1287	0
Heavy Vehicles (%)	19%	6%	0%	7%	10%	0%
Turn Type	Prot	Perm		NA	NA	
Protected Phases	4			2	6	
Permitted Phases		4				
Actuated Green, G (s)	8.2	8.2		91.6	91.6	
Effective Green, g (s)	8.2	8.2		91.6	91.6	
Actuated g/C Ratio	0.07	0.07		0.82	0.82	
Clearance Time (s)	6.0	6.0		6.0	6.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	216	101		2764	2689	
v/s Ratio Prot	c0.02			0.31	c0.39	
v/s Ratio Perm		0.00				
v/c Ratio	0.25	0.07		0.38	0.48	
Uniform Delay, d1	48.9	48.2		2.6	3.0	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.6	0.3		0.4	0.6	
Delay (s)	49.5	48.5		3.0	3.6	
Level of Service	D	D		A	A	
Approach Delay (s)	49.1			3.0	3.6	
Approach LOS	D			A	A	

Intersection Summary			
HCM 2000 Control Delay	7.5	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.46		
Actuated Cycle Length (s)	111.8	Sum of lost time (s)	12.0
Intersection Capacity Utilization	61.4%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

Queues

Base Year - AM Peak Hour

22: Winston Churchill Boulevard & Hwy 401 (Westbound Ramp)

Premier Gateway



Lane Group	WBL	WBR	NBT	SBT
Lane Group Flow (vph)	444	206	1251	1048
v/c Ratio	0.78	0.72	0.50	0.42
Control Delay	51.7	42.4	9.1	8.1
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	51.7	42.4	9.1	8.1
Queue Length 50th (m)	48.0	32.1	67.8	51.8
Queue Length 95th (m)	66.8	63.5	104.5	81.2
Internal Link Dist (m)	284.7		32.1	320.2
Turn Bay Length (m)				
Base Capacity (vph)	771	370	2518	2518
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.58	0.56	0.50	0.42

Intersection Summary



HCM Signalized Intersection Capacity Analysis  
 22: Winston Churchill Boulevard & Hwy 401 (Westbound Ramp)

Base Year - AM Peak Hour  
 Premier Gateway



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↙↘	↗	↕↕			↕↕
Traffic Volume (vph)	248	370	1188	0	0	996
Future Volume (vph)	248	370	1188	0	0	996
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.0	7.0	8.0			8.0
Lane Util. Factor	0.97	0.91	0.95			0.95
Frt	0.94	0.85	1.00			1.00
Flt Protected	0.97	1.00	1.00			1.00
Satd. Flow (prot)	3052	1301	3471			3471
Flt Permitted	0.97	1.00	1.00			1.00
Satd. Flow (perm)	3052	1301	3471			3471
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	261	389	1251	0	0	1048
RTOR Reduction (vph)	79	79	0	0	0	0
Lane Group Flow (vph)	365	127	1251	0	0	1048
Heavy Vehicles (%)	8%	13%	4%	0%	0%	4%
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Actuated Green, G (s)	21.0	21.0	95.1			95.1
Effective Green, g (s)	21.0	21.0	95.1			95.1
Actuated g/C Ratio	0.16	0.16	0.73			0.73
Clearance Time (s)	7.0	7.0	8.0			8.0
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	488	208	2517			2517
v/s Ratio Prot	c0.12		c0.36			0.30
v/s Ratio Perm		0.10				
v/c Ratio	0.75	0.61	0.50			0.42
Uniform Delay, d1	52.5	51.2	7.7			7.1
Progression Factor	1.00	1.00	1.00			1.00
Incremental Delay, d2	6.2	5.2	0.7			0.5
Delay (s)	58.7	56.5	8.4			7.6
Level of Service	E	E	A			A
Approach Delay (s)	58.0		8.4			7.6
Approach LOS	E		A			A

**Intersection Summary**

HCM 2000 Control Delay	19.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.54		
Actuated Cycle Length (s)	131.1	Sum of lost time (s)	15.0
Intersection Capacity Utilization	179.5%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

Queues

Base Year - AM Peak Hour

23: Winston Churchill Boulevard & Hwy 401 (Eastbound Ramp)

Premier Gateway



Lane Group	EBL	EBR	NBT	SBT
Lane Group Flow (vph)	580	263	1034	1047
v/c Ratio	0.81	0.66	0.44	0.45
Control Delay	56.5	33.7	11.3	11.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	56.5	33.7	11.3	11.4
Queue Length 50th (m)	75.0	39.8	63.6	65.2
Queue Length 95th (m)	95.9	73.5	92.7	94.7
Internal Link Dist (m)	152.5		433.2	198.3
Turn Bay Length (m)				
Base Capacity (vph)	977	500	2351	2328
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.59	0.53	0.44	0.45

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 23: Winston Churchill Boulevar & Hwy 401 (Eastbound Ramp)

Base Year - AM Peak Hour  
 Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	←←	→		↑↑	↑↑	
Traffic Volume (vph)	459	342	0	982	995	0
Future Volume (vph)	459	342	0	982	995	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.0	8.0		7.0	7.0	
Lane Util. Factor	0.97	0.91		0.95	0.95	
Frt	0.97	0.85		1.00	1.00	
Flt Protected	0.96	1.00		1.00	1.00	
Satd. Flow (prot)	3296	1427		3505	3471	
Flt Permitted	0.96	1.00		1.00	1.00	
Satd. Flow (perm)	3296	1427		3505	3471	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	483	360	0	1034	1047	0
RTOR Reduction (vph)	13	92	0	0	0	0
Lane Group Flow (vph)	567	171	0	1034	1047	0
Heavy Vehicles (%)	5%	3%	0%	3%	4%	0%
Turn Type	Prot	Perm		NA	NA	
Protected Phases	4			2	6	
Permitted Phases		4				
Actuated Green, G (s)	27.7	27.7		87.2	87.2	
Effective Green, g (s)	27.7	27.7		87.2	87.2	
Actuated g/C Ratio	0.21	0.21		0.67	0.67	
Clearance Time (s)	8.0	8.0		7.0	7.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	702	304		2352	2330	
v/s Ratio Prot	c0.17			0.30	c0.30	
v/s Ratio Perm		0.12				
v/c Ratio	0.81	0.56		0.44	0.45	
Uniform Delay, d1	48.6	45.7		10.0	10.0	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	6.8	2.4		0.6	0.6	
Delay (s)	55.3	48.1		10.6	10.7	
Level of Service	E	D		B	B	
Approach Delay (s)	53.1			10.6	10.7	
Approach LOS	D			B	B	

Intersection Summary

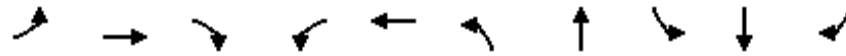
HCM 2000 Control Delay	22.9	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.54		
Actuated Cycle Length (s)	129.9	Sum of lost time (s)	15.0
Intersection Capacity Utilization	179.5%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

## Queues

Base Year - AM Peak Hour

## 24: James Snow Parkway &amp; Main Street East

Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	793	226	427	78	96	209	883	126	579	245
v/c Ratio	0.87	0.28	0.52	0.51	0.22	0.60	0.83	0.62	0.66	0.43
Control Delay	41.0	15.3	8.7	45.0	18.4	25.4	35.6	33.6	32.9	6.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	41.0	15.3	8.7	45.0	18.4	25.4	35.6	33.6	32.9	6.6
Queue Length 50th (m)	64.7	22.5	17.8	12.3	3.5	22.2	71.9	12.7	46.5	0.0
Queue Length 95th (m)	#107.2	36.9	40.4	26.1	10.3	#44.9	#119.0	#32.6	70.5	18.7
Internal Link Dist (m)		274.7			467.9		430.6		1282.4	
Turn Bay Length (m)	70.0		50.0	105.0		100.0		135.0		135.0
Base Capacity (vph)	967	1080	1016	261	719	352	1065	204	873	576
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.21	0.42	0.30	0.13	0.59	0.83	0.62	0.66	0.43

## Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 24: James Snow Parkway & Main Street East

Base Year - AM Peak Hour  
 Premier Gateway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	753	215	406	74	43	48	199	730	109	120	550	233
Future Volume (vph)	753	215	406	74	43	48	199	730	109	120	550	233
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	6.0	6.0	6.0	6.0		4.5	6.0		4.5	6.0	6.0
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95		1.00	0.95		1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	0.92		1.00	0.98		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	3433	1900	1599	1752	2964		1752	3470		1687	3374	1524
Flt Permitted	0.95	1.00	1.00	0.62	1.00		0.26	1.00		0.19	1.00	1.00
Satd. Flow (perm)	3433	1900	1599	1139	2964		477	3470		343	3374	1524
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	793	226	427	78	45	51	209	768	115	126	579	245
RTOR Reduction (vph)	0	0	140	0	45	0	0	13	0	0	0	182
Lane Group Flow (vph)	793	226	287	78	51	0	209	870	0	126	579	63
Heavy Vehicles (%)	2%	0%	1%	3%	2%	21%	3%	2%	2%	7%	7%	6%
Turn Type	Prot	NA	Perm	Perm	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases	7	4			8		5	2		1	6	
Permitted Phases			4	8			2			6		6
Actuated Green, G (s)	21.2	35.0	35.0	9.3	9.3		32.9	24.2		25.9	20.7	20.7
Effective Green, g (s)	21.2	35.0	35.0	9.3	9.3		32.9	24.2		25.9	20.7	20.7
Actuated g/C Ratio	0.26	0.43	0.43	0.11	0.11		0.41	0.30		0.32	0.26	0.26
Clearance Time (s)	4.5	6.0	6.0	6.0	6.0		4.5	6.0		4.5	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	899	822	691	130	340		331	1037		196	863	389
v/s Ratio Prot	c0.23	0.12			0.02		c0.07	c0.25		0.04	0.17	
v/s Ratio Perm			0.18	c0.07			0.19			0.16		0.04
v/c Ratio	0.88	0.27	0.42	0.60	0.15		0.63	0.84		0.64	0.67	0.16
Uniform Delay, d1	28.7	14.8	15.9	34.0	32.2		17.0	26.5		21.0	27.0	23.4
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	10.2	0.2	0.4	7.3	0.2		3.9	8.1		7.0	4.1	0.9
Delay (s)	38.8	15.0	16.3	41.3	32.4		20.9	34.7		28.0	31.2	24.2
Level of Service	D	B	B	D	C		C	C		C	C	C
Approach Delay (s)		28.4			36.4			32.0			29.0	
Approach LOS		C			D			C			C	

Intersection Summary

HCM 2000 Control Delay	30.0	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.82		
Actuated Cycle Length (s)	80.9	Sum of lost time (s)	21.0
Intersection Capacity Utilization	73.4%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

HCM Unsignalized Intersection Capacity Analysis  
1: Fifth Line & 5 Side Road

Base Year - PM Peak Hour  
Premier Gateway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	34	191	4	19	455	24	6	38	21	1	26	12
Future Volume (Veh/h)	34	191	4	19	455	24	6	38	21	1	26	12
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	35	199	4	20	474	25	6	40	22	1	27	13
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	499			203			824	810	201	840	800	486
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	499			203			824	810	201	840	800	486
tC, single (s)	4.1			4.1			7.2	6.5	6.2	7.1	6.5	6.4
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.6	4.0	3.3	3.5	4.0	3.5
p0 queue free %	97			99			98	87	97	100	91	98
cM capacity (veh/h)	1075			1381			246	301	835	243	306	546
<b>Direction, Lane #</b>	<b>EB 1</b>	<b>WB 1</b>	<b>NB 1</b>	<b>SB 1</b>								
Volume Total	238	519	68	41								
Volume Left	35	20	6	1								
Volume Right	4	25	22	13								
cSH	1075	1381	371	353								
Volume to Capacity	0.03	0.01	0.18	0.12								
Queue Length 95th (m)	0.8	0.4	5.3	3.1								
Control Delay (s)	1.5	0.4	16.9	16.5								
Lane LOS	A	A	C	C								
Approach Delay (s)	1.5	0.4	16.9	16.5								
Approach LOS			C	C								
<b>Intersection Summary</b>												
Average Delay			2.8									
Intersection Capacity Utilization			41.4%		ICU Level of Service				A			
Analysis Period (min)			15									

# HCM Unsignalized Intersection Capacity Analysis

## 2: Sixth Line & 5 Side Road

Base Year - PM Peak Hour  
Premier Gateway

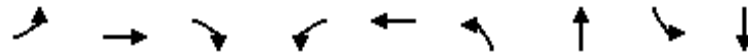


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	3	205	2	16	506	21	7	26	14	6	13	6
Future Volume (Veh/h)	3	205	2	16	506	21	7	26	14	6	13	6
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	3	214	2	17	527	22	7	27	15	6	14	6
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	549			216			806	804	215	822	794	538
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	549			216			806	804	215	822	794	538
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.3
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.4
p0 queue free %	100			99			98	91	98	98	96	99
cM capacity (veh/h)	1031			1366			286	314	830	268	318	522
<b>Direction, Lane #</b>	<b>EB 1</b>	<b>WB 1</b>	<b>NB 1</b>	<b>SB 1</b>								
Volume Total	219	566	49	26								
Volume Left	3	17	7	6								
Volume Right	2	22	15	6								
cSH	1031	1366	381	334								
Volume to Capacity	0.00	0.01	0.13	0.08								
Queue Length 95th (m)	0.1	0.3	3.5	2.0								
Control Delay (s)	0.1	0.4	15.8	16.7								
Lane LOS	A	A	C	C								
Approach Delay (s)	0.1	0.4	15.8	16.7								
Approach LOS			C	C								
<b>Intersection Summary</b>												
Average Delay			1.7									
Intersection Capacity Utilization			47.1%		ICU Level of Service				A			
Analysis Period (min)			15									

Queues  
3: Trafalgar Rd & 5 Side Road

Base Year - PM Peak Hour

Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	48	147	36	40	501	88	890	7	470
v/c Ratio	0.68	0.28	0.08	0.12	0.93	0.17	0.46	0.02	0.28
Control Delay	78.0	29.7	1.1	28.1	60.7	8.8	13.9	7.9	15.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	78.0	29.7	1.1	28.1	60.7	8.8	13.9	7.9	15.3
Queue Length 50th (m)	8.8	23.6	0.0	6.1	100.0	6.8	48.7	0.5	29.2
Queue Length 95th (m)	#29.5	40.5	1.5	14.7	#164.9	13.2	82.5	2.2	40.3
Internal Link Dist (m)		593.5			641.2		240.1		238.0
Turn Bay Length (m)	40.0		40.0	40.0		40.0		50.0	
Base Capacity (vph)	73	544	487	355	551	520	1933	360	1691
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.66	0.27	0.07	0.11	0.91	0.17	0.46	0.02	0.28

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.



