



URBANTECH[®]

**FUNCTIONAL SERVICING REPORT &
STORMWATER MANAGEMENT STUDY**

130 Mountainview Road North

Town of Halton Hills

REGION OF HALTON

PREPARED FOR
**Whitestone Georgetown Developments
General Partnership**

Urbantech File No.: 23-763

1st Submission - November 2024

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Appendix A Site Statistics and Site Plan (Arcadis Architects (Canada) Inc.)

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1. INTRODUCTION

This report provides functional servicing design and stormwater management information in support of Official Plan Amendment (OPA), Zoning Bylaw Amendment (ZBA) and Draft Plan of Subdivision (DPOS) for the 130 Mountainview Road North property in the Town of Halton Hills (Town), Region of Halton (Region). When approved, the OPA/ZBA will establish a planning framework for the property, including a multi-block mixed use development, new public road and privately owned public park space.

The servicing and development concepts presented in this report are based on the site plan prepared by Arcadis Architects (Canada) Inc. (architectural plan and site statistics are available in **Appendix A**), with input from the following reference reports:

- Preliminary Geotechnical Assessment (BlueFrog Environmental Consulting Inc., November 2022)
- Town of Halton Hills Pre-Consultation / Development Review Committee Meeting Notes (Town of Halton Hills, March 2022)
- Town of Halton Hills GO Station Secondary Plan and Land Use Schedule (Town of Halton Hills, 2024 Consolidation)
- Sustainable Halton Water and Wastewater Master Plan (AECOM, 2011)
- Region of Halton 2022 Development Charges Update Water/Wastewater Technical Report (GM BluePlan, September 2021)
- Province of Ontario Provincial Planning Statement (October 2024)
- Municipal Class Environmental Assessment Addendum John Street Wastewater Pumping Station Upgrades (Associated Engineering, March 2024)
- Functional Servicing and Stormwater Management Report 1 Rosetta Street (Arcadis Professional Services (Canada) Inc., 2023)

Design information considers the following:

- Regional Municipality of Halton Water and Wastewater Linear Design Manual (2019)
- Town, Conservation and MECP design criteria for municipal services and stormwater management (SWM) measures

This report is intended to demonstrate the feasibility of site servicing for the proposed development, including water, sanitary and stormwater management. Site grading, and preliminary erosion / sediment control information is also included. The functional servicing design has been prepared in accordance with design criteria and requirements of the Town of Halton Hills and Credit Valley Conservation Authority.

Supporting drawings are available in **Appendix B**. Drawings do not provide design information pertaining to the internal servicing of the proposed buildings. As part of the civil engineering design, a set of storm, sanitary and water connections will be provided for the proposed residential development, beyond which it will be the responsibility of the building mechanical engineer to design the internal servicing concept, in conformance with the servicing and grading design.

1.1 Subject Site

The 130 Mountainview Road North property is located in the Town of Halton Hills (Georgetown) within the Region of Halton. The site is bounded by River Drive to the north, the CN Railway Line to the south, Mountainview Road to the east, and existing residential and employment uses to the west. The site is adjacent to a designated heritage building, located to the west of the site at 2 Rosetta Street.

The site is approximately 3.75 ha. The land has been vacant for many years, save for one existing structure in the southeast corner of the property, but was previously used for industrial purposes. The majority of the area is naturalized (tree cover). The site lies within the limits of the Credit River sub-watershed, under the Credit Valley Conservation Authority (CVC) jurisdiction. It is noted that there are no CVC regulated areas or areas identified as Natural Heritage on the property.

The proposed development includes three (3) blocks of mixed residential and commercial towers (identified as Parcel A, Parcel B, and Parcel C in the Architectural Plans attached in **Appendix A**), all with multiple buildings, podiums and underground parking. Parcel C includes a privately owned park (POP) space. A new, 20 m right-of-way public road bisects the property, connecting the area to both Mountainview Road North and River Drive.

1.2 Georgetown GO Station Area Background

The 130 Mountainview Road site is within the Town of Halton Hills' Georgetown GO Station Area Secondary Plan as outlined in the Town's Official Plan. The property is designated as High Density Residential / Mixed Use Area 1. Apartment buildings and mixed-use buildings are permitted under the current land use policies.

The property is also within an identified Strategic Growth Area, being a Major Transit Station Area (MTSA). In accordance with the Provincial Planning Statement (2024), redevelopment and intensification will be focused in these important area designations to maximize the number of potential transit users that are within walking distance of transit.

The development application for 130 Mountainview Road North is intended amend the current Secondary Plan policies to allow for an additional height and density.

Urbantech also acknowledges the following on-going work in this area by the Town and Region:

- The Town is currently undertaking a Secondary Plan review of the Georgetown GO Station Area / Mill Street Corridor, led by Sajecki Planning. A draft preferred land use option was presented to the public in October 2023. Draft technical studies (i.e., Traffic Impact Study and Area Servicing Plan) are on-going. At the time of writing, no technical work has been made available for review. Approval of an updated Secondary Plan is currently planned for Q1, 2025.
- The Region of Halton is currently updating their Water and Wastewater Master Plan. The current Master Plan was completed in 2011 with a planning horizon to 2031. The new Master Plan will review infrastructure needs to 2051. The Region is working with the Town of Halton Hills to ensure growth projections for the GO Station area are properly identified and considered. Future infrastructure improvements and phasing will be determined through the Master Plan process. To-date, there has been no specific information shared regarding future servicing plans or infrastructure projects associated with the GO Station area.
- The Region of Halton is currently constructing new, large-scale water and sanitary infrastructure on Trafalgar Road and Eighth Line that will transition a portion of Georgetown (generally south of Silver Creek) from groundwater service to lake-water service. The transfer of service impacts the GO Station area as follows:
 - The transfer of south Georgetown from groundwater to lake-based water will free up groundwater capacity for growth in north Georgetown (including the GO Station area).

- The transfer of south Georgetown from the Georgetown Wastewater Treatment Plant (WWTP) to the Mid-Halton WWTP (Oakville) will free up wastewater treatment capacity for areas in north Georgetown (including the GO Station area).