# HCM Signalized Intersection Capacity Analysis

## 3: Trafalgar Rd & 5 Side Road

Base Year - PM Peak Hour

Premier Gateway



















Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	144	35	39	456	35	86	821	51	7	455	6
Future Volume (vph)	47	144	35	39	456	35	86	821	51	7	455	6
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.4	6.4	6.4	6.4	6.4		4.0	6.0		4.0	6.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	1.00	0.85	1.00	0.99		1.00	0.99		1.00	1.00	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1671	1863	1482	1752	1880		1671	3408		1626	3376	
Flt Permitted	0.14	1.00	1.00	0.66	1.00		0.43	1.00		0.28	1.00	
Satd. Flow (perm)	251	1863	1482	1218	1880		758	3408		482	3376	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	48	147	36	40	465	36	88	838	52	7	464	6
RTOR Reduction (vph)	0	0	26	0	3	0	0	4	0	0	1	0
Lane Group Flow (vph)	48	147	10	40	498	0	88	886	0	7	469	0
Heavy Vehicles (%)	8%	2%	9%	3%	0%	0%	8%	5%	5%	11%	6%	63%
Turn Type	Perm	NA	Perm	Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8			2			6		
Actuated Green, G (s)	28.0	28.0	28.0	28.0	28.0		61.0	55.6		53.0	51.6	
Effective Green, g (s)	28.0	28.0	28.0	28.0	28.0		61.0	55.6		53.0	51.6	
Actuated g/C Ratio	0.28	0.28	0.28	0.28	0.28		0.60	0.55		0.52	0.51	
Clearance Time (s)	6.4	6.4	6.4	6.4	6.4		4.0	6.0		4.0	6.0	
Vehicle Extension (s)	5.0	5.0	5.0	5.0	5.0		3.0	5.0		3.0	5.0	
Lane Grp Cap (vph)	69	514	409	336	519		504	1868		267	1717	
v/s Ratio Prot		0.08			c0.27		c0.01	c0.26		0.00	0.14	
v/s Ratio Perm	0.19		0.01	0.03			0.10			0.01		
v/c Ratio	0.70	0.29	0.02	0.12	0.96		0.17	0.47		0.03	0.27	
Uniform Delay, d1	32.9	28.8	26.7	27.5	36.1		8.7	14.0		11.8	14.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	32.2	0.6	0.1	0.3	29.7		0.2	0.9		0.0	0.4	
Delay (s)	65.1	29.5	26.8	27.8	65.9		8.9	14.8		11.8	14.6	
Level of Service	E	C	C	C	E		A	B		B	B	
Approach Delay (s)		36.5			63.1			14.3			14.6	
Approach LOS		D			E			B			B	

### Intersection Summary

HCM 2000 Control Delay	28.5	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.63		
Actuated Cycle Length (s)	101.4	Sum of lost time (s)	16.4
Intersection Capacity Utilization	82.9%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			

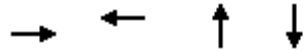
HCM Unsignalized Intersection Capacity Analysis  
4: Eighth Line & 5 Side Road

Base Year - PM Peak Hour  
Premier Gateway

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	47	144	35	18	489	70	4	325	40	18	99	38
Future Volume (vph)	47	144	35	18	489	70	4	325	40	18	99	38
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	49	150	36	19	509	73	4	339	42	19	103	40
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	235	601	385	162								
Volume Left (vph)	49	19	4	19								
Volume Right (vph)	36	73	42	40								
Hadj (s)	0.03	-0.04	0.01	-0.08								
Departure Headway (s)	7.3	6.5	7.0	7.7								
Degree Utilization, x	0.48	1.09	0.75	0.34								
Capacity (veh/h)	462	541	501	425								
Control Delay (s)	16.8	90.9	28.0	14.6								
Approach Delay (s)	16.8	90.9	28.0	14.6								
Approach LOS	C	F	D	B								
Intersection Summary												
Delay			51.9									
Level of Service			F									
Intersection Capacity Utilization			60.1%	ICU Level of Service	B							
Analysis Period (min)			15									

Queues  
5: Ninth Line & 5 Side Road

Base Year - PM Peak Hour  
Premier Gateway



Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	259	803	816	375
v/c Ratio	0.61	1.19	0.89	0.45
Control Delay	27.9	126.9	33.1	15.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	27.9	126.9	33.1	15.3
Queue Length 50th (m)	33.0	~155.5	112.3	36.5
Queue Length 95th (m)	58.8	#225.2	#190.3	58.8
Internal Link Dist (m)	556.9	434.3	3096.2	305.9
Turn Bay Length (m)				
Base Capacity (vph)	428	673	914	841
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.61	1.19	0.89	0.45

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 5: Ninth Line & 5 Side Road

Base Year - PM Peak Hour  
Premier Gateway



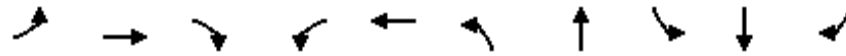
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	44	198	5	6	534	223	17	750	9	27	305	25
Future Volume (vph)	44	198	5	6	534	223	17	750	9	27	305	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0			6.0			6.0			6.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		1.00			0.96			1.00			0.99	
Flt Protected		0.99			1.00			1.00			1.00	
Satd. Flow (prot)		1814			1812			1895			1859	
Flt Permitted		0.64			1.00			0.99			0.92	
Satd. Flow (perm)		1180			1808			1875			1717	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	46	208	5	6	562	235	18	789	9	28	321	26
RTOR Reduction (vph)	0	1	0	0	18	0	0	1	0	0	4	0
Lane Group Flow (vph)	0	258	0	0	785	0	0	815	0	0	371	0
Heavy Vehicles (%)	2%	4%	0%	0%	1%	0%	0%	0%	0%	0%	1%	0%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		29.0			29.0			39.0			39.0	
Effective Green, g (s)		29.0			29.0			39.0			39.0	
Actuated g/C Ratio		0.36			0.36			0.49			0.49	
Clearance Time (s)		6.0			6.0			6.0			6.0	
Vehicle Extension (s)		3.5			3.5			5.5			5.5	
Lane Grp Cap (vph)		427			655			914			837	
v/s Ratio Prot												
v/s Ratio Perm		0.22			0.43			0.44			0.22	
v/c Ratio		0.61			1.20			0.89			0.44	
Uniform Delay, d1		20.8			25.5			18.6			13.4	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		2.5			103.3			12.9			1.7	
Delay (s)		23.4			128.8			31.5			15.1	
Level of Service		C			F			C			B	
Approach Delay (s)		23.4			128.8			31.5			15.1	
Approach LOS		C			F			C			B	

### Intersection Summary

HCM 2000 Control Delay	62.5	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	1.02		
Actuated Cycle Length (s)	80.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	96.8%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
6: Brownridge Road/Fifth Line & Steeles Avenue

Base Year - PM Peak Hour  
Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	49	718	3	2	1056	45	43	68	3	105
v/c Ratio	0.15	0.32	0.00	0.01	0.44	0.23	0.18	0.38	0.01	0.33
Control Delay	6.5	5.5	0.0	5.0	6.3	30.8	13.6	35.3	26.7	9.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.5	5.5	0.0	5.0	6.3	30.8	13.6	35.3	26.7	9.4
Queue Length 50th (m)	2.2	19.6	0.0	0.1	32.5	5.9	0.9	9.1	0.4	0.0
Queue Length 95th (m)	7.5	33.4	0.0	0.8	53.7	14.9	9.2	20.9	2.6	12.5
Internal Link Dist (m)		462.3			679.6		261.2		67.4	
Turn Bay Length (m)	145.0		65.0	30.0		20.0		25.0		25.0
Base Capacity (vph)	336	2274	955	395	2410	306	359	274	346	439
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.32	0.00	0.01	0.44	0.15	0.12	0.25	0.01	0.24

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
6: Brownridge Road/Fifth Line & Steeles Avenue

Base Year - PM Peak Hour

Premier Gateway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	689	3	2	1001	12	43	7	35	65	3	101
Future Volume (vph)	47	689	3	2	1001	12	43	7	35	65	3	101
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.0	8.0	8.0	8.0	8.0		6.0	6.0		6.0	6.0	6.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		1.00	1.00		1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00		1.00	0.87		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1687	3112	1292	1357	3298		1687	1454		1570	1520	1568
Flt Permitted	0.26	1.00	1.00	0.38	1.00		0.76	1.00		0.73	1.00	1.00
Satd. Flow (perm)	461	3112	1292	541	3298		1342	1454		1204	1520	1568
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	49	718	3	2	1043	12	45	7	36	68	3	105
RTOR Reduction (vph)	0	0	1	0	1	0	0	32	0	0	0	93
Lane Group Flow (vph)	49	718	2	2	1055	0	45	11	0	68	3	12
Heavy Vehicles (%)	7%	16%	25%	33%	9%	29%	7%	0%	17%	15%	25%	3%
Turn Type	Perm	NA	Perm	Perm	NA		Perm	NA		Perm	NA	Perm
Protected Phases		4			8			2				6
Permitted Phases	4		4	8			2			6		6
Actuated Green, G (s)	52.8	52.8	52.8	52.8	52.8		9.0	9.0		9.0	9.0	9.0
Effective Green, g (s)	52.8	52.8	52.8	52.8	52.8		9.0	9.0		9.0	9.0	9.0
Actuated g/C Ratio	0.70	0.70	0.70	0.70	0.70		0.12	0.12		0.12	0.12	0.12
Clearance Time (s)	8.0	8.0	8.0	8.0	8.0		6.0	6.0		6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	321	2167	899	376	2297		159	172		142	180	186
v/s Ratio Prot		0.23			c0.32			0.01				0.00
v/s Ratio Perm	0.11		0.00	0.00			0.03			c0.06		0.01
v/c Ratio	0.15	0.33	0.00	0.01	0.46		0.28	0.07		0.48	0.02	0.07
Uniform Delay, d1	3.9	4.5	3.5	3.5	5.1		30.5	29.7		31.2	29.5	29.7
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	1.0	0.4	0.0	0.0	0.7		1.0	0.2		2.5	0.0	0.2
Delay (s)	4.9	4.9	3.5	3.5	5.8		31.4	29.8		33.7	29.5	29.8
Level of Service	A	A	A	A	A		C	C		C	C	C
Approach Delay (s)		4.9			5.8			30.7			31.3	
Approach LOS		A			A			C			C	

Intersection Summary		
HCM 2000 Control Delay	8.7	HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio	0.46	A
Actuated Cycle Length (s)	75.8	Sum of lost time (s)
Intersection Capacity Utilization	68.3%	14.0
Analysis Period (min)	15	ICU Level of Service
c Critical Lane Group		C

Queues  
7: Fifth Line South & Steeles Avenue

Base Year - PM Peak Hour  
Premier Gateway



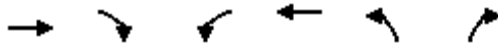
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	838	1	2	1065	15	10
v/c Ratio	0.30	0.00	0.00	0.36	0.07	0.05
Control Delay	2.5	2.0	3.0	2.7	36.5	20.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	2.5	2.0	3.0	2.7	36.5	20.6
Queue Length 50th (m)	0.0	0.0	0.0	0.0	2.0	0.0
Queue Length 95th (m)	32.1	0.4	0.6	43.1	8.7	5.0
Internal Link Dist (m)	679.6			455.7	532.9	
Turn Bay Length (m)		30.0	60.0		15.0	
Base Capacity (vph)	2787	1446	573	2939	271	235
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.30	0.00	0.00	0.36	0.06	0.04

Intersection Summary

# HCM Signalized Intersection Capacity Analysis

## 7: Fifth Line South & Steeles Avenue

Base Year - PM Peak Hour  
Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑
Traffic Volume (vph)	788	1	2	1001	14	9
Future Volume (vph)	788	1	2	1001	14	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.0	8.0	8.0	8.0	6.0	6.0
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3112	1615	1805	3282	1703	1429
Flt Permitted	1.00	1.00	0.34	1.00	0.95	1.00
Satd. Flow (perm)	3112	1615	639	3282	1703	1429
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	838	1	2	1065	15	10
RTOR Reduction (vph)	0	0	0	0	0	10
Lane Group Flow (vph)	838	1	2	1065	15	0
Heavy Vehicles (%)	16%	0%	0%	10%	6%	13%
Turn Type	NA	Perm	Perm	NA	Perm	Perm
Protected Phases	4			8		
Permitted Phases		4	8		2	2
Actuated Green, G (s)	68.6	68.6	68.6	68.6	3.6	3.6
Effective Green, g (s)	68.6	68.6	68.6	68.6	3.6	3.6
Actuated g/C Ratio	0.80	0.80	0.80	0.80	0.04	0.04
Clearance Time (s)	8.0	8.0	8.0	8.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	2476	1285	508	2611	71	59
v/s Ratio Prot	0.27			c0.32		
v/s Ratio Perm		0.00	0.00		c0.01	0.00
v/c Ratio	0.34	0.00	0.00	0.41	0.21	0.01
Uniform Delay, d1	2.5	1.8	1.8	2.7	39.9	39.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.4	0.0	0.0	0.5	1.5	0.0
Delay (s)	2.8	1.8	1.8	3.1	41.4	39.6
Level of Service	A	A	A	A	D	D
Approach Delay (s)	2.8			3.1	40.7	
Approach LOS	A			A	D	

### Intersection Summary

HCM 2000 Control Delay	3.5	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.40		
Actuated Cycle Length (s)	86.2	Sum of lost time (s)	14.0
Intersection Capacity Utilization	55.0%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			



# HCM Unsignalized Intersection Capacity Analysis

## 8: Steeles Avenue & Sixth Line

Base Year - PM Peak Hour  
Premier Gateway



Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	↶	↗↗	↗↗	↶	↶	↶		
Traffic Volume (veh/h)	44	753	972	18	5	31		
Future Volume (Veh/h)	44	753	972	18	5	31		
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94		
Hourly flow rate (vph)	47	801	1034	19	5	33		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type		None	None					
Median storage (veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	1053				1528	517		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	1053				1528	517		
tC, single (s)	4.1				6.8	6.9		
tC, 2 stage (s)								
tF (s)	2.2				3.5	3.3		
p0 queue free %	93				95	94		
cM capacity (veh/h)	657				102	509		
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	SB 2
Volume Total	47	400	400	517	517	19	5	33
Volume Left	47	0	0	0	0	0	5	0
Volume Right	0	0	0	0	0	19	0	33
cSH	657	1700	1700	1700	1700	1700	102	509
Volume to Capacity	0.07	0.24	0.24	0.30	0.30	0.01	0.05	0.06
Queue Length 95th (m)	1.8	0.0	0.0	0.0	0.0	0.0	1.2	1.7
Control Delay (s)	10.9	0.0	0.0	0.0	0.0	0.0	42.0	12.6
Lane LOS	B						E	B
Approach Delay (s)	0.6			0.0			16.4	
Approach LOS							C	
Intersection Summary								
Average Delay			0.6					
Intersection Capacity Utilization			43.5%		ICU Level of Service			A
Analysis Period (min)			15					