The transfer of water service is complete (the transfer occurred in November 2024). The transfer of wastewater service is planned for the end of 2025.

2. EXISTING CONDITIONS & SITE GRADING

2.1 Existing Soil Conditions

A preliminary geotechnical and hydrogeological investigation was conducted by BlueFrog Environmental Consulting Inc. (available under separate cover). Eleven (11) boreholes were drilled to provide geotechnical parameters and recommendations for design. These were drilled at depths ranging from 3.1 mbgs (meters below ground surface) to 7.9 mbgs.

The findings of the geotechnical investigation conclude that soil stratigraphy of the site consists of an upper layer of topsoil, black coal, brown sand and gravel fill. In some areas, either exposed or just below ground cover, were silty fine sand to silt fill interlayered with white kaolinite fill and fine sand. Fill at increased depths included trace gravel, rootlets, coal and topsoil. Weathered grey shale bedrock was inferred at a depth of approximately 3.6 mbgs.

An earthworks and soil management program will be defined at detailed design for removals, soil handling and kaolinite management.

2.2 Existing Groundwater Conditions

Six (6) groundwater monitoring wells were installed on the site and an additional eight (8) monitoring wells were existing. Groundwater depths (measured two weeks after drilling and then one month later), range from 1.4 mbgs to 7.2 mbgs (with one dry well). It is noted in the report that groundwater levels are seasonal and may vary from these measurements at other times of the year and in response to weather events.

Excavation for development may require temporary and/or permanent dewatering. Details on the ultimate proposed excavation and a refined hydrogeological investigation, including groundwater quality, will be required to estimate dewatering volumes and identify discharge requirements. Further consultation with a de-watering contractor, hydrogeologist and environmental consultant at detailed design is required to better understand post-construction de-watering options.

Management options will be further evaluated at the detailed design stage, including overcontrol of stormwater storage tank discharge, waterproofing, etc., depending on the environmental and water quality results.

2.3 Site Grading Design

Under existing conditions, site grades range from approximately 260 m to 248 m, with highest grades at the CN Railway Line and lowest grades at the intersection of Mountainview Road and River Drive. The proposed grading design for the site takes into consideration the following requirements and constraints:

- Conforms to the Town of Halton Hills' design criteria.
- Minimizes cut to fill operations and work towards a balanced site.
- Matches existing lot boundary and road grading conditions (ensure no impact to abutting properties).
- Provides emergency overland flow conveyance for major storm conditions.
- Provides minimum cover on proposed servicing.
- Ensures compatibility of access to surrounding public streets and adjacent private property.

Grading along the new public road conforms with all Town of Halton Hills' design guidelines and matches existing grades at Mountainview Road North and River Drive intersections.

It is anticipated that Parcel A may require either exposed foundations or retaining walls to accommodate grade changes along the Street Line. Further grading coordination will be required as design proceeds.

Preliminary grading is shown on **Drawing GRD-1** in **Appendix B**. Detailed block grading will be completed at the site plan stage.

3. STORM DRAINAGE & STORMWATER MANAGEMENT

3.1 Stormwater Management Design Criteria

The Town and CVC outline the following design criteria for the site as follows:

- **Quantity Control:** maintain post development peak runoff at pre-development levels for all events up to the 100-year events.
- **Quality Control:** Ensure minimum 80% TSS removal on site.
- **Infiltration / Retention:** Per CVC SWM Criteria (Section 4.2), the first 5 mm of runoff is to be retained on-site.
- Provide safe overland flow conveyance of the 100-year event.

3.2 Existing Stormwater Infrastructure

The existing stormwater sewer network in the vicinity of the site includes:

- 300 mm storm sewer on Mountainview Drive
- 525 mm storm sewer on River Drive, and
- 525 mm storm sewer connection from the Subject Lands to River Drive.

The locations of these sewers are shown on **Drawing STM-1**.

Under existing conditions, drainage from the site does not flow to any existing infrastructure. Runoff from surrounding public roads flows to catchbasins located within Mountainview Road and River Drive.

Two external drainage catchments (0.27 ha) located to the south of the subject lands are conveyed though the subject lands under existing conditions:

- 15 River Drive – existing detached residential building
- 11 River Drive – existing detached residential building

3.3 Proposed Stormwater Management Strategy / Storm Sewer Design

The storm drainage concept for the site is designed to maintain flows and contributing drainage areas to the existing outlet, in accordance with the targets established in **Section 3.5** below. The full-coverage buildings will convey flow via area drains to proposed storage tanks (one (1) tank per parcel, three (3) total) located within the underground parking structures. These storage tanks will discharge controlled flows to the proposed storm sewer within the new public ROW.

The storm sewer is sized to accommodate a 100-year storm event on-site, ensuring target flow rates are not exceeded. Refer to **Drawing STM-2** in **Appendix B** for drainage areas and pipe networks, and **Appendix C** for storm design sheets.

To achieve the target flows, a super pipe is proposed between MH 3 and MH 7 to retain the 100-year event and mitigate overland flow to surrounding roads. The super pipe will consist of a 1500 x 2400 mm concrete box storm sewer, approximately 114 m in length. An orifice plate will be installed in MH 7 to create storage in the super pipe, with the orifice size to be confirmed at detailed design.

The controlled flows for the site will outlet through the existing 525 mm sewer connection to River Drive. Due to grading constraints, a portion of the site adjacent to the existing ROWs (~0.59 ha) will drain uncontrolled to the road, similar to current conditions. This uncontrolled flow has been considered in the design of the site's quantity controls.

3.4 Proposed Quality Control

As identified in **Section 3.1**, the site is required to meet a minimum of 80% TSS removal on site for quality control. In order to achieve the required TSS removal, a combination of catchbasin shields, and ETV certified Oil Grit Separators (OGS) will be used to accomplish TSS removal for the proposed buildings and the road. OGS devices will be used for the building blocks and a combination of CB shields and OGS will be used for the road portion of the development. The final sizing of the OGS devices will be determined as the design advances further. Preliminary sizing calculations are included in **Appendix C**.