# HCM Unsignalized Intersection Capacity Analysis

## 9: Sixth Line South & Steeles Avenue

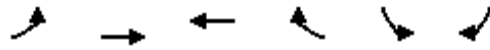
Base Year - PM Peak Hour  
Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↑↑	↑	↓	↑↑	↓	↓	
Traffic Volume (veh/h)	758	0	0	983	7	2	
Future Volume (Veh/h)	758	0	0	983	7	2	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Hourly flow rate (vph)	798	0	0	1035	7	2	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage (veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume			798		1316	399	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			798		1316	399	
tC, single (s)			4.1		6.8	6.9	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		95	100	
cM capacity (veh/h)			833		152	606	
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1
Volume Total	399	399	0	0	518	518	9
Volume Left	0	0	0	0	0	0	7
Volume Right	0	0	0	0	0	0	2
cSH	1700	1700	1700	1700	1700	1700	182
Volume to Capacity	0.23	0.23	0.00	0.00	0.30	0.30	0.05
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0	1.2
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	25.8
Lane LOS							D
Approach Delay (s)	0.0			0.0			25.8
Approach LOS							D
Intersection Summary							
Average Delay			0.1				
Intersection Capacity Utilization			37.2%		ICU Level of Service		A
Analysis Period (min)			15				

HCM Unsignalized Intersection Capacity Analysis  
 10: Steeles Avenue & Hornby Road

Base Year - PM Peak Hour  
 Premier Gateway



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↵	↑↑	↑↑	↵	↵	↵
Traffic Volume (veh/h)	30	730	922	14	7	61
Future Volume (Veh/h)	30	730	922	14	7	61
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	31	760	960	15	7	64
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	975				1402	480
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	975				1402	480
tC, single (s)	4.3				7.2	7.1
tC, 2 stage (s)						
tF (s)	2.3				3.7	3.4
p0 queue free %	95				93	87
cM capacity (veh/h)	656				106	511

Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	SB 2
Volume Total	31	380	380	480	480	15	7	64
Volume Left	31	0	0	0	0	0	7	0
Volume Right	0	0	0	0	0	15	0	64
cSH	656	1700	1700	1700	1700	1700	106	511
Volume to Capacity	0.05	0.22	0.22	0.28	0.28	0.01	0.07	0.13
Queue Length 95th (m)	1.2	0.0	0.0	0.0	0.0	0.0	1.7	3.4
Control Delay (s)	10.8	0.0	0.0	0.0	0.0	0.0	41.4	13.1
Lane LOS	B						E	B
Approach Delay (s)	0.4			0.0			15.8	
Approach LOS							C	

Intersection Summary		
Average Delay		0.8
Intersection Capacity Utilization	35.9%	ICU Level of Service
Analysis Period (min)	15	A

HCM Unsignalized Intersection Capacity Analysis  
 11: Trafalgar Rd & Hornby Rd

Base Year - PM Peak Hour  
 Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	38	6	3	864	435	65
Future Volume (Veh/h)	38	6	3	864	435	65
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Hourly flow rate (vph)	39	6	3	882	444	66
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	924	255	444			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	924	255	444			
tC, single (s)	7.0	7.1	4.3			
tC, 2 stage (s)						
tF (s)	3.6	3.4	2.3			
p0 queue free %	85	99	100			
cM capacity (veh/h)	257	721	1058			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	45	297	588	296	214	
Volume Left	39	3	0	0	0	
Volume Right	6	0	0	0	66	
cSH	281	1058	1700	1700	1700	
Volume to Capacity	0.16	0.00	0.35	0.17	0.13	
Queue Length 95th (m)	4.5	0.1	0.0	0.0	0.0	
Control Delay (s)	20.2	0.1	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	20.2	0.0		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			36.0%	ICU Level of Service		A
Analysis Period (min)			15			

Queues  
12: Trafalgar Road & Steeles Avenue

Base Year - PM Peak Hour  
Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	57	518	193	636	758	151	200	695	641	73	387
v/c Ratio	0.25	0.79	0.42	0.88	0.58	0.21	0.61	0.60	0.74	0.28	0.44
Control Delay	20.9	55.3	7.3	62.5	31.7	3.8	61.6	39.2	15.2	25.4	40.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	20.9	55.3	7.3	62.5	31.7	3.8	61.6	39.2	15.2	25.4	40.9
Queue Length 50th (m)	7.5	67.0	0.0	82.2	81.6	0.0	25.6	82.4	31.0	10.9	44.4
Queue Length 95th (m)	13.7	82.3	16.9	#111.5	94.1	11.5	39.2	109.6	90.7	22.6	63.8
Internal Link Dist (m)		443.0			287.3			749.5			265.5
Turn Bay Length (m)	115.0		40.0	130.0		70.0	100.0		65.0		
Base Capacity (vph)	232	814	522	741	1430	773	342	1158	861	260	888
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.64	0.37	0.86	0.53	0.20	0.58	0.60	0.74	0.28	0.44































Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
12: Trafalgar Road & Steeles Avenue

Base Year - PM Peak Hour

Premier Gateway

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 		 	 		 	 			 	
Traffic Volume (vph)	55	497	185	611	728	145	192	667	615	70	355	16
Future Volume (vph)	55	497	185	611	728	145	192	667	615	70	355	16
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	7.0	7.0	5.0	7.0	7.0	5.0	8.0	8.0	4.0	8.0	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.95	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1583	3085	1417	3433	3312	1583	3099	3438	1568	1570	3277	
Flt Permitted	0.36	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.32	1.00	
Satd. Flow (perm)	607	3085	1417	3433	3312	1583	3099	3438	1568	525	3277	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	57	518	193	636	758	151	200	695	641	73	370	17
RTOR Reduction (vph)	0	0	151	0	0	92	0	0	339	0	2	0
Lane Group Flow (vph)	57	518	42	636	758	59	200	695	302	73	385	0
Heavy Vehicles (%)	14%	17%	14%	2%	9%	2%	13%	5%	3%	15%	9%	19%
Turn Type	pm+pt	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4			8			2	6		
Actuated Green, G (s)	33.0	27.4	27.4	26.3	49.1	49.1	13.3	40.6	40.6	39.7	33.0	
Effective Green, g (s)	33.0	27.4	27.4	26.3	49.1	49.1	13.3	40.6	40.6	39.7	33.0	
Actuated g/C Ratio	0.26	0.22	0.22	0.21	0.39	0.39	0.11	0.32	0.32	0.32	0.26	
Clearance Time (s)	4.0	7.0	7.0	5.0	7.0	7.0	5.0	8.0	8.0	4.0	8.0	
Vehicle Extension (s)	3.0	3.0	3.0	4.0	3.0	3.0	4.0	0.2	0.2	3.0	0.2	
Lane Grp Cap (vph)	203	676	310	722	1300	621	329	1116	509	222	865	
v/s Ratio Prot	0.01	c0.17		c0.19	0.23		c0.06	c0.20		0.02	0.12	
v/s Ratio Perm	0.06		0.03			0.04			0.19	0.09		
v/c Ratio	0.28	0.77	0.14	0.88	0.58	0.10	0.61	0.62	0.59	0.33	0.44	
Uniform Delay, d1	35.1	45.8	39.3	47.8	29.9	23.9	53.4	35.7	35.3	30.7	38.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.8	5.2	0.2	12.4	0.7	0.1	3.7	2.6	5.0	0.9	1.7	
Delay (s)	35.9	51.0	39.5	60.3	30.6	24.0	57.0	38.3	40.3	31.6	40.0	
Level of Service	D	D	D	E	C	C	E	D	D	C	D	
Approach Delay (s)		47.0			42.2			41.6			38.7	
Approach LOS		D			D			D			D	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			42.4		HCM 2000 Level of Service					D		
HCM 2000 Volume to Capacity ratio			0.74									
Actuated Cycle Length (s)			125.0		Sum of lost time (s)					25.0		
Intersection Capacity Utilization			78.4%		ICU Level of Service					D		
Analysis Period (min)			15									
c Critical Lane Group												

Queues  
13: Toronto Premier Outlets & Steeles Avenue

Base Year - PM Peak Hour  
Premier Gateway



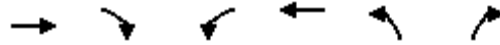
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	848	383	21	1315	231	52
v/c Ratio	0.52	0.39	0.06	0.71	0.20	0.10
Control Delay	19.4	3.1	10.2	19.8	23.9	7.0
Queue Delay	0.0	0.0	0.0	0.4	0.0	0.0
Total Delay	19.4	3.1	10.2	20.2	23.9	7.0
Queue Length 50th (m)	53.5	0.0	1.8	99.5	17.0	0.0
Queue Length 95th (m)	88.3	16.7	5.2	125.5	26.1	8.2
Internal Link Dist (m)	287.3			176.7	95.1	
Turn Bay Length (m)		130.0	45.0			40.0
Base Capacity (vph)	1633	988	394	1856	1178	529
Starvation Cap Reductn	0	0	0	160	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.52	0.39	0.05	0.78	0.20	0.10

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
13: Toronto Premier Outlets & Steeles Avenue

Base Year - PM Peak Hour

Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↵	↑↑	↵↵	↵
Traffic Volume (vph)	814	368	20	1262	222	50
Future Volume (vph)	814	368	20	1262	222	50
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	4.0	6.0	6.0	6.0
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3282	1599	1752	3438	3467	1455
Flt Permitted	1.00	1.00	0.24	1.00	0.95	1.00
Satd. Flow (perm)	3282	1599	448	3438	3467	1455
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	848	383	21	1315	231	52
RTOR Reduction (vph)	0	192	0	0	0	36
Lane Group Flow (vph)	848	191	21	1315	231	16
Heavy Vehicles (%)	10%	1%	3%	5%	1%	11%
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4		3	8	2	
Permitted Phases		4	8			2
Actuated Green, G (s)	49.8	49.8	56.4	56.4	31.6	31.6
Effective Green, g (s)	49.8	49.8	56.4	56.4	31.6	31.6
Actuated g/C Ratio	0.50	0.50	0.56	0.56	0.32	0.32
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0
Vehicle Extension (s)	0.2	0.2	3.0	0.2	4.0	4.0
Lane Grp Cap (vph)	1634	796	286	1939	1095	459
v/s Ratio Prot	0.26		0.00	c0.38	c0.07	
v/s Ratio Perm		0.12	0.04			0.01
v/c Ratio	0.52	0.24	0.07	0.68	0.21	0.04
Uniform Delay, d1	17.0	14.3	10.7	15.4	25.1	23.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.2	0.7	0.1	1.9	0.4	0.1
Delay (s)	18.2	15.0	10.8	17.3	25.5	23.8
Level of Service	B	B	B	B	C	C
Approach Delay (s)	17.2			17.2	25.2	
Approach LOS	B			B	C	

Intersection Summary

HCM 2000 Control Delay	18.0	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.53		
Actuated Cycle Length (s)	100.0	Sum of lost time (s)	16.0
Intersection Capacity Utilization	53.2%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

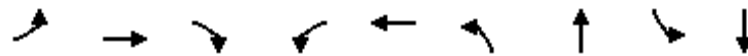


## Queues

Base Year - PM Peak Hour

## 14: Toronto Premium Outlets/Eighth Line &amp; Steeles Avenue

Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	231	672	16	121	1367	55	134	35	82
v/c Ratio	0.80	0.47	0.02	0.27	0.97	0.15	0.25	0.21	0.29
Control Delay	41.8	20.7	0.1	10.9	46.8	40.8	9.1	41.2	17.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	41.8	20.7	0.1	10.9	46.8	40.8	9.1	41.2	17.7
Queue Length 50th (m)	27.6	49.1	0.0	10.1	~143.0	5.2	4.0	6.3	3.5
Queue Length 95th (m)	#70.3	68.4	0.0	18.3	#193.8	11.1	17.5	15.9	17.3
Internal Link Dist (m)		176.7			846.8		194.1		472.6
Turn Bay Length (m)	105.0		55.0	30.0				20.0	
Base Capacity (vph)	289	1436	794	496	1406	1229	992	166	281
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.80	0.47	0.02	0.24	0.97	0.04	0.14	0.21	0.29

## Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 14: Toronto Premium Outlets/Eighth Line & Steeles Avenue

Base Year - PM Peak Hour

Premier Gateway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	217	632	15	114	1172	113	52	27	99	33	19	58
Future Volume (vph)	217	632	15	114	1172	113	52	27	99	33	19	58
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	6.0	6.0	4.0	6.0		7.0	7.0		7.0	7.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.97	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.99		1.00	0.88		1.00	0.89	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1752	3223	1615	1770	3343		3467	1664		1687	1635	
Flt Permitted	0.10	1.00	1.00	0.35	1.00		0.95	1.00		0.67	1.00	
Satd. Flow (perm)	177	3223	1615	652	3343		3467	1664		1192	1635	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	231	672	16	121	1247	120	55	29	105	35	20	62
RTOR Reduction (vph)	0	0	9	0	5	0	0	74	0	0	53	0
Lane Group Flow (vph)	231	672	7	121	1362	0	55	60	0	35	29	0
Heavy Vehicles (%)	3%	12%	0%	2%	7%	2%	1%	0%	1%	7%	0%	4%
Turn Type	pm+pt	NA	Perm	pm+pt	NA		Prot	NA		Perm	NA	
Protected Phases	7	4		3	8		5	2				6
Permitted Phases	4		4	8						6		
Actuated Green, G (s)	52.8	41.7	41.7	47.8	39.2		7.7	27.8		13.1	13.1	
Effective Green, g (s)	52.8	41.7	41.7	47.8	39.2		7.7	27.8		13.1	13.1	
Actuated g/C Ratio	0.56	0.44	0.44	0.50	0.41		0.08	0.29		0.14	0.14	
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		7.0	7.0		7.0	7.0	
Vehicle Extension (s)	3.0	0.2	0.2	3.0	0.2		4.0	4.0		3.0	3.0	
Lane Grp Cap (vph)	282	1413	708	428	1377		280	486		164	225	
v/s Ratio Prot	c0.10	0.21		0.03	c0.41		c0.02	0.04			0.02	
v/s Ratio Perm	0.36		0.00	0.12						c0.03		
v/c Ratio	0.82	0.48	0.01	0.28	0.99		0.20	0.12		0.21	0.13	
Uniform Delay, d1	23.7	18.9	15.1	12.8	27.7		40.8	24.7		36.4	36.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	16.7	1.1	0.0	0.4	21.7		0.5	0.2		3.0	1.2	
Delay (s)	40.4	20.1	15.1	13.1	49.5		41.3	24.9		39.4	37.1	
Level of Service	D	C	B	B	D		D	C		D	D	
Approach Delay (s)		25.1			46.5			29.6			37.8	
Approach LOS		C			D			C			D	