The proposed treatment train is as follows:

Subject lands (Parcels A, B, C):

- Proposed OGS within tanks (60% TSS removal, EFO-6 for Parcels, A, B, & C)
- Proposed OGS within the downstream ROW (60% TSS removal, EFO-12)
- Total TSS Removal = $0.6 + (1 - 0.6) \times 0.6 = 84\%$

Subject Lands (Proposed ROW):

- Proposed CB Shields within ROW (50% TSS removal)
- Proposed OGS within ROW (60% TSS removal, EFO-12)
- Total TSS Removal = $0.5 + (1 - 0.5) \times 0.6 = 80\%$

3.5 Quantity Control

A Visual OTTHYMO model was created to model the drainage from the site to determine the pre-development flows. A 24-hour Chicago storm was used to simulate the rainfall on the site using the Town of Halton hills IDF parameters. In accordance with the existing drainage plan, the site was modelled as 1 catchment area with runoff coefficient of 0.25. See drainage map in **Appendix C** for existing catchments. Table 3-1 outlines the pre-development flows.

Table 3-1: Pre-Development Targets

Area (ha)	Runoff Coefficient	5-year Target (m ³ /s)
3.75	0.25	0.249

The site will discharge to the existing storm sewer system, and the existing downstream infrastructure to the site has been designed for the 5-year flows. Given this information, the flow targets for the site are limited to the 5-year existing flow.

Refer to **Drawing STM-2** in **Appendix B** for the existing storm drainage plan.

Under post-development conditions, flows will be retained onsite in storage tanks within the proposed parcel blocks, as well as within the proposed super pipe within the road that will ultimately outlet to River Drive. Due to grading constraints along the property boundary, 0.59 ha of site will be uncontrolled, with the remainder of the site being overcontrolled to ensure that the target flow rate of 0.249 m³/s is not exceeded.

The tank flows are controlled to ensure that the 5-year pre-development target is not exceeded during the 100-year event. **Table 3-2** summarizes the flow, storage values, and orifice sizes required based on the VO6 calculations.

Table 3-2: Flow and Required Storage Volume Results

Outlet	Area (ha)	Runoff Coefficient	Post Development 100-year Flows (m ³ /s)	Required Volume (m ³)
Buildings A, B, C	0.72	0.9	0.026	455
Buildings D, E	0.77	0.9	0.026	498
Buildings F, G	0.80	0.83	0.026	499
Super Pipe	3.19	0.82	0.144	413

Quantity control will be provided through the use of orifice tubes. The sizes of the orifice tubes will be confirmed as the design advances.

Table 3-3 summarizes the pre-to post flows for 2-year to 100-year storm events.

Table 3-3: Pre to Post Flows for the 2-year to 100-year Storm Events

Outlet	Storm Event	Existing Flows (m ³ /s)	Post-Development Flows (m ³ /s)
Overall Site	2-year	0.141	0.102
	5-year	0.249	0.148
	10-year	0.329	0.172
	25-year	0.433	0.203
	50-year	0.513	0.226
	100-year	0.593	0.249

Based on the above, the proposed quantity control measures have been shown to ensure that the 100-year target flow rate of 0.249 m³/s is not exceeded.

Refer to SWM Calculations in **Appendix C** for supporting calculations and Drawing **STM-2** in **Appendix B** for the post development storm drainage plan.

3.6 Water Balance / Site Retention

As noted in **Section 3.1**, 5 mm of retention is targeted over the entire area of the subject site. Based on the total development area of 2.59 ha, which includes the three (3) building parcels and new public road ROW, a total retention volume of 129.5 m³ is required for the proposed development.

Water balance retention volume can be achieved for high density urbanized areas by a combination of Low Impact Development (LID) measures including, but not limited to:

- Storage for irrigation purposes;
- Rainwater harvesting;
- Green roofs;
- Infiltration trenches and soakaway pits (for example, within park space);

- Bioretention;
- Permeable pavement; and
- Perforated pipe systems.

The use of LIDs within the 130 Mountainview Road North site is somewhat limited due to the urban / high-density nature of the development. For example, opportunities for infiltration-based LID measures are not practical in areas occupied by buildings due to the underground parking structures. A detailed LID and water balance mitigation plan will be provided through detailed design and the site plan approval stage as more details become available. Site constraints such as clearance to the seasonally high groundwater level will be considered in the selection, implementation and location of the proposed LID measures.

For the purposes of this initial work, stormwater storage tanks within the parking structures have been sized with sumps for retention storage. This storage can be used for irrigation purposes or other mechanical re-use measures. The sumps are sized to contain the 5 mm event across the three (3) tanks.

It is noted that a site-specific water balance assessment is underway by a hydrogeologist. The required site retention will be reviewed and adjusted accordingly (as necessary) as design progresses.

4. WASTEWATER (SANITARY) SERVICING

4.1 Existing Conditions – Wastewater

4.1.1 Existing Wastewater Infrastructure

Existing wastewater infrastructure immediately adjacent to the 130 Mountainview Road North property includes:

- 300 mm sanitary sewer on River Drive (immediately north of the site), draining east towards Mountainview Road.
- 300 mm sanitary sewer on Mountainview Road, draining north towards John Street.
- 300 mm to 375 mm sanitary sewer on John Street, draining west towards the John Street Wastewater Pump Station (WWPS)

The proposed development is within the John Street WWPS drainage catchment. The pump station, located within John Street Park, is owned and operated by the Region of Halton. John Street WWPS is an older station, originally constructed in 1970. The Region completed a Schedule B Municipal Class Environmental Assessment (MCEA) Study in 2018 to identify upgrades required. Recommendations in 2018 included the construction of a new station (within John Street Park), a second forcemain, and emergency overflow pipe. An Addendum to the 2018 MCEA Study was filed in March 2024 by the Region and Associated Engineering, which concludes that a new station is not required. Instead, the John Street WWPS will undergo mechanical, electrical and instrumentation upgrades to bring the station up to current design standards. A one-hour emergency storage tank, back-up generator, emergency overflow pipe, and second forcemain will be added to the station for redundancy and safety. Detailed design work is ongoing. The work planned is not intended to change the overall capacity of the pump station.