Intersection Summary			
HCM 2000 Control Delay	37.7	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.73		
Actuated Cycle Length (s)	95.1	Sum of lost time (s)	24.0
Intersection Capacity Utilization	84.7%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			

HCM Unsignalized Intersection Capacity Analysis  
 15: Eighth Line South & Steeles Avenue

Base Year - PM Peak Hour  
 Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↑↑		↙	↑↑	↙	↗	
Traffic Volume (veh/h)	762	2	0	1398	1	5	
Future Volume (Veh/h)	762	2	0	1398	1	5	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	
Hourly flow rate (vph)	794	2	0	1456	1	5	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage (veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume			796		1523	398	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			796		1523	398	
tC, single (s)			4.1		6.8	7.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.5	
p0 queue free %			100		99	99	
cM capacity (veh/h)			835		111	561	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1	NB 2
Volume Total	529	267	0	728	728	1	5
Volume Left	0	0	0	0	0	1	0
Volume Right	0	2	0	0	0	0	5
cSH	1700	1700	1700	1700	1700	111	561
Volume to Capacity	0.31	0.16	0.00	0.43	0.43	0.01	0.01
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.2	0.2
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	37.7	11.5
Lane LOS						E	B
Approach Delay (s)	0.0		0.0			15.8	
Approach LOS						C	
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization			48.6%	ICU Level of Service		A	
Analysis Period (min)			15				

Queues  
16: Steeles Avenue & Ninth Line

Base Year - PM Peak Hour  
Premier Gateway



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	108	699	1401	743	280	71
v/c Ratio	0.53	0.35	0.82	0.63	0.65	0.16
Control Delay	19.9	10.3	26.0	3.9	41.5	8.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	19.9	10.3	26.0	3.9	41.5	8.4
Queue Length 50th (m)	7.7	34.3	121.6	0.0	51.2	0.0
Queue Length 95th (m)	20.8	45.3	152.8	18.4	80.0	10.9
Internal Link Dist (m)		501.4	674.5		3096.2	
Turn Bay Length (m)	65.0			75.0	90.0	
Base Capacity (vph)	203	2002	1719	1171	434	437
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.53	0.35	0.82	0.63	0.65	0.16
<b>Intersection Summary</b>						

HCM Signalized Intersection Capacity Analysis  
 16: Steeles Avenue & Ninth Line

Base Year - PM Peak Hour  
 Premier Gateway



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	103	664	1331	706	266	67
Future Volume (vph)	103	664	1331	706	266	67
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	7.0	7.0	7.0	7.0	7.0
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Frt	1.00	1.00	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1736	3282	3438	1599	1736	1538
Flt Permitted	0.08	1.00	1.00	1.00	0.95	1.00
Satd. Flow (perm)	145	3282	3438	1599	1736	1538
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	108	699	1401	743	280	71
RTOR Reduction (vph)	0	0	0	372	0	53
Lane Group Flow (vph)	108	699	1401	372	280	18
Heavy Vehicles (%)	4%	10%	5%	1%	4%	5%
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	7	4	8		6	
Permitted Phases	4			8		6
Actuated Green, G (s)	61.0	61.0	50.0	50.0	25.0	25.0
Effective Green, g (s)	61.0	61.0	50.0	50.0	25.0	25.0
Actuated g/C Ratio	0.61	0.61	0.50	0.50	0.25	0.25
Clearance Time (s)	4.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	0.2	0.2	0.2	3.0	3.0
Lane Grp Cap (vph)	199	2002	1719	799	434	384
v/s Ratio Prot	c0.04	0.21	c0.41		c0.16	
v/s Ratio Perm	0.29			0.23		0.01
v/c Ratio	0.54	0.35	0.82	0.46	0.65	0.05
Uniform Delay, d1	15.4	9.7	21.1	16.3	33.5	28.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	3.0	0.5	4.4	1.9	7.2	0.2
Delay (s)	18.4	10.1	25.5	18.2	40.7	28.7
Level of Service	B	B	C	B	D	C
Approach Delay (s)		11.2	23.0		38.3	
Approach LOS		B	C		D	

**Intersection Summary**

HCM 2000 Control Delay	21.7	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.74		
Actuated Cycle Length (s)	100.0	Sum of lost time (s)	18.0
Intersection Capacity Utilization	72.4%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
17: Ninth Line (South) & Steeles Avenue

Base Year - PM Peak Hour  
Premier Gateway



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	743	247	306	1649	518	338
v/c Ratio	0.69	0.36	0.74	0.94	0.84	0.43
Control Delay	33.0	4.9	24.6	34.4	43.9	4.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.0	4.9	24.6	34.4	43.9	4.6
Queue Length 50th (m)	68.3	0.0	32.0	155.7	97.3	0.0
Queue Length 95th (m)	91.4	17.1	52.5	#213.6	#155.9	18.7
Internal Link Dist (m)	674.5			487.1	143.5	
Turn Bay Length (m)		75.0	145.0		60.0	
Base Capacity (vph)	1098	699	444	1822	619	780
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.68	0.35	0.69	0.91	0.84	0.43

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 17: Ninth Line (South) & Steeles Avenue

Base Year - PM Peak Hour  
 Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↓	↑↑	↓	↑
Traffic Volume (vph)	698	232	288	1550	487	318
Future Volume (vph)	698	232	288	1550	487	318
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.0	7.0	4.0	7.0	7.0	7.0
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3282	1599	1787	3438	1787	1615
Flt Permitted	1.00	1.00	0.21	1.00	0.95	1.00
Satd. Flow (perm)	3282	1599	402	3438	1787	1615
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	743	247	306	1649	518	338
RTOR Reduction (vph)	0	166	0	0	0	221
Lane Group Flow (vph)	743	81	306	1649	518	117
Heavy Vehicles (%)	10%	1%	1%	5%	1%	0%
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4		3	8	2	
Permitted Phases		4	8			2
Actuated Green, G (s)	32.3	32.3	50.2	50.2	34.0	34.0
Effective Green, g (s)	32.3	32.3	50.2	50.2	34.0	34.0
Actuated g/C Ratio	0.33	0.33	0.51	0.51	0.35	0.35
Clearance Time (s)	7.0	7.0	4.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1079	525	401	1757	618	559
v/s Ratio Prot	0.23		0.11	c0.48	c0.29	
v/s Ratio Perm		0.05	0.28			0.07
v/c Ratio	0.69	0.15	0.76	0.94	0.84	0.21
Uniform Delay, d1	28.6	23.3	16.3	22.6	29.6	22.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.8	0.1	8.4	10.2	12.8	0.9
Delay (s)	30.4	23.4	24.6	32.7	42.4	23.5
Level of Service	C	C	C	C	D	C
Approach Delay (s)	28.7			31.4	34.9	
Approach LOS	C			C	C	

Intersection Summary			
HCM 2000 Control Delay	31.5	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.94		
Actuated Cycle Length (s)	98.2	Sum of lost time (s)	18.0
Intersection Capacity Utilization	81.5%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

Queues

Base Year - PM Peak Hour

18: James Snow Parkway & Hwy 401 (Westbound Ramp)

Premier Gateway



Lane Group	WBL	WBR	NBT	SBT
Lane Group Flow (vph)	740	254	442	935
v/c Ratio	0.74	0.44	0.28	0.57
Control Delay	27.4	5.4	12.9	16.2
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	27.4	5.4	12.9	16.2
Queue Length 50th (m)	47.1	0.0	19.0	47.7
Queue Length 95th (m)	65.2	16.5	32.0	73.1
Internal Link Dist (m)	390.4		415.8	504.8
Turn Bay Length (m)				
Base Capacity (vph)	1234	653	1555	1630
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.60	0.39	0.28	0.57

Intersection Summary



HCM Signalized Intersection Capacity Analysis  
 18: James Snow Parkway & Hwy 401 (Westbound Ramp)

Base Year - PM Peak Hour  
 Premier Gateway



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	←←	←	↑↑			↑↑
Traffic Volume (vph)	676	268	420	0	0	888
Future Volume (vph)	676	268	420	0	0	888
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.2	8.2	9.3			9.3
Lane Util. Factor	0.97	0.91	0.95			0.95
Frt	0.99	0.85	1.00			1.00
Flt Protected	0.95	1.00	1.00			1.00
Satd. Flow (prot)	3489	1386	3343			3505
Flt Permitted	0.95	1.00	1.00			1.00
Satd. Flow (perm)	3489	1386	3343			3505
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	712	282	442	0	0	935
RTOR Reduction (vph)	4	181	0	0	0	0
Lane Group Flow (vph)	736	73	442	0	0	935
Heavy Vehicles (%)	0%	6%	8%	0%	0%	3%
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Actuated Green, G (s)	20.2	20.2	32.8			32.8
Effective Green, g (s)	20.2	20.2	32.8			32.8
Actuated g/C Ratio	0.29	0.29	0.47			0.47
Clearance Time (s)	8.2	8.2	9.3			9.3
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	999	397	1555			1630
v/s Ratio Prot	c0.21		0.13			c0.27
v/s Ratio Perm		0.05				
v/c Ratio	0.74	0.18	0.28			0.57
Uniform Delay, d1	22.7	18.9	11.6			13.7
Progression Factor	1.00	1.00	1.00			1.00
Incremental Delay, d2	2.9	0.2	0.5			1.5
Delay (s)	25.6	19.2	12.1			15.2
Level of Service	C	B	B			B
Approach Delay (s)	24.0		12.1			15.2
Approach LOS	C		B			B

Intersection Summary			
HCM 2000 Control Delay	18.3	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.64		
Actuated Cycle Length (s)	70.5	Sum of lost time (s)	17.5
Intersection Capacity Utilization	61.2%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

Queues

Base Year - PM Peak Hour

19: James Snow Parkway & Hwy 401 (Eastbound Ramp)

Premier Gateway



Lane Group	EBL	EBR	NBT	SBT
Lane Group Flow (vph)	173	106	573	1324
v/c Ratio	0.37	0.41	0.24	0.54
Control Delay	19.0	17.1	4.9	7.2
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	19.0	17.1	4.9	7.2
Queue Length 50th (m)	6.7	4.9	14.0	42.8
Queue Length 95th (m)	15.3	19.7	22.1	63.8
Internal Link Dist (m)	305.5		1282.4	415.8
Turn Bay Length (m)				
Base Capacity (vph)	566	305	2414	2438
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.31	0.35	0.24	0.54

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 19: James Snow Parkway & Hwy 401 (Eastbound Ramp)

Base Year - PM Peak Hour  
 Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔↔	↗		↕↕	↕↕	
Traffic Volume (vph)	65	209	0	562	1298	0
Future Volume (vph)	65	209	0	562	1298	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0		7.4	7.4	
Lane Util. Factor	0.97	0.91		0.95	0.95	
Frt	0.91	0.85		1.00	1.00	
Flt Protected	0.98	1.00		1.00	1.00	
Satd. Flow (prot)	2936	1427		3539	3574	
Flt Permitted	0.98	1.00		1.00	1.00	
Satd. Flow (perm)	2936	1427		3539	3574	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	66	213	0	573	1324	0
RTOR Reduction (vph)	61	61	0	0	0	0
Lane Group Flow (vph)	112	45	0	573	1324	0
Heavy Vehicles (%)	26%	3%	0%	2%	1%	0%
Turn Type	Prot	Perm		NA	NA	
Protected Phases	4			2	6	
Permitted Phases		4				
Actuated Green, G (s)	10.5	10.5		51.5	51.5	
Effective Green, g (s)	10.5	10.5		51.5	51.5	
Actuated g/C Ratio	0.14	0.14		0.68	0.68	
Clearance Time (s)	6.0	6.0		7.4	7.4	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	408	198		2417	2441	
v/s Ratio Prot	c0.04			0.16	c0.37	
v/s Ratio Perm		0.03				
v/c Ratio	0.27	0.23		0.24	0.54	
Uniform Delay, d1	29.0	28.8		4.5	6.0	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.4	0.6		0.2	0.9	
Delay (s)	29.4	29.4		4.8	6.9	
Level of Service	C	C		A	A	
Approach Delay (s)	29.4			4.8	6.9	
Approach LOS	C			A	A	

Intersection Summary			
HCM 2000 Control Delay	9.2	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.50		
Actuated Cycle Length (s)	75.4	Sum of lost time (s)	13.4
Intersection Capacity Utilization	61.2%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

## Queues

Base Year - PM Peak Hour

## 20: Trafalgar Road &amp; Hwy 401 (Westbound Ramp)

Premier Gateway



Lane Group	WBL	WBR	NBT	SBT
Lane Group Flow (vph)	611	303	913	705
v/c Ratio	0.75	0.76	0.41	0.33
Control Delay	34.0	34.8	9.8	9.0
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	34.0	34.8	9.8	9.0
Queue Length 50th (m)	47.1	40.4	39.5	28.4
Queue Length 95th (m)	65.4	73.4	70.8	52.2
Internal Link Dist (m)	383.1		312.7	749.5
Turn Bay Length (m)				
Base Capacity (vph)	1686	771	2201	2141
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.36	0.39	0.41	0.33

## Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 20: Trafalgar Road & Hwy 401 (Westbound Ramp)

Base Year - PM Peak Hour  
 Premier Gateway



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔↔	↗	↕↕			↕↕
Traffic Volume (vph)	299	588	886	0	0	684
Future Volume (vph)	299	588	886	0	0	684
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	6.0			6.0
Lane Util. Factor	0.97	0.91	0.95			0.95
Frt	0.93	0.85	1.00			1.00
Flt Protected	0.98	1.00	1.00			1.00
Satd. Flow (prot)	3200	1413	3438			3343
Flt Permitted	0.98	1.00	1.00			1.00
Satd. Flow (perm)	3200	1413	3438			3343
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	308	606	913	0	0	705
RTOR Reduction (vph)	75	75	0	0	0	0
Lane Group Flow (vph)	536	228	913	0	0	705
Heavy Vehicles (%)	4%	4%	5%	0%	0%	8%
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Actuated Green, G (s)	21.8	21.8	60.2			60.2
Effective Green, g (s)	21.8	21.8	60.2			60.2
Actuated g/C Ratio	0.23	0.23	0.64			0.64
Clearance Time (s)	6.0	6.0	6.0			6.0
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	742	327	2201			2140
v/s Ratio Prot	c0.17		c0.27			0.21
v/s Ratio Perm		0.16				
v/c Ratio	0.72	0.70	0.41			0.33
Uniform Delay, d1	33.3	33.1	8.3			7.7
Progression Factor	1.00	1.00	1.00			1.00
Incremental Delay, d2	3.5	6.4	0.6			0.4
Delay (s)	36.8	39.5	8.9			8.1
Level of Service	D	D	A			A
Approach Delay (s)	37.7		8.9			8.1
Approach LOS	D		A			A