The wastewater collected at John Street WWPS is discharged to a large downstream sewer network that includes the Silver Creek Trunk sewer, terminating at the Georgetown Wastewater Treatment Plant (WWTP).

Refer to **Drawing SAN-1** in **Appendix B** for additional information.

4.1.2 Existing Wastewater Demands

There is no current wastewater generation associated with the 130 Mountainview Road North site.

4.2 Proposed Conditions – Wastewater

4.2.1 Proposed Wastewater Servicing Strategy

The sanitary servicing strategy for the 130 Mountainview Road North property includes a new proposed 250 mm sewer to be located within the planned public ROW. The new sewer will connect to the existing 300 mm wastewater main on River Drive at existing MH5A.

Wastewater from Parcel B and Parcel C will discharge to the new 250 mm sanitary sewer on the public ROW via 250 mm sewer laterals. Wastewater from Parcel A is planned to connect directly to the existing 300 mm pipe on River Drive via existing sewer connections at MH3A and MH4A. All wastewater flow from the 130 Mountainview Road North development will be conveyed in the existing sewer system north on Mountainview Road and west to the John Street WWPS.

Per Region of Halton guidelines, a 250 mm sanitary service connection with property line inspection manhole will ultimately be provided for Parcel A, B and C. The ultimate elevation of the connection will be confirmed at detailed design with the mechanical engineer. The mechanical consultant will also be responsible for the design of all internal sanitary services.

Refer to **Drawing SAN-1** in **Appendix B** for additional information.

4.2.2 Wastewater Generation Rates

Wastewater generation rates associated with the 130 Mountainview Road North property have been calculated from Site Statistics as shown in **Appendix A**, and in accordance with Halton Region Design Criteria as follows:

- Region of Halton Water and Wastewater Linear Design Criteria (per capita generation rate, average day demand, peaking factor and inflow / infiltration rate).
- Region of Halton Built Boundary Housing Occupancy Rates (per the 2022 DC Background Study, Table A-4).

Projected wastewater generation for the site under build-out conditions is shown below, with supporting calculations available in **Appendix C**. Wastewater from all parcels ultimately discharges to the same existing 300 mm pipe on River Drive.

Total Wastewater Generation (Parcel A, B and C):

- Population: 2,409 people, 14 jobs
- Avg. Day Usage: 275 L/person/day
- Avg. Dry Weather Flow: 7.71 L/s
- Peaking Factor (Modified Harmon): 3.52
- Inflow / Infiltration: 1.07 L/s (3.75 ha area)
- Peak Wet Weather Flow: 28.22 L/s

4.2.3 Wastewater System Capacity Analysis - Linear

The Region of Halton provided their InfoWorks ICM 2024.4.0 base hydraulic model for Milton / Mid-Halton / Georgetown in November 2023 for use on this project. The model was run in accordance with instructions provided by the Region, using the 10-yr storm event and DWF wastewater profile simulations.

The model was run under both existing conditions and with the following modifications to simulate post-development conditions in the linear wastewater system:

- 130 Mountainview Road North population and jobs (2,409 people, 14 jobs), added to SMH786
- 1 Rosetta population (905 people), added to SMH52120

The 1 Rosetta development was added to the model run as a conservative estimate of long-term downstream flow conditions. Given the status of the 1 Rosetta application, the intent was to ensure that the wastewater system would be able to accommodate both developments going forward without any reduction in level of service.

Results from the modeling run are shown in **Table 4-1**. Information (i.e., max flow and pipe full capacity) has been extracted from the model (on a segment-by-segment basis) for all pipes from the proposed development to the John Street WWPS. Tabulated values include results from both the existing conditions

model and the post-development conditions model, for comparison purposes. Pipe segment numbers referenced in the table are shown graphically in **Figure 4-1**, which is taken from the Region's model output and modified to show the development locations.

Table 4-1: Wastewater Hydraulic Modeling Results

Pipe Segment	Peak Flow (Model) (m ³ s)		Pipe Full Capacity (m ³ /s)	Pipe % Full (Peak Conditions)	
	Existing Conditions	Future Conditions	All Conditions	Existing Conditions	Future Conditions
SMH55328.1	0.020	0.022	0.231	9%	10%
SMH786.1	0.035	0.042	0.198	17%	21%
SMH785.1	0.036	0.043	0.198	18%	22%
SMH14466.1	0.038	0.045	0.077	49%	58%
SMH14477.1	0.077	0.084	0.112	69%	75%
SMH55329.1	0.077	0.084	0.112	69%	75%
SMH14480.1	0.084	0.091	0.110	77%	83%
SMH14483.1	0.115	0.121	0.287	40%	42%
SMH14482.1	0.121	0.128	0.259	47%	49%

Results from the model show a relatively minor increase in future flow conditions with the addition of both 130 Mountainview Road North and 1 Rosetta developments. There is significant capacity remaining in all pipes under post-development conditions.

Wastewater flow and pipe capacity downstream of the John Street WWPS is set by pumping rate and is not anticipated to change, regardless of the flow rate into the station. The downstream wastewater network will already be sized for the pump station peak discharge rate. As such, the wastewater capacity analysis does not extend beyond the pump station.