**Intersection Summary**

HCM 2000 Control Delay	19.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.50		
Actuated Cycle Length (s)	94.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	58.8%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
 21: Trafalgar Road & Hwy 401 (Eastbound Ramp)

Base Year - PM Peak Hour  
 Premier Gateway



Lane Group	EBL	EBR	NBT	SBT
Lane Group Flow (vph)	372	186	1113	771
v/c Ratio	0.71	0.57	0.43	0.31
Control Delay	28.7	13.4	5.0	4.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	28.7	13.4	5.0	4.3
Queue Length 50th (m)	18.7	0.0	34.3	20.8
Queue Length 95th (m)	34.1	21.8	55.4	34.8
Internal Link Dist (m)	204.3		1138.2	312.7
Turn Bay Length (m)				
Base Capacity (vph)	1082	559	2604	2509
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.34	0.33	0.43	0.31
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
 21: Trafalgar Road & Hwy 401 (Eastbound Ramp)

Base Year - PM Peak Hour

Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	181	366	0	1091	756	0
Future Volume (vph)	181	366	0	1091	756	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0		6.0	6.0	
Lane Util. Factor	0.97	0.91		0.95	0.95	
Frt	0.92	0.85		1.00	1.00	
Flt Protected	0.98	1.00		1.00	1.00	
Satd. Flow (prot)	3094	1400		3406	3282	
Flt Permitted	0.98	1.00		1.00	1.00	
Satd. Flow (perm)	3094	1400		3406	3282	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	185	373	0	1113	771	0
RTOR Reduction (vph)	165	164	0	0	0	0
Lane Group Flow (vph)	207	22	0	1113	771	0
Heavy Vehicles (%)	10%	5%	0%	6%	10%	0%
Turn Type	Prot	Perm		NA	NA	
Protected Phases	4			2	6	
Permitted Phases		4				
Actuated Green, G (s)	11.7	11.7		77.1	77.1	
Effective Green, g (s)	11.7	11.7		77.1	77.1	
Actuated g/C Ratio	0.12	0.12		0.76	0.76	
Clearance Time (s)	6.0	6.0		6.0	6.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	359	162		2605	2510	
v/s Ratio Prot	c0.07			c0.33	0.23	
v/s Ratio Perm		0.02				
v/c Ratio	0.58	0.13		0.43	0.31	
Uniform Delay, d1	42.2	40.0		4.1	3.6	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	2.2	0.4		0.5	0.3	
Delay (s)	44.4	40.4		4.7	4.0	
Level of Service	D	D		A	A	
Approach Delay (s)	43.1			4.7	4.0	
Approach LOS	D			A	A	

Intersection Summary			
HCM 2000 Control Delay	13.2	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.45		
Actuated Cycle Length (s)	100.8	Sum of lost time (s)	12.0
Intersection Capacity Utilization	58.8%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

Queues

Base Year - PM Peak Hour

22: Winston Churchill Boulevard & Hwy 401 (Westbound Ramp)

Premier Gateway



Lane Group	WBL	WBR	NBT	SBT
Lane Group Flow (vph)	642	300	1272	1144
v/c Ratio	0.80	0.81	0.57	0.50
Control Delay	50.1	53.7	14.4	13.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	50.1	53.7	14.4	13.4
Queue Length 50th (m)	76.0	67.0	89.3	75.5
Queue Length 95th (m)	97.5	105.4	140.3	119.0
Internal Link Dist (m)	284.7		32.1	320.2
Turn Bay Length (m)				
Base Capacity (vph)	1085	490	2246	2268
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.59	0.61	0.57	0.50

Intersection Summary



HCM Signalized Intersection Capacity Analysis  
 22: Winston Churchill Boulevard & Hwy 401 (Westbound Ramp)

Base Year - PM Peak Hour  
 Premier Gateway



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔↔	↗	↕↕			↕↕
Traffic Volume (vph)	403	492	1208	0	0	1087
Future Volume (vph)	403	492	1208	0	0	1087
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.0	7.0	8.0			8.0
Lane Util. Factor	0.97	0.91	0.95			0.95
Frt	0.95	0.85	1.00			1.00
Flt Protected	0.97	1.00	1.00			1.00
Satd. Flow (prot)	3266	1400	3438			3471
Flt Permitted	0.97	1.00	1.00			1.00
Satd. Flow (perm)	3266	1400	3438			3471
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	424	518	1272	0	0	1144
RTOR Reduction (vph)	49	49	0	0	0	0
Lane Group Flow (vph)	593	251	1272	0	0	1144
Heavy Vehicles (%)	3%	5%	5%	0%	0%	4%
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Actuated Green, G (s)	29.6	29.6	84.2			84.2
Effective Green, g (s)	29.6	29.6	84.2			84.2
Actuated g/C Ratio	0.23	0.23	0.65			0.65
Clearance Time (s)	7.0	7.0	8.0			8.0
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	750	321	2247			2269
v/s Ratio Prot	c0.18		c0.37			0.33
v/s Ratio Perm		0.18				
v/c Ratio	0.79	0.78	0.57			0.50
Uniform Delay, d1	46.7	46.6	12.3			11.5
Progression Factor	1.00	1.00	1.00			1.00
Incremental Delay, d2	5.7	11.7	1.0			0.8
Delay (s)	52.4	58.2	13.3			12.3
Level of Service	D	E	B			B
Approach Delay (s)	54.2		13.3			12.3
Approach LOS	D		B			B

Intersection Summary			
HCM 2000 Control Delay		24.4	HCM 2000 Level of Service C
HCM 2000 Volume to Capacity ratio		0.62	
Actuated Cycle Length (s)		128.8	Sum of lost time (s) 15.0
Intersection Capacity Utilization		182.8%	ICU Level of Service H
Analysis Period (min)		15	
c Critical Lane Group			

## Queues

Base Year - PM Peak Hour

## 23: Winston Churchill Boulevard &amp; Hwy 401 (Eastbound Ramp)

Premier Gateway



Lane Group	EBL	EBR	NBT	SBT
Lane Group Flow (vph)	440	203	1691	1383
v/c Ratio	0.81	0.67	0.66	0.54
Control Delay	64.3	43.4	11.7	9.7
Queue Delay	0.0	0.0	0.0	0.3
Total Delay	64.3	43.4	11.7	10.0
Queue Length 50th (m)	59.1	36.1	121.7	85.3
Queue Length 95th (m)	78.8	67.6	155.3	109.7
Internal Link Dist (m)	152.5		433.2	198.3
Turn Bay Length (m)				
Base Capacity (vph)	624	337	2569	2569
Starvation Cap Reductn	0	0	0	539
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.71	0.60	0.66	0.68

## Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 23: Winston Churchill Boulevar & Hwy 401 (Eastbound Ramp)

Base Year - PM Peak Hour  
 Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	319	292	0	1606	1314	0
Future Volume (vph)	319	292	0	1606	1314	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.0	8.0		7.0	7.0	
Lane Util. Factor	0.97	0.91		0.95	0.95	
Frt	0.96	0.85		1.00	1.00	
Flt Protected	0.96	1.00		1.00	1.00	
Satd. Flow (prot)	3161	1427		3539	3539	
Flt Permitted	0.96	1.00		1.00	1.00	
Satd. Flow (perm)	3161	1427		3539	3539	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	336	307	0	1691	1383	0
RTOR Reduction (vph)	23	68	0	0	0	0
Lane Group Flow (vph)	417	135	0	1691	1383	0
Heavy Vehicles (%)	10%	3%	0%	2%	2%	0%
Turn Type	Prot	Perm		NA	NA	
Protected Phases	4			2	6	
Permitted Phases		4				
Actuated Green, G (s)	22.3	22.3		99.1	99.1	
Effective Green, g (s)	22.3	22.3		99.1	99.1	
Actuated g/C Ratio	0.16	0.16		0.73	0.73	
Clearance Time (s)	8.0	8.0		7.0	7.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	516	233		2571	2571	
v/s Ratio Prot	c0.13			c0.48	0.39	
v/s Ratio Perm		0.09				
v/c Ratio	0.81	0.58		0.66	0.54	
Uniform Delay, d1	55.0	52.7		9.8	8.4	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	9.1	3.6		1.3	0.8	
Delay (s)	64.1	56.4		11.1	9.2	
Level of Service	E	E		B	A	
Approach Delay (s)	61.6			11.1	9.2	
Approach LOS	E			B	A	

Intersection Summary

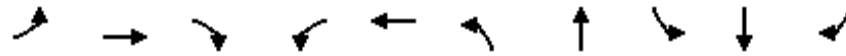
HCM 2000 Control Delay	19.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.69		
Actuated Cycle Length (s)	136.4	Sum of lost time (s)	15.0
Intersection Capacity Utilization	182.8%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

## Queues

Base Year - PM Peak Hour

## 24: James Snow Parkway &amp; Main Street East

Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	362	74	262	83	161	374	636	91	746	749
v/c Ratio	0.81	0.13	0.39	0.49	0.34	0.81	0.41	0.24	0.64	0.83
Control Delay	51.6	20.8	4.6	43.4	27.0	28.8	16.6	11.8	27.4	16.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	51.6	20.8	4.6	43.4	27.0	28.8	16.6	11.8	27.4	16.5
Queue Length 50th (m)	30.4	8.8	0.0	13.0	10.0	31.3	35.3	6.4	54.6	23.8
Queue Length 95th (m)	#56.5	18.3	15.8	26.8	19.0	#84.5	55.0	14.9	80.3	#106.7
Internal Link Dist (m)		274.7			467.9		430.6		1282.4	
Turn Bay Length (m)	70.0		50.0	105.0		100.0		135.0		135.0
Base Capacity (vph)	446	761	804	294	786	460	1543	383	1165	900
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.81	0.10	0.33	0.28	0.20	0.81	0.41	0.24	0.64	0.83

## Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 24: James Snow Parkway & Main Street East

Base Year - PM Peak Hour

Premier Gateway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	344	70	249	79	119	34	355	484	121	86	709	712
Future Volume (vph)	344	70	249	79	119	34	355	484	121	86	709	712
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	6.0	6.0	6.0	6.0		4.5	6.0		4.5	6.0	6.0
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95		1.00	0.95		1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	0.97		1.00	0.97		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	3502	1900	1615	1805	3473		1805	3406		1752	3610	1599
Flt Permitted	0.95	1.00	1.00	0.71	1.00		0.21	1.00		0.41	1.00	1.00
Satd. Flow (perm)	3502	1900	1615	1347	3473		394	3406		757	3610	1599
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	362	74	262	83	125	36	374	509	127	91	746	749
RTOR Reduction (vph)	0	0	182	0	32	0	0	22	0	0	0	379
Lane Group Flow (vph)	362	74	80	83	129	0	374	614	0	91	746	370
Heavy Vehicles (%)	0%	0%	0%	0%	0%	2%	0%	3%	2%	3%	0%	1%
Turn Type	Prot	NA	Perm	Perm	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases	7	4			8		5	2		1	6	
Permitted Phases			4	8			2			6		6
Actuated Green, G (s)	10.5	25.4	25.4	10.4	10.4		46.1	36.9		32.4	27.7	27.7
Effective Green, g (s)	10.5	25.4	25.4	10.4	10.4		46.1	36.9		32.4	27.7	27.7
Actuated g/C Ratio	0.13	0.30	0.30	0.12	0.12		0.55	0.44		0.39	0.33	0.33
Clearance Time (s)	4.5	6.0	6.0	6.0	6.0		4.5	6.0		4.5	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	440	577	491	167	432		452	1505		349	1197	530
v/s Ratio Prot	c0.10	0.04			0.04		c0.14	0.18		0.01	0.21	
v/s Ratio Perm			0.05	c0.06			c0.32			0.09		0.23
v/c Ratio	0.82	0.13	0.16	0.50	0.30		0.83	0.41		0.26	0.62	0.70
Uniform Delay, d1	35.6	21.0	21.3	34.1	33.2		13.1	15.9		16.5	23.5	24.3
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	11.8	0.1	0.2	2.3	0.4		11.8	0.8		0.4	2.5	7.5
Delay (s)	47.4	21.1	21.4	36.4	33.6		24.9	16.7		16.9	26.0	31.7
Level of Service	D	C	C	D	C		C	B		B	C	C
Approach Delay (s)		34.8			34.6			19.7			28.2	
Approach LOS		C			C			B			C	

### Intersection Summary

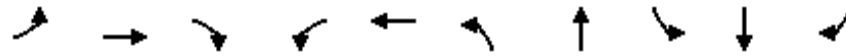
HCM 2000 Control Delay	27.5	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.81		
Actuated Cycle Length (s)	83.5	Sum of lost time (s)	21.0
Intersection Capacity Utilization	81.9%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

## Queues

Base Year - SAT Peak Hour

## 6: Brownridge Road/Fifth Line &amp; Steeles Avenue

Premier Gateway



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	28	419	4	3	523	10	7	14	2	35
v/c Ratio	0.04	0.15	0.00	0.00	0.19	0.06	0.03	0.10	0.01	0.13
Control Delay	4.0	3.3	0.0	4.0	3.4	29.4	18.7	30.7	28.0	3.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	4.0	3.3	0.0	4.0	3.4	29.4	18.7	30.7	28.0	3.9
Queue Length 50th (m)	1.2	10.1	0.0	0.2	13.0	1.6	0.2	2.2	0.3	0.0
Queue Length 95th (m)	3.6	15.1	0.0	0.9	18.9	5.3	3.5	6.7	2.1	3.2
Internal Link Dist (m)		462.3			679.6		261.2		67.4	
Turn Bay Length (m)	145.0		65.0	30.0		20.0		25.0		25.0
Base Capacity (vph)	662	2748	1323	784	2815	337	427	264	485	462
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.04	0.15	0.00	0.00	0.19	0.03	0.02	0.05	0.00	0.08