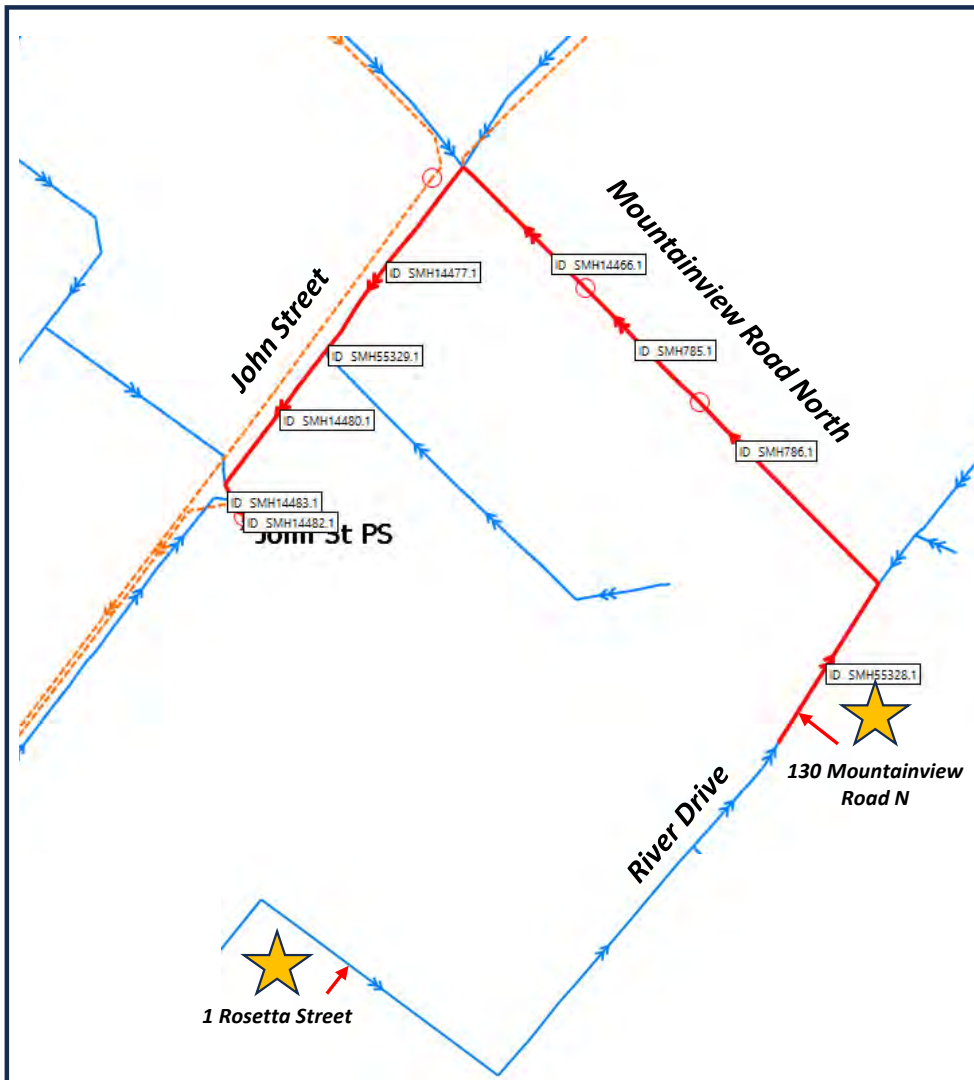


Figure 4-1: Wastewater Hydraulic Model Output

4.2.4 Wastewater System Capacity Analysis – John Street Pump Station

As outlined in the John Street Wastewater Pumping Station MCEA Study Addendum (March 2024), John Street WWPS currently operates as follows:

- Daily Average Flow Rate = 5 L/s
- Peak Hour Flow Rate = 67 L/s

The station includes a below grade wet well with one (1) submersible pump (62 L/s) and a dry well with two (2) dry pit pumps (each rated at 63 L/s). The Region calculates pump station capacity (firm capacity) with the largest pump out of service (i.e., 2 operating, 1 standby). As such, firm capacity of the John Street station is 125 L/s.

Given the current peak hour flow rate of 67 L/s, there is 58 L/s of available capacity at the John Street WWPS, which is sufficient to accommodate peak wastewater flow from the 130 Mountainview Road North development (estimated at 28.1 L/s) and the 1 Rosetta property (11.5 L/s, per Arcadis FSR).

4.2.5 Wastewater Treatment Capacity Analysis

As outlined in **Section 1.2**, The Region of Halton is currently constructing new, large-scale sanitary trunk infrastructure on Eighth Line (from Milton to Georgetown) that will transition a portion of Georgetown (generally south of Silver Creek) from groundwater service to lake-water service. When complete (in 2025) the Eighth Line trunk sewer will transfer wastewater generated in south Georgetown from the Georgetown Wastewater Treatment Plant to the Mid-Halton Wastewater Treatment Plant (Oakville).

Wastewater generated from the 130 Mountainview Road North development will continue to be treated in the Georgetown Wastewater Treatment Plant long-term. The transfer of service will free up significant capacity at the Georgetown Wastewater Treatment Plant for growth areas in north Georgetown, including the GO Station area. As such, wastewater treatment capacity is not anticipated to be a limiting factor for this development.

5. WATER SERVICING

5.1 Existing Conditions – Water

5.1.1 Existing Water Infrastructure

The existing water network, per the region's record information, in the vicinity of the site includes:

- 250 mm watermain within River Drive
- 250 mm watermain within Mountainview Road North
- Hydrant at the southwest corner of Mountainview Road North and River Drive, within the subject lands.

The 130 Mountainview Road North property is within the G6G water pressure zone, which is serviced directly by the Town's three municipal groundwater wellfields (Princess Anne, Lindsey Court and Cedarvale). The distribution network is highly integrated; water supply is not specific to an area but is provided throughout the network by all wellfields. Floating storage for zone G6G is provided by the No. 22 Sideroad Reservoir and the Todd Road Elevated Tank.

5.1.2 Existing Water Demands

There is no current water demand associated with the 130 Mountainview Road North site.

5.2 Proposed Conditions - Water

5.2.1 Proposed Water Servicing Strategy

The water servicing strategy for the 130 Mountainview Road North property includes a new proposed 250 mm watermain to be located within the planned public ROW. The watermain will connect at either end of the public road to existing infrastructure on River Drive and Mountainview Road North. Each parcel will have a water service connection to the new watermain for both domestic and fire.

In addition to the above, please note that:

- Watermains will provide all domestic flow and fire protection to the 130 Mountainview Road North development.
- Per Region of Halton guidelines, a 200 mm diameter fire service and 150 mm diameter domestic (residential) service will be required for each parcel. A second 200 mm fire service may be required but will be confirmed at site plan with the mechanical engineer. The mechanical consultant will also be responsible for the design of all the internal water services.
- The final location of the fire department connection (siamese connection) for the buildings will need to be located within 80 m of a municipal fire hydrant. Fire hydrant location will be determined at detailed design but will meet Region of Halton criteria for location and spacing.

Refer to **Drawing SERV-1** in **Appendix B** for additional information.

5.2.2 Water System Demand

Municipal Engineering Solutions (MES) was retained to complete water hydraulic modeling for the 130 Mountainview Road North development. The full MES report is available in **Appendix D**. As part of the

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work, MES estimated the future water demand for the development as follows, in accordance with Halton Region and the Fire Underwriters Survey (FUS) design standards:

- Average Day Demand 7.60 L/s
- Maximum Day Demand 17.10 L/s
- Peak Hour Demand 30.36 L/s

- Fire Flow Range from 167 L/s to 200 L/s (FUS calculation)
- Fire Flow Pressure 20 psi (minimum)

5.2.3 Water System Capacity Analysis – Linear

The MES report in **Appendix D** summarizes the modeling methodology used to assess water system capacity under post-development conditions. The proposed development layout and projected populations were incorporated into the Region's existing InfoWater model (dated August 2022). The model was run under future conditions (i.e., post the south Georgetown water transfer) to assess available system capacity and pressure.

Key results are as follows:

- The site is adequately supplied from Zone G6G.
- All service flows, fire flows and delivery pressures are within the Region's required ranges under all operating conditions (average day, max day, peak hour, max day + fire) and for both 2026 and 2031 modeling scenarios.

Refer to the full report in **Appendix D** for additional information.

It is also important to note that:

- Water modeling suggests that a 200 mm pipe may be sufficient for service. A 250 mm watermain has been shown on the servicing drawings as a conservative preliminary size. Sizing will be refined at detailed design.
- Similar to the wastewater analysis, MES water modeling accounts for future development at the 1 Rosetta property, to ensure no long-term impacts.
- Hydrant tests at the two (2) closest hydrants to the subject property were tested by Watermark in September 2024. Results confirm that flow and pressure available under fire conditions are suitable for high-rise development (i.e., 360 L/s to 445 L/s at 20 psi). The Watermark report is available as **Appendix E**.

5.2.4 Water Treatment Capacity Analysis

With the completion of the Georgetown lake-based transfer of water service in November 2024, north Georgetown (including the GO Station area) remains serviced by three (3) municipal groundwater wellfields and associated treatment facilities. South Georgetown (generally south of Silver Creek) is serviced by the south Halton lake-based system. The transfer of service frees up groundwater capacity for new growth in areas of north Georgetown like the GO Station MTSA. With the transfer now complete, it is not anticipated that municipal groundwater capacity will be a limiting factor for development of the 130 Mountainview Road North site.

6. EROSION AND SEDIMENT CONTROL

Erosion and sediment controls for the subject lands will be designed in conformance with the Town and CVC guidelines. Erosion and sediment controls will be implemented during all site construction works including but not limited to topsoil stripping, bulk earthworks, foundation excavation, site servicing and stockpiling of materials and will conform to ESC guidelines (2019). The following erosion and sediment control measures are proposed to be implemented during construction:

- Installing heavy duty silt control fencing along the perimeter of the site at strategic locations.
- Installing a temporary mud mat at the construction site entrance.
- Wrapping the tops of all inlet structures with filter fabric and using install silt sacks.
- Tree preservation fencing in accordance with the tree preservation plan, if required.
- Gravel mud mat at the construction vehicle access point to minimize off-site tracking of sediments.
- Inspecting all sediment and erosion control controls to maintain them in good repair until such time as the Engineer or the Town approves their removal.

If required, site-specific measures will be determined during the detailed design / site alteration application stage. A detailed Erosion & Sediment Control Plan will be provided in the future through the detailed design stage.

7. CONCLUSIONS

This report has demonstrated that:

- Stormwater management for the site is designed to maintain flows and contributing drainage areas to the existing outlet, in accordance with quality and quantity control targets:
 - Maintain post development peak runoff at pre-development levels for all events up to the 100-year events.
 - Ensure minimum 80% TSS removal on site.
- Stormwater quantity control storage for the site plan blocks (Parcel A, B and C) will be provided by storage tanks located in the underground parking structures. Quantity control storage for the road ROW catchment and park block will be provided by superpipe, integrated with the proposed storm servicing plan. Runoff from the road ROW catchments is to be captured by proposed catchbasins to the storm sewer system, where the proposed superpipe will provide the required storage attenuation.
- Enhanced Level 1 quality control will be provided by a treatment train approach combining treatment via oil/grit separator (OGS) units and LIDs.
- Per CVC SWM Criteria, the first 5 mm of runoff will be retained on-site. Based on the total development area including the site plan blocks and proposed ROWs, a total retention volume of 129.5 m³ is required. Retention will ultimately be achieved by a combination of Low Impact Development (LID) measures, to be defined at the site plan approval stage. Retention volumes will be checked and refined (as required) when site water balance information is available.
- Sanitary servicing will be provided by a new 250 mm diameter sewer within the proposed public ROW. Wastewater generated from the property will be conveyed to the John Street WWPS via existing sanitary sewers on River Drive and Mountainview Road.
- Sanitary hydraulic modelling shows that capacity is available in the existing system to accommodate build-out of the proposed 130 Mountainview Road North development. Downstream pump station and wastewater treatment capacity is also available.
- Water servicing for the site will be provided by a new 250 mm diameter watermain within the proposed public ROW. The new watermain will connect at both ends to existing 250 mm watermains on River Drive and Mountainview Road North.
- Water modelling carried out by MES shows that adequate flow and pressure can be delivered to the proposed development under both domestic use and fire flow conditions. Water treatment capacity and municipal groundwater supply is available with the recent completion of the Region's lake-based service transfer (November 2024).
- Erosion and sediment controls will be implemented during construction in accordance with Erosion and Sediment Control Guidelines.