## Intersection Summary

# HCM Signalized Intersection Capacity Analysis

## 6: Brownridge Road/Fifth Line & Steeles Avenue

Base Year - SAT Peak Hour

Premier Gateway



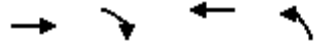
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	27	402	4	3	499	3	10	1	6	13	2	34
Future Volume (vph)	27	402	4	3	499	3	10	1	6	13	2	34
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.0	8.0	8.0	8.0	8.0		6.0	6.0		6.0	6.0	6.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		1.00	1.00		1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00		1.00	0.87		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1687	3374	1615	1805	3456		1656	1656		1308	1900	1615
Flt Permitted	0.46	1.00	1.00	0.51	1.00		0.76	1.00		0.75	1.00	1.00
Satd. Flow (perm)	814	3374	1615	963	3456		1319	1656		1037	1900	1615
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	28	419	4	3	520	3	10	1	6	14	2	35
RTOR Reduction (vph)	0	0	1	0	0	0	0	6	0	0	0	32
Lane Group Flow (vph)	28	419	3	3	523	0	10	1	0	14	2	3
Heavy Vehicles (%)	7%	7%	0%	0%	4%	67%	9%	0%	0%	38%	0%	0%
Turn Type	Perm	NA	Perm	Perm	NA		Perm	NA		Perm	NA	Perm
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8			2			6		6
Actuated Green, G (s)	57.6	57.6	57.6	57.6	57.6		5.8	5.8		5.8	5.8	5.8
Effective Green, g (s)	57.6	57.6	57.6	57.6	57.6		5.8	5.8		5.8	5.8	5.8
Actuated g/C Ratio	0.74	0.74	0.74	0.74	0.74		0.07	0.07		0.07	0.07	0.07
Clearance Time (s)	8.0	8.0	8.0	8.0	8.0		6.0	6.0		6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	605	2510	1201	716	2571		98	124		77	142	121
v/s Ratio Prot		0.12			c0.15			0.00			0.00	
v/s Ratio Perm	0.03		0.00	0.00			0.01			c0.01		0.00
v/c Ratio	0.05	0.17	0.00	0.00	0.20		0.10	0.01		0.18	0.01	0.02
Uniform Delay, d1	2.6	2.9	2.5	2.5	3.0		33.4	33.1		33.6	33.2	33.2
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.1	0.1	0.0	0.0	0.2		0.5	0.0		1.1	0.0	0.1
Delay (s)	2.8	3.0	2.5	2.6	3.2		33.8	33.2		34.7	33.2	33.2
Level of Service	A	A	A	A	A		C	C		C	C	C
Approach Delay (s)		3.0			3.2			33.6			33.6	
Approach LOS		A			A			C			C	

### Intersection Summary

HCM 2000 Control Delay	5.1	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.20		
Actuated Cycle Length (s)	77.4	Sum of lost time (s)	14.0
Intersection Capacity Utilization	68.3%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
7: Fifth Line South & Steeles Avenue

Base Year - SAT Peak Hour  
Premier Gateway



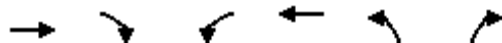
Lane Group	EBT	EBR	WBT	NBL
Lane Group Flow (vph)	440	3	529	2
v/c Ratio	0.14	0.00	0.16	0.01
Control Delay	1.2	1.7	1.2	31.5
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	1.2	1.7	1.2	31.5
Queue Length 50th (m)	0.0	0.0	0.0	0.3
Queue Length 95th (m)	15.2	0.7	18.2	2.4
Internal Link Dist (m)	679.6		455.7	532.9
Turn Bay Length (m)		30.0		15.0
Base Capacity (vph)	3131	1527	3282	410
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.14	0.00	0.16	0.00
<b>Intersection Summary</b>				



# HCM Signalized Intersection Capacity Analysis

## 7: Fifth Line South & Steeles Avenue

Base Year - SAT Peak Hour  
Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑
Traffic Volume (vph)	418	3	0	503	2	0
Future Volume (vph)	418	3	0	503	2	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.0	8.0		8.0	6.0	
Lane Util. Factor	0.95	1.00		0.95	1.00	
Frt	1.00	0.85		1.00	1.00	
Flt Protected	1.00	1.00		1.00	0.95	
Satd. Flow (prot)	3312	1615		3471	1805	
Flt Permitted	1.00	1.00		1.00	0.95	
Satd. Flow (perm)	3312	1615		3471	1805	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	440	3	0	529	2	0
RTOR Reduction (vph)	0	1	0	0	0	0
Lane Group Flow (vph)	440	2	0	529	2	0
Heavy Vehicles (%)	9%	0%	0%	4%	0%	0%
Turn Type	NA	Perm	Perm	NA	Perm	Perm
Protected Phases	4			8		
Permitted Phases		4	8		2	2
Actuated Green, G (s)	64.4	64.4		64.4	1.7	
Effective Green, g (s)	64.4	64.4		64.4	1.7	
Actuated g/C Ratio	0.80	0.80		0.80	0.02	
Clearance Time (s)	8.0	8.0		8.0	6.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	2662	1298		2790	38	
v/s Ratio Prot	0.13			c0.15		
v/s Ratio Perm		0.00			c0.00	
v/c Ratio	0.17	0.00		0.19	0.05	
Uniform Delay, d1	1.8	1.5		1.8	38.4	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.1	0.0		0.2	0.6	
Delay (s)	1.9	1.5		2.0	39.0	
Level of Service	A	A		A	D	
Approach Delay (s)	1.9			2.0	39.0	
Approach LOS	A			A	D	

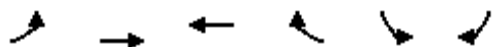
### Intersection Summary

HCM 2000 Control Delay	2.0	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.19		
Actuated Cycle Length (s)	80.1	Sum of lost time (s)	14.0
Intersection Capacity Utilization	40.8%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

# HCM Unsignalized Intersection Capacity Analysis

## 8: Steeles Avenue & Sixth Line

Base Year - SAT Peak Hour  
Premier Gateway

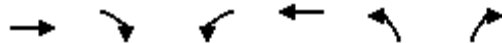


Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	↙	↑↑	↑↑	↘	↙	↘			
Traffic Volume (veh/h)	32	386	487	8	7	16			
Future Volume (Veh/h)	32	386	487	8	7	16			
Sign Control	Free		Free		Stop				
Grade	0%		0%		0%				
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94			
Hourly flow rate (vph)	34	411	518	9	7	17			
Pedestrians									
Lane Width (m)									
Walking Speed (m/s)									
Percent Blockage									
Right turn flare (veh)									
Median type	None		None						
Median storage (veh)									
Upstream signal (m)									
pX, platoon unblocked									
vC, conflicting volume	527				792		259		
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
vCu, unblocked vol	527				792		259		
tC, single (s)	4.2				6.8		6.9		
tC, 2 stage (s)									
tF (s)	2.2				3.5		3.3		
p0 queue free %	97				98		98		
cM capacity (veh/h)	1029				320		746		
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	SB 2	
Volume Total	34	206	206	259	259	9	7	17	
Volume Left	34	0	0	0	0	0	7	0	
Volume Right	0	0	0	0	0	9	0	17	
cSH	1029	1700	1700	1700	1700	1700	320	746	
Volume to Capacity	0.03	0.12	0.12	0.15	0.15	0.01	0.02	0.02	
Queue Length 95th (m)	0.8	0.0	0.0	0.0	0.0	0.0	0.5	0.6	
Control Delay (s)	8.6	0.0	0.0	0.0	0.0	0.0	16.5	9.9	
Lane LOS	A						C	A	
Approach Delay (s)	0.7		0.0				11.9		
Approach LOS							B		
Intersection Summary									
Average Delay			0.6						
Intersection Capacity Utilization			30.1%		ICU Level of Service			A	
Analysis Period (min)			15						

# HCM Unsignalized Intersection Capacity Analysis

## 9: Sixth Line South & Steeles Avenue

Base Year - SAT Peak Hour  
Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↑↑	↑	↓	↑↑	↓	↓	
Traffic Volume (veh/h)	393	0	0	495	0	0	
Future Volume (Veh/h)	393	0	0	495	0	0	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Hourly flow rate (vph)	423	0	0	532	0	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage (veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume			423			689	212
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			423			689	212
tC, single (s)			4.1			6.8	6.9
tC, 2 stage (s)							
tF (s)			2.2			3.5	3.3
p0 queue free %			100			100	100
cM capacity (veh/h)			1147			384	800
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1
Volume Total	212	212	0	0	266	266	0
Volume Left	0	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0	0
cSH	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.12	0.12	0.00	0.00	0.16	0.16	0.00
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS							A
Approach Delay (s)	0.0			0.0		0.0	
Approach LOS							A
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization			17.0%		ICU Level of Service		A
Analysis Period (min)			15				

HCM Unsignalized Intersection Capacity Analysis  
 10: Steeles Avenue & Hornby Road

Base Year - SAT Peak Hour  
 Premier Gateway



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	31	362	423	9	7	72
Future Volume (Veh/h)	31	362	423	9	7	72
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	33	389	455	10	8	77
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	465				716	228
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	465				716	228
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	97				98	90
cM capacity (veh/h)	1107				358	778

Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	SB 2
Volume Total	33	194	194	228	228	10	8	77
Volume Left	33	0	0	0	0	0	8	0
Volume Right	0	0	0	0	0	10	0	77
cSH	1107	1700	1700	1700	1700	1700	358	778
Volume to Capacity	0.03	0.11	0.11	0.13	0.13	0.01	0.02	0.10
Queue Length 95th (m)	0.7	0.0	0.0	0.0	0.0	0.0	0.5	2.6
Control Delay (s)	8.4	0.0	0.0	0.0	0.0	0.0	15.3	10.1
Lane LOS	A						C	B
Approach Delay (s)	0.7			0.0			10.6	
Approach LOS							B	

Intersection Summary		
Average Delay		1.2
Intersection Capacity Utilization	28.4%	ICU Level of Service
Analysis Period (min)	15	A

HCM Unsignalized Intersection Capacity Analysis  
 11: Trafalgar Rd & Hornby Rd

Base Year - SAT Peak Hour  
 Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	39	1	2	584	519	77
Future Volume (Veh/h)	39	1	2	584	519	77
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	40	1	2	602	535	79
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	880	307	535			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	880	307	535			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	86	100	100			
cM capacity (veh/h)	286	695	1043			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	41	203	401	357	257	
Volume Left	40	2	0	0	0	
Volume Right	1	0	0	0	79	
cSH	290	1043	1700	1700	1700	
Volume to Capacity	0.14	0.00	0.24	0.21	0.15	
Queue Length 95th (m)	3.9	0.0	0.0	0.0	0.0	
Control Delay (s)	19.4	0.1	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	19.4	0.0		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization			27.5%	ICU Level of Service		A
Analysis Period (min)			15			

Queues  
12: Trafalgar Road & Steeles Avenue

Base Year - SAT Peak Hour  
Premier Gateway



























Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	36	252	96	654	348	56	91	518	935	72	469
v/c Ratio	0.16	0.48	0.27	1.02	0.31	0.10	0.39	0.36	0.92	0.16	0.36
Control Delay	26.2	51.4	2.5	89.8	33.1	0.3	58.6	26.8	26.7	15.7	29.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.2	51.4	2.5	89.8	33.1	0.3	58.6	26.8	26.7	15.7	29.7
Queue Length 50th (m)	5.6	31.6	0.0	-90.1	37.1	0.0	11.7	49.1	100.0	8.8	45.6
Queue Length 95th (m)	12.8	45.8	1.6	#130.3	51.2	0.0	20.2	66.1	#213.8	16.8	62.9
Internal Link Dist (m)		443.0			287.3			749.5			265.5
Turn Bay Length (m)	115.0		40.0	130.0		70.0	100.0		65.0		
Base Capacity (vph)	225	858	476	644	1375	683	235	1457	1021	448	1287
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.29	0.20	1.02	0.25	0.08	0.39	0.36	0.92	0.16	0.36

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
12: Trafalgar Road & Steeles Avenue

Base Year - SAT Peak Hour  
Premier Gateway

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	35	242	92	628	334	54	87	497	898	69	440	11
Future Volume (vph)	35	242	92	628	334	54	87	497	898	69	440	11
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	7.0	7.0	5.0	7.0	7.0	5.0	8.0	8.0	4.0	8.0	
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.95	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1399	3252	1369	3502	3438	1538	2824	3505	1599	1736	3456	
Flt Permitted	0.54	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.45	1.00	
Satd. Flow (perm)	800	3252	1369	3502	3438	1538	2824	3505	1599	825	3456	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	36	252	96	654	348	56	91	518	935	72	458	11
RTOR Reduction (vph)	0	0	79	0	0	37	0	0	369	0	1	0
Lane Group Flow (vph)	36	252	17	654	348	19	91	518	566	72	468	0
Heavy Vehicles (%)	29%	11%	18%	0%	5%	5%	24%	3%	1%	4%	4%	8%
Turn Type	pm+pt	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4			8			2	6		
Actuated Green, G (s)	25.8	21.6	21.6	23.0	41.4	41.4	10.4	49.6	49.6	51.8	45.0	
Effective Green, g (s)	25.8	21.6	21.6	23.0	41.4	41.4	10.4	49.6	49.6	51.8	45.0	
Actuated g/C Ratio	0.21	0.17	0.17	0.18	0.33	0.33	0.08	0.40	0.40	0.41	0.36	
Clearance Time (s)	4.0	7.0	7.0	5.0	7.0	7.0	5.0	8.0	8.0	4.0	8.0	
Vehicle Extension (s)	3.0	3.0	3.0	4.0	3.0	3.0	4.0	0.2	0.2	3.0	0.2	
Lane Grp Cap (vph)	185	561	236	644	1138	509	234	1390	634	391	1244	
v/s Ratio Prot	0.01	c0.08		c0.19	0.10		c0.03	0.15		0.01	0.14	
v/s Ratio Perm	0.03		0.01			0.01			c0.35	0.07		
v/c Ratio	0.19	0.45	0.07	1.02	0.31	0.04	0.39	0.37	0.89	0.18	0.38	
Uniform Delay, d1	40.4	46.4	43.3	51.0	31.1	28.3	54.3	26.7	35.2	22.4	29.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.5	0.6	0.1	39.4	0.2	0.0	1.5	0.8	17.5	0.2	0.9	
Delay (s)	40.9	46.9	43.4	90.4	31.3	28.3	55.7	27.5	52.7	22.6	30.5	
Level of Service	D	D	D	F	C	C	E	C	D	C	C	
Approach Delay (s)		45.5			67.7			44.4			29.4	
Approach LOS		D			E			D			C	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			49.2								HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio			0.81									
Actuated Cycle Length (s)			125.0								Sum of lost time (s)	25.0
Intersection Capacity Utilization			93.9%								ICU Level of Service	F
Analysis Period (min)			15									
c Critical Lane Group												

Queues  
13: Toronto Premier Outlets & Steeles Avenue

Base Year - SAT Peak Hour  
Premier Gateway



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	625	634	42	648	410	52
v/c Ratio	0.38	0.58	0.10	0.34	0.34	0.09
Control Delay	18.7	3.9	10.5	13.6	25.7	7.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.7	3.9	10.5	13.6	25.7	7.0
Queue Length 50th (m)	45.0	0.0	3.6	37.2	31.9	0.0
Queue Length 95th (m)	61.4	20.6	8.5	49.2	44.7	8.2
Internal Link Dist (m)	287.3			176.7	95.1	
Turn Bay Length (m)		130.0	45.0			40.0
Base Capacity (vph)	1643	1098	485	1892	1190	583
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.38	0.58	0.09	0.34	0.34	0.09