Report Prepared by:



Janna Ormond, P. Eng.
Project Manager, Water Resources



Kate Connell, P. Eng.
Senior Project Manager

APPENDIX A

Site Statistics and Site Plan (Arcadis Architects (Canada) Inc.)

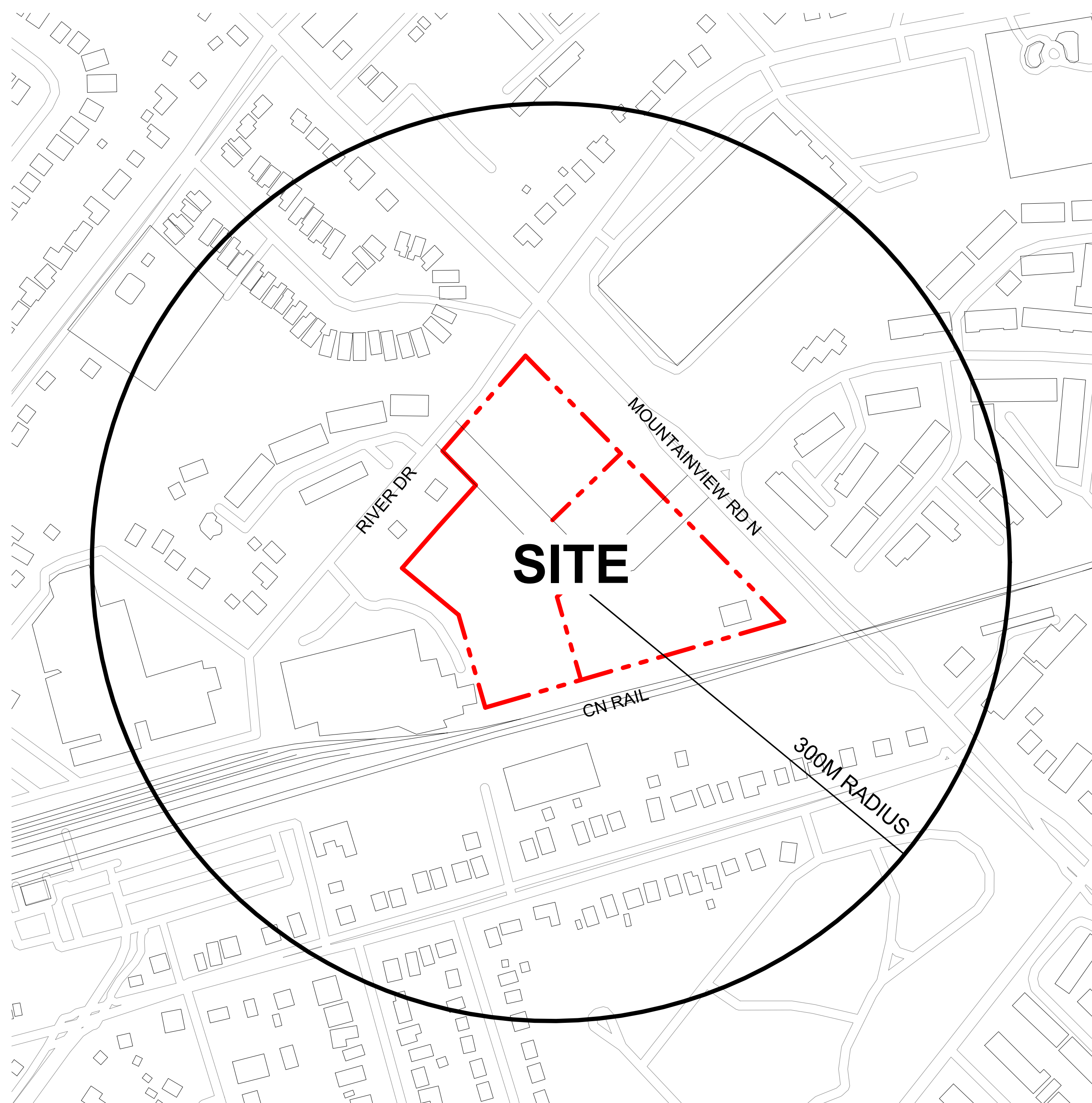
PROJECT OVERALL STATISTICS			
m - Denotes Meters	min - Denotes Minimum		
sqm - Denotes Square Meters	max - Denotes Maximum		
PROJECT DATA			
Municipal Address of Subject Lands:	130 Mountainview Rd N, Halton Hills, Ontario		
Zoning:	Halton Hills Zoning By-Law No. 2010-0057		
Proposed Use:	Mixed Use Residential		
OCB Building Classification:	New Construction - Part 3 - Group C - Residential Use - Sprinklered		
Number of Buildings:	2 Townhouses + 1 Mid-Rise Building		
Proposed F.S.I.:	4.55 (GFA + Project Site)		
Parcel Area:	7865.00 sqm *** Project Site		
Established Grade:	257.05 m CDG (Canadian Geodetic Datum)		
Survey Benchmark: Elevations shown hereon are geodetic and are derived from the control station 08B20178040 having an elevation of 272.174m (cgvd-1928/1978). Refer to Survey for clarification.			
BUILDING HEIGHT			
Tower A	Height to Top of Residential Roof	59.30 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	17.00	65.30 m
Tower B	Height to Top of Residential Roof	74.75 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	22.00	80.75 m
Tower C	Height to Top of Residential Roof	59.30 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	17.00	65.30 m
Tower D	Height to Top of Residential Roof	59.30 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	17.00	65.30 m
Tower E	Height to Top of Residential Roof	69.25 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	20.00	75.25 m
Tower F	Height to Top of Residential Roof	75.30 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	22.00	81.30 m
Tower G	Height to Top of Residential Roof	43.35 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	12.00	49.35 m
Note: Building height excludes mechanical penthouses up to 6.0m, chimney vents, skylights, antennae, elevator machine rooms and parapet walls, and is measured from the established grade.			
PROPOSED AREAS			
Residential GFA:	128,978.00 sqm * GFA as defined by Halton Hills Zoning By-Law No. 2010-0057		
Non-Res GFA:	525.00 sqm		
Total GFA:	129,503.00 sqm		
At Grade Condition:	16,271.40 sqm *** Building Footprint including cantilevered areas		
Landscaped Open Space:	6,839.00 sqm *** Project Site Soil Landscaping + Hard Landscaping areas		
Paved Surface Area:	1,054.00 sqm *** Driveway + Parking lot		
RESIDENTIAL UNIT MIX			
Unit Type	Unit Count	Typical Unit Size	Percent
Studio	33	42.00 sqm	3.3%
1 Bedroom	737	42.00 sqm	49.8%
2 Bedroom	629	42.00 sqm	42.5%
3 Bedroom	62	42.00 sqm	4.2%
TOTAL:	1,461		
AMENITY			
Indoor Amenity Provided:	Area	2,962.00 2.0 m ² per unit	
Outdoor Amenity Provided:	Area	2,962.00 2.0 m ² per unit	
Total Indoor & Outdoor Provided Amenity:	Area	5,924.00 4.0 m ² per unit	
PARKING SPACE			
REQUIRED (as per B185)	Parking Spaces	PROVIDED	Parking Spaces
Total Parking Spaces Required:	0	Total Parking Spaces Provided:	670
Breakdown of parking space by use allocation:			
Residential (0.0 per Unit)	0	Residential (0.0 per Unit)	603
Visitors (0.1 per Unit)	0	Visitors (0.1 per Unit)	67
No. of parking Spaces to be Accessible	0	Total Accessible Spaces 2 vs +14 Res	16
Breakdown of Provided parking space by location:			
Above Grade Parking	629		
Below Grade Parking	652		
BICYCLE PARKING SPACE			
REQUIRED Bicycle Parking Space	Spaces	PROVIDED Bicycle Parking Space	Spaces
Long-term Res. Spaces (0.7/Unit)	1,037	Long-term Spaces @ Mezz Level	339
Short-term Vis. Spaces @ Grade	32	Long-term Spaces @ L1	398
Non-Res Spaces @ Grade	2	Short-term Spaces @ Grade	34
TOTAL REQUIRED:	1,071	TOTAL PROVIDED:	1,071
1,037 indoor long-term spaces (Bike Room @ L1 & Mezz) + 34 outdoor spaces (32 short-term Res+2 Non-Res) located next to Entrances @ grade. Refer to A200 & A201			
LOADING SPACE			
1 Type G Loading Space Provided: Size: 13(L) X 4(W) X 7.5(H)	200mm reinforced concrete pad with 7.5m Ceiling Height for entire loading area. Refer to A200 & A201 & A202 Drawings.		
1 Type B Loading Space: Size: 11(L) X 3.5(W) X 4.0(H)	200mm reinforced concrete pad with 7.5m Ceiling Height for entire loading area. Refer to A200 & A201 & A202 Drawings.		