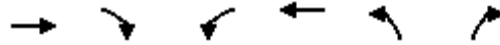
Intersection Summary



HCM Signalized Intersection Capacity Analysis  
13: Toronto Premier Outlets & Steeles Avenue

Base Year - SAT Peak Hour

Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↑	↑↑	↑↑	↑
Traffic Volume (vph)	600	609	40	622	394	50
Future Volume (vph)	600	609	40	622	394	50
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	4.0	6.0	6.0	6.0
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3471	1615	1805	3505	3502	1615
Flt Permitted	1.00	1.00	0.34	1.00	0.95	1.00
Satd. Flow (perm)	3471	1615	638	3505	3502	1615
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	625	634	42	648	410	52
RTOR Reduction (vph)	0	334	0	0	0	35
Lane Group Flow (vph)	625	300	42	648	410	17
Heavy Vehicles (%)	4%	0%	0%	3%	0%	0%
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4		3	8	2	
Permitted Phases		4	8			2
Actuated Green, G (s)	47.3	47.3	55.6	55.6	32.4	32.4
Effective Green, g (s)	47.3	47.3	55.6	55.6	32.4	32.4
Actuated g/C Ratio	0.47	0.47	0.56	0.56	0.32	0.32
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0
Vehicle Extension (s)	0.2	0.2	3.0	0.2	4.0	4.0
Lane Grp Cap (vph)	1641	763	404	1948	1134	523
v/s Ratio Prot	0.18		0.00	c0.18	c0.12	
v/s Ratio Perm		c0.19	0.05			0.01
v/c Ratio	0.38	0.39	0.10	0.33	0.36	0.03
Uniform Delay, d1	16.9	17.1	10.6	12.1	25.9	23.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.7	1.5	0.1	0.5	0.9	0.1
Delay (s)	17.6	18.6	10.7	12.6	26.8	23.2
Level of Service	B	B	B	B	C	C
Approach Delay (s)	18.1			12.4	26.4	
Approach LOS	B			B	C	

Intersection Summary

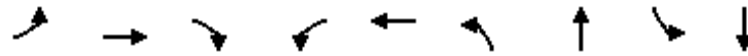
HCM 2000 Control Delay	18.1	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.38		
Actuated Cycle Length (s)	100.0	Sum of lost time (s)	16.0
Intersection Capacity Utilization	51.0%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

## Queues

Base Year - SAT Peak Hour

## 14: Toronto Premium Outlets/Eighth Line &amp; Steeles Avenue

Premier Gateway



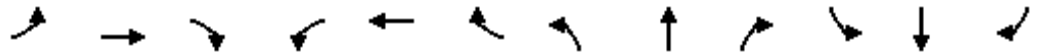
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	131	504	29	161	437	167	214	17	118
v/c Ratio	0.27	0.40	0.04	0.34	0.33	0.45	0.29	0.07	0.28
Control Delay	15.8	27.8	0.1	16.6	25.9	51.9	6.3	38.2	14.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	15.8	27.8	0.1	16.6	25.9	51.9	6.3	38.2	14.4
Queue Length 50th (m)	15.0	44.5	0.0	18.7	36.8	19.1	4.4	3.2	5.2
Queue Length 95th (m)	27.3	65.3	0.0	32.6	54.4	31.0	20.5	10.1	21.9
Internal Link Dist (m)		176.7			846.8		194.1		472.6
Turn Bay Length (m)	105.0		55.0	30.0				20.0	
Base Capacity (vph)	535	1273	712	557	1341	496	796	252	428
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.40	0.04	0.29	0.33	0.34	0.27	0.07	0.28

## Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 14: Toronto Premium Outlets/Eighth Line & Steeles Avenue

Base Year - SAT Peak Hour

Premier Gateway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	128	494	28	158	410	19	164	29	180	17	27	88
Future Volume (vph)	128	494	28	158	410	19	164	29	180	17	27	88
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	6.0	6.0	4.0	6.0		7.0	7.0		7.0	7.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.97	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.99		1.00	0.87		1.00	0.89	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1770	3406	1615	1805	3519		3502	1655		1805	1683	
Flt Permitted	0.46	1.00	1.00	0.39	1.00		0.95	1.00		0.62	1.00	
Satd. Flow (perm)	853	3406	1615	743	3519		3502	1655		1186	1683	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	131	504	29	161	418	19	167	30	184	17	28	90
RTOR Reduction (vph)	0	0	18	0	2	0	0	114	0	0	71	0
Lane Group Flow (vph)	131	504	11	161	435	0	167	100	0	17	47	0
Heavy Vehicles (%)	2%	6%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Turn Type	pm+pt	NA	Perm	pm+pt	NA		Prot	NA		Perm	NA	
Protected Phases	7	4		3	8		5	2			6	
Permitted Phases	4		4	8						6		
Actuated Green, G (s)	52.3	42.3	42.3	53.9	43.1		11.9	42.9		24.0	24.0	
Effective Green, g (s)	52.3	42.3	42.3	53.9	43.1		11.9	42.9		24.0	24.0	
Actuated g/C Ratio	0.46	0.37	0.37	0.48	0.38		0.11	0.38		0.21	0.21	
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		7.0	7.0		7.0	7.0	
Vehicle Extension (s)	3.0	0.2	0.2	3.0	0.2		4.0	4.0		3.0	3.0	
Lane Grp Cap (vph)	475	1274	604	455	1342		368	628		251	357	
v/s Ratio Prot	0.02	c0.15		c0.03	0.12		c0.05	c0.06			0.03	
v/s Ratio Perm	0.10		0.01	0.13						0.01		
v/c Ratio	0.28	0.40	0.02	0.35	0.32		0.45	0.16		0.07	0.13	
Uniform Delay, d1	17.7	26.0	22.3	17.2	24.7		47.5	23.1		35.6	36.1	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.3	0.9	0.1	0.5	0.6		1.2	0.2		0.5	0.8	
Delay (s)	18.0	26.9	22.3	17.7	25.3		48.7	23.3		36.1	36.8	
Level of Service	B	C	C	B	C		D	C		D	D	
Approach Delay (s)		24.9			23.3			34.4			36.7	
Approach LOS		C			C			C			D	

Intersection Summary		
HCM 2000 Control Delay	27.3	HCM 2000 Level of Service C
HCM 2000 Volume to Capacity ratio	0.34	
Actuated Cycle Length (s)	113.0	Sum of lost time (s) 24.0
Intersection Capacity Utilization	53.7%	ICU Level of Service A
Analysis Period (min)	15	
c Critical Lane Group		

HCM Unsignalized Intersection Capacity Analysis  
 15: Eighth Line South & Steeles Avenue

Base Year - SAT Peak Hour  
 Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↑↑		↵	↑↑	↵	↵	
Traffic Volume (veh/h)	689	2	1	585	2	0	
Future Volume (Veh/h)	689	2	1	585	2	0	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Hourly flow rate (vph)	741	2	1	629	2	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None		None				
Median storage (veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume			743		1058	372	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			743		1058	372	
tC, single (s)			4.1		6.8	6.9	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		99	100	
cM capacity (veh/h)			873		223	632	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	WB 3	NB 1	NB 2
Volume Total	494	249	1	314	314	2	0
Volume Left	0	0	1	0	0	2	0
Volume Right	0	2	0	0	0	0	0
cSH	1700	1700	873	1700	1700	223	1700
Volume to Capacity	0.29	0.15	0.00	0.18	0.18	0.01	0.00
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.2	0.0
Control Delay (s)	0.0	0.0	9.1	0.0	0.0	21.3	0.0
Lane LOS			A			C	A
Approach Delay (s)	0.0		0.0			21.3	
Approach LOS						C	
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization			29.1%	ICU Level of Service		A	
Analysis Period (min)			15				

Queues  
16: Steeles Avenue & Ninth Line

Base Year - SAT Peak Hour  
Premier Gateway



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	95	629	525	284	296	85
v/c Ratio	0.23	0.39	0.41	0.37	0.42	0.13
Control Delay	14.7	18.0	25.8	4.6	24.6	4.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	14.7	18.0	25.8	4.6	24.6	4.9
Queue Length 50th (m)	9.9	42.2	42.8	0.0	43.3	0.0
Queue Length 95th (m)	18.7	56.1	60.5	18.0	66.7	9.4
Internal Link Dist (m)		501.4	674.5		3096.2	
Turn Bay Length (m)	65.0			75.0	90.0	
Base Capacity (vph)	442	1631	1294	765	703	675
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.21	0.39	0.41	0.37	0.42	0.13

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 16: Steeles Avenue & Ninth Line

Base Year - SAT Peak Hour  
 Premier Gateway



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	91	604	504	273	284	82
Future Volume (vph)	91	604	504	273	284	82
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	7.0	7.0	7.0	7.0	7.0
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00
Frt	1.00	1.00	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1719	3471	3539	1599	1805	1599
Flt Permitted	0.36	1.00	1.00	1.00	0.95	1.00
Satd. Flow (perm)	651	3471	3539	1599	1805	1599
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	95	629	525	284	296	85
RTOR Reduction (vph)	0	0	0	181	0	52
Lane Group Flow (vph)	95	629	525	103	296	33
Heavy Vehicles (%)	5%	4%	2%	1%	0%	1%
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	7	4	8		6	
Permitted Phases	4			8		6
Actuated Green, G (s)	47.8	47.8	36.6	36.6	39.0	39.0
Effective Green, g (s)	47.8	47.8	36.6	36.6	39.0	39.0
Actuated g/C Ratio	0.47	0.47	0.36	0.36	0.39	0.39
Clearance Time (s)	4.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	0.2	0.2	0.2	3.0	3.0
Lane Grp Cap (vph)	384	1645	1284	580	698	618
v/s Ratio Prot	0.02	c0.18	c0.15		c0.16	
v/s Ratio Perm	0.10			0.06		0.02
v/c Ratio	0.25	0.38	0.41	0.18	0.42	0.05
Uniform Delay, d1	15.2	17.0	24.0	21.9	22.7	19.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.3	0.7	1.0	0.7	1.9	0.2
Delay (s)	15.5	17.7	25.0	22.5	24.5	19.5
Level of Service	B	B	C	C	C	B
Approach Delay (s)		17.4	24.1		23.4	
Approach LOS		B	C		C	

Intersection Summary

HCM 2000 Control Delay	21.4	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.42		
Actuated Cycle Length (s)	100.8	Sum of lost time (s)	18.0
Intersection Capacity Utilization	53.2%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
17: Ninth Line (South) & Steeles Avenue

Base Year - SAT Peak Hour  
Premier Gateway



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	756	199	100	639	197	144
v/c Ratio	0.70	0.31	0.31	0.42	0.29	0.20
Control Delay	27.7	4.7	13.0	15.4	20.1	4.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.7	4.7	13.0	15.4	20.1	4.7
Queue Length 50th (m)	55.0	0.0	8.0	33.7	20.7	0.0
Queue Length 95th (m)	76.4	13.9	15.6	45.7	44.1	12.4
Internal Link Dist (m)	674.5			176.7	143.5	
Turn Bay Length (m)		75.0	145.0		60.0	
Base Capacity (vph)	1990	1003	365	2663	684	707
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.38	0.20	0.27	0.24	0.29	0.20

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 17: Ninth Line (South) & Steeles Avenue

Base Year - SAT Peak Hour  
 Premier Gateway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑	↓	↑↑	↓	↑
Traffic Volume (vph)	703	185	93	594	183	134
Future Volume (vph)	703	185	93	594	183	134
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.0	7.0	4.0	7.0	7.0	7.0
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3505	1615	1787	3539	1787	1615
Flt Permitted	1.00	1.00	0.20	1.00	0.95	1.00
Satd. Flow (perm)	3505	1615	382	3539	1787	1615
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	756	199	100	639	197	144
RTOR Reduction (vph)	0	138	0	0	0	89
Lane Group Flow (vph)	756	61	100	639	197	55
Heavy Vehicles (%)	3%	0%	1%	2%	1%	0%
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4		3	8	2	
Permitted Phases		4	8			2
Actuated Green, G (s)	23.7	23.7	34.1	34.1	29.4	29.4
Effective Green, g (s)	23.7	23.7	34.1	34.1	29.4	29.4
Actuated g/C Ratio	0.31	0.31	0.44	0.44	0.38	0.38
Clearance Time (s)	7.0	7.0	4.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1071	493	284	1557	677	612
v/s Ratio Prot	c0.22		0.03	c0.18	c0.11	
v/s Ratio Perm		0.04	0.13			0.03
v/c Ratio	0.71	0.12	0.35	0.41	0.29	0.09
Uniform Delay, d1	23.8	19.4	14.0	14.8	16.8	15.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	2.1	0.1	0.8	0.2	1.1	0.3
Delay (s)	26.0	19.5	14.8	15.0	17.9	15.7
Level of Service	C	B	B	B	B	B
Approach Delay (s)	24.6			15.0	17.0	
Approach LOS	C			B	B	

Intersection Summary

HCM 2000 Control Delay	19.8	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.48		
Actuated Cycle Length (s)	77.5	Sum of lost time (s)	18.0
Intersection Capacity Utilization	50.4%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			



Queues

Base Year - SAT Peak Hour

18: James Snow Parkway & Hwy 401 (Westbound Ramp)

Premier Gateway



Lane Group	WBL	WBR	NBT	SBT
Lane Group Flow (vph)	655	240	430	704
v/c Ratio	0.69	0.43	0.27	0.43
Control Delay	25.5	5.5	12.2	13.6
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	25.5	5.5	12.2	13.6
Queue Length 50th (m)	38.5	0.0	17.0	30.4
Queue Length 95th (m)	54.4	15.7	30.7	51.6
Internal Link Dist (m)	390.4		415.8	504.8
Turn Bay Length (m)				
Base Capacity (vph)	1419	711	1599	1646
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.46	0.34	0.27	0.43

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 18: James Snow Parkway & Hwy 401 (Westbound Ramp)

Base Year - SAT Peak Hour

Premier Gateway



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↙↙	↗	↕↕			↕↕
Traffic Volume (vph)	609	259	417	0	0	683
Future Volume (vph)	609	259	417	0	0	683
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.2	8.2	9.3			9.3
Lane Util. Factor	0.97	0.91	0.95			0.95
Frt	0.99	0.85	1.00			1.00
Flt Protected	0.95	1.00	1.00			1.00
Satd. Flow (prot)	3488	1400	3438			3539
Flt Permitted	0.95	1.00	1.00			1.00
Satd. Flow (perm)	3488	1400	3438			3539
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	628	267	430	0	0	704
RTOR Reduction (vph)	4	175	0	0	0	0
Lane Group Flow (vph)	651	65	430	0	0	704
Heavy Vehicles (%)	0%	5%	5%	0%	0%	2%
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Actuated Green, G (s)	17.9	17.9	30.8			30.8
Effective Green, g (s)	17.9	17.9	30.8			30.8
Actuated g/C Ratio	0.27	0.27	0.47			0.47
Clearance Time (s)	8.2	8.2	9.3			9.3
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	943	378	1599			1646
v/s Ratio Prot	c0.19		0.13			c0.20
v/s Ratio Perm		0.05				
v/c Ratio	0.69	0.17	0.27			0.43
Uniform Delay, d1	21.7	18.5	10.8			11.8
Progression Factor	1.00	1.00	1.00			1.00
Incremental Delay, d2	2.1	0.2	0.4			0.8
Delay (s)	23.8	18.7	11.2			12.6
Level of Service	C	B	B			B
Approach Delay (s)	22.4		11.2			12.6
Approach LOS	C		B			B

Intersection Summary			
HCM 2000 Control Delay	16.7	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.52		
Actuated Cycle Length (s)	66.2	Sum of lost time (s)	17.5
Intersection Capacity Utilization	53.5%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

Queues

Base Year - SAT Peak Hour

19: James Snow Parkway & Hwy 401 (Eastbound Ramp)

Premier Gateway



Lane Group	EBL	EBR	NBT	SBT
Lane Group Flow (vph)	123	58	480	1095
v/c Ratio	0.27	0.23	0.18	0.41
Control Delay	17.7	10.9	4.1	5.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	17.7	10.9	4.1	5.3
Queue Length 50th (m)	4.2	0.0	11.3	32.2
Queue Length 95th (m)	11.3	10.4	16.5	43.4
Internal Link Dist (m)	305.5		1282.4	415.8
Turn Bay Length (m)				
Base Capacity (vph)	626	330	2633	2659
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.20	0.18	0.18	0.41

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 19: James Snow Parkway & Hwy 401 (Eastbound Ramp)

Base Year - SAT Peak Hour

Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	64	115	0	475	1084	0
Future Volume (vph)	64	115	0	475	1084	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0		7.4	7.4	
Lane Util. Factor	0.97	0.91		0.95	0.95	
Frt	0.93	0.85		1.00	1.00	
Flt Protected	0.97	1.00		1.00	1.00	
Satd. Flow (prot)	3004	1470		3539	3574	
Flt Permitted	0.97	1.00		1.00	1.00	
Satd. Flow (perm)	3004	1470		3539	3574	
Peak-hour factor, PHF	0.99	0.99	0.99	0.99	0.99	0.99
Adj. Flow (vph)	65	116	0	480	1095	0
RTOR Reduction (vph)	52	52	0	0	0	0
Lane Group Flow (vph)	71	6	0	480	1095	0
Heavy Vehicles (%)	21%	0%	0%	2%	1%	0%
Turn Type	Prot	Perm		NA	NA	
Protected Phases	4			2	6	
Permitted Phases		4				
Actuated Green, G (s)	7.9	7.9		52.5	52.5	
Effective Green, g (s)	7.9	7.9		52.5	52.5	
Actuated g/C Ratio	0.11	0.11		0.71	0.71	
Clearance Time (s)	6.0	6.0		7.4	7.4	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	321	157		2517	2542	
v/s Ratio Prot	c0.02			0.14	c0.31	
v/s Ratio Perm		0.00				
v/c Ratio	0.22	0.04		0.19	0.43	
Uniform Delay, d1	30.1	29.5		3.6	4.4	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.4	0.1		0.2	0.5	
Delay (s)	30.5	29.7		3.7	5.0	
Level of Service	C	C		A	A	
Approach Delay (s)	30.2			3.7	5.0	
Approach LOS	C			A	A	

Intersection Summary

HCM 2000 Control Delay	7.2	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.40		
Actuated Cycle Length (s)	73.8	Sum of lost time (s)	13.4
Intersection Capacity Utilization	53.5%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

Queues

Base Year - SAT Peak Hour

20: Trafalgar Road & Hwy 401 (Westbound Ramp)

Premier Gateway



Lane Group	WBL	WBR	NBT	SBT
Lane Group Flow (vph)	439	301	910	1166
v/c Ratio	0.62	0.80	0.37	0.48
Control Delay	29.0	37.4	7.9	9.0
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	29.0	37.4	7.9	9.0
Queue Length 50th (m)	29.2	36.8	34.3	48.8
Queue Length 95th (m)	44.4	70.8	65.2	91.7
Internal Link Dist (m)	383.1		312.7	749.5
Turn Bay Length (m)				
Base Capacity (vph)	1407	679	2430	2407
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.31	0.44	0.37	0.48

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 20: Trafalgar Road & Hwy 401 (Westbound Ramp)

Base Year - SAT Peak Hour  
 Premier Gateway



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔↔	↗	↕↕			↕↕
Traffic Volume (vph)	135	590	892	0	0	1143
Future Volume (vph)	135	590	892	0	0	1143
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	6.0			6.0
Lane Util. Factor	0.97	0.91	0.95			0.95
Frt	0.90	0.85	1.00			1.00
Flt Protected	0.98	1.00	1.00			1.00
Satd. Flow (prot)	3172	1441	3539			3505
Flt Permitted	0.98	1.00	1.00			1.00
Satd. Flow (perm)	3172	1441	3539			3505
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	138	602	910	0	0	1166
RTOR Reduction (vph)	103	103	0	0	0	0
Lane Group Flow (vph)	336	198	910	0	0	1166
Heavy Vehicles (%)	4%	2%	2%	0%	0%	3%
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Actuated Green, G (s)	18.6	18.6	67.3			67.3
Effective Green, g (s)	18.6	18.6	67.3			67.3
Actuated g/C Ratio	0.19	0.19	0.69			0.69
Clearance Time (s)	6.0	6.0	6.0			6.0
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	602	273	2432			2409
v/s Ratio Prot	0.11		0.26			c0.33
v/s Ratio Perm		c0.14				
v/c Ratio	0.56	0.73	0.37			0.48
Uniform Delay, d1	35.9	37.3	6.4			7.2
Progression Factor	1.00	1.00	1.00			1.00
Incremental Delay, d2	1.1	9.2	0.4			0.7
Delay (s)	37.1	46.5	6.9			7.9
Level of Service	D	D	A			A
Approach Delay (s)	40.9		6.9			7.9
Approach LOS	D		A			A

Intersection Summary			
HCM 2000 Control Delay	16.2	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.54		
Actuated Cycle Length (s)	97.9	Sum of lost time (s)	12.0
Intersection Capacity Utilization	59.0%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

Queues  
 21: Trafalgar Road & Hwy 401 (Eastbound Ramp)

Base Year - SAT Peak Hour  
 Premier Gateway



Lane Group	EBL	EBR	NBT	SBT
Lane Group Flow (vph)	348	160	1355	621
v/c Ratio	0.69	0.47	0.51	0.23
Control Delay	48.4	11.4	6.4	4.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	48.4	11.4	6.4	4.4
Queue Length 50th (m)	35.6	0.0	53.7	18.2
Queue Length 95th (m)	51.1	20.4	81.7	29.4
Internal Link Dist (m)	204.3		1138.2	312.7
Turn Bay Length (m)				
Base Capacity (vph)	779	442	2633	2684
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.45	0.36	0.51	0.23
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
 21: Trafalgar Road & Hwy 401 (Eastbound Ramp)

Base Year - SAT Peak Hour

Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	244	224	0	1247	571	0
Future Volume (vph)	244	224	0	1247	571	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		6.0	6.0	
Lane Util. Factor	0.97	0.91		0.95	0.95	
Frt	0.96	0.85		1.00	1.00	
Flt Protected	0.96	1.00		1.00	1.00	
Satd. Flow (prot)	3341	1413		3471	3539	
Flt Permitted	0.96	1.00		1.00	1.00	
Satd. Flow (perm)	3341	1413		3471	3539	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	265	243	0	1355	621	0
RTOR Reduction (vph)	28	137	0	0	0	0
Lane Group Flow (vph)	320	23	0	1355	621	0
Heavy Vehicles (%)	2%	4%	0%	4%	2%	0%
Turn Type	Prot	Perm		NA	NA	
Protected Phases	4			2	6	
Permitted Phases		4				
Actuated Green, G (s)	15.7	15.7		84.1	84.1	
Effective Green, g (s)	15.7	15.7		84.1	84.1	
Actuated g/C Ratio	0.14	0.14		0.76	0.76	
Clearance Time (s)	5.0	5.0		6.0	6.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	473	200		2634	2686	
v/s Ratio Prot	c0.10			c0.39	0.18	
v/s Ratio Perm		0.02				
v/c Ratio	0.68	0.11		0.51	0.23	
Uniform Delay, d1	45.1	41.5		5.3	3.9	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	3.8	0.3		0.7	0.2	
Delay (s)	48.9	41.7		6.0	4.1	
Level of Service	D	D		A	A	
Approach Delay (s)	46.7			6.0	4.1	
Approach LOS	D			A	A	

Intersection Summary			
HCM 2000 Control Delay	13.8	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.54		
Actuated Cycle Length (s)	110.8	Sum of lost time (s)	11.0
Intersection Capacity Utilization	59.0%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			



Queues

Base Year - SAT Peak Hour

22: Winston Churchill Boulevard & Hwy 401 (Westbound Ramp)

Premier Gateway



Lane Group	WBL	WBR	NBT	SBT
Lane Group Flow (vph)	681	250	668	814
v/c Ratio	0.83	0.49	0.30	0.37
Control Delay	50.2	7.7	10.8	11.5
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	50.2	7.7	10.8	11.5
Queue Length 50th (m)	78.6	0.0	36.5	47.0
Queue Length 95th (m)	100.5	22.8	51.6	65.3
Internal Link Dist (m)	284.7		32.1	320.2
Turn Bay Length (m)				
Base Capacity (vph)	974	558	2216	2195
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.70	0.45	0.30	0.37

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 22: Winston Churchill Boulevard & Hwy 401 (Westbound Ramp)

Base Year - SAT Peak Hour  
 Premier Gateway



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WT	WT	WT			WT
Traffic Volume (vph)	633	270	648	0	0	790
Future Volume (vph)	633	270	648	0	0	790
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.0	7.0	8.0			8.0
Lane Util. Factor	0.97	0.91	0.95			0.95
Frt	0.99	0.85	1.00			1.00
Flt Protected	0.95	1.00	1.00			1.00
Satd. Flow (prot)	3383	1324	3539			3505
Flt Permitted	0.95	1.00	1.00			1.00
Satd. Flow (perm)	3383	1324	3539			3505
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	653	278	668	0	0	814
RTOR Reduction (vph)	3	189	0	0	0	0
Lane Group Flow (vph)	678	61	668	0	0	814
Heavy Vehicles (%)	3%	11%	2%	0%	0%	3%
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Actuated Green, G (s)	28.0	28.0	72.1			72.1
Effective Green, g (s)	28.0	28.0	72.1			72.1
Actuated g/C Ratio	0.24	0.24	0.63			0.63
Clearance Time (s)	7.0	7.0	8.0			8.0
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	822	322	2216			2195
v/s Ratio Prot	c0.20		0.19			c0.23
v/s Ratio Perm		0.05				
v/c Ratio	0.82	0.19	0.30			0.37
Uniform Delay, d1	41.2	34.5	9.9			10.5
Progression Factor	1.00	1.00	1.00			1.00
Incremental Delay, d2	6.8	0.3	0.4			0.5
Delay (s)	48.0	34.8	10.3			10.9
Level of Service	D	C	B			B
Approach Delay (s)	44.5		10.3			10.9
Approach LOS	D		B			B

**Intersection Summary**

HCM 2000 Control Delay	23.7	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.50		
Actuated Cycle Length (s)	115.1	Sum of lost time (s)	15.0
Intersection Capacity Utilization	157.8%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

Queues

Base Year - SAT Peak Hour

23: Winston Churchill Boulevard & Hwy 401 (Eastbound Ramp)

Premier Gateway



Lane Group	EBL	EBR	NBT	SBT
Lane Group Flow (vph)	330	157	787	1073
v/c Ratio	0.69	0.54	0.29	0.40
Control Delay	36.8	17.1	5.0	5.7
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	36.8	17.1	5.0	5.7
Queue Length 50th (m)	23.0	3.6	25.4	38.8
Queue Length 95th (m)	38.4	25.6	39.7	58.9
Internal Link Dist (m)	152.5		433.2	198.3
Turn Bay Length (m)				
Base Capacity (vph)	740	402	2682	2682
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.45	0.39	0.29	0.40

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 23: Winston Churchill Boulevar & Hwy 401 (Eastbound Ramp)

Base Year - SAT Peak Hour  
 Premier Gateway



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔↔	↔		↑↑	↑↑	
Traffic Volume (vph)	193	284	0	771	1052	0
Future Volume (vph)	193	284	0	771	1052	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.0	8.0		7.0	7.0	
Lane Util. Factor	0.97	0.91		0.95	0.95	
Frt	0.94	0.85		1.00	1.00	
Flt Protected	0.97	1.00		1.00	1.00	
Satd. Flow (prot)	3215	1455		3574	3574	
Flt Permitted	0.97	1.00		1.00	1.00	
Satd. Flow (perm)	3215	1455		3574	3574	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	197	290	0	787	1073	0
RTOR Reduction (vph)	111	125	0	0	0	0
Lane Group Flow (vph)	219	32	0	787	1073	0
Heavy Vehicles (%)	7%	1%	0%	1%	1%	0%
Turn Type	Prot	Perm		NA	NA	
Protected Phases	4			2	6	
Permitted Phases		4				
Actuated Green, G (s)	12.6	12.6		83.1	83.1	
Effective Green, g (s)	12.6	12.6		83.1	83.1	
Actuated g/C Ratio	0.11	0.11		0.75	0.75	
Clearance Time (s)	8.0	8.0		7.0	7.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	365	165		2682	2682	
v/s Ratio Prot	c0.07			0.22	c0.30	
v/s Ratio Perm		0.02				
v/c Ratio	0.60	0.19		0.29	0.40	
Uniform Delay, d1	46.7	44.4		4.4	4.9	
Progression Factor	1.00	1.00		1.00	1.00	
Incremental Delay, d2	2.8	0.6		0.3	0.4	
Delay (s)	49.4	45.0		4.7	5.4	
Level of Service	D	D		A	A	
Approach Delay (s)	48.0			4.7	5.4	
Approach LOS	D			A	A	

Intersection Summary

HCM 2000 Control Delay	14.0	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.43		
Actuated Cycle Length (s)	110.7	Sum of lost time (s)	15.0
Intersection Capacity Utilization	157.8%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			