PROJECT STATISTICS PARCEL B			
m - Denotes Meters	min - Denotes Minimum		
sqm - Denotes Square Meters	max - Denotes Maximum		
PROJECT DATA			
Municipal Address of Subject Lands:	Parcel B - 130 Mountainview Rd N, Halton Hills, Ontario		
Zoning:	Halton Hills Zoning By-Law No. 2010-0057		
Proposed Use:	Residential		
OCB Building Classification:	New Construction - Part 3 - Group C - Residential Use - Sprinklered		
Number of Buildings:	2 Towers on shared podium		
Proposed F.S.I.:	4.42 (GFA + Project Site)		
Parcel Area:	7865.00 sqm *** Project Site		
Established Grade:	257.05 m CDG (Canadian Geodetic Datum)		
Survey Benchmark: Elevations shown hereon are geodetic and are derived from the control station 08B20178040 having an elevation of 272.174m (cgvd-1928/1978). Refer to Survey for clarification.			
BUILDING HEIGHT			
Tower D	Height to Top of Residential Roof	59.30 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	17.00	65.30 m
Tower E	Height to Top of Residential Roof	69.25 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	20.00	75.25 m
Note: Building height excludes mechanical penthouses up to 6.0m, chimney vents, skylights, antennae, elevator machine rooms and parapet walls, and is measured from the established grade.			
PROPOSED AREAS			
Residential GFA:	34,750.00 sqm * GFA as defined by Halton Hills Zoning By-Law No. 2010-0057		
Non-Res GFA:	5415.00 sqm *** Building Footprint including cantilevered areas		
Landscaped Open Space:	1,749.00 sqm *** Project Site Soil Landscaping + Hard Landscaping areas		
Paved Surface Area:	701.00 sqm *** Driveway + Parking lot		
RESIDENTIAL UNIT MIX			
Unit Type	Unit Count	Typical Unit Size	Percent
Studio	11	42.00 sqm	2.7%
1 Bedroom	259	42.00 sqm	62.7%
2 Bedroom	123	42.00 sqm	29.8%
3 Bedroom	20	42.00 sqm	4.8%
TOTAL:	413		
AMENITY			
Indoor Amenity Provided:	Area	826.00 2.0 m ² per unit	
Outdoor Amenity Provided:	Area	826.00 2.0 m ² per unit	
Total Indoor & Outdoor Provided Amenity:	Area	1,652.00 4.0 m ² per unit	
PARKING SPACE			
REQUIRED (as per B185)	Parking Spaces	PROVIDED	Parking Spaces
Total Parking Spaces Required:	0	Total Parking Spaces Provided:	413
Breakdown of parking space by use allocation:			
Residential (0.0 per Unit)	0	Residential (0.0 per Unit)	372
Visitors (0.1 per Unit)	0	Visitors (0.1 per Unit)	41
No. of parking Spaces to be Accessible	0	Total Accessible Spaces 1 vs +10 Res	11
Breakdown of Provided parking space by location:			
Above Grade Parking	338		
Below Grade Parking	75		
BICYCLE PARKING SPACE			
REQUIRED Bicycle Parking Space	Spaces	PROVIDED Bicycle Parking Space	Spaces
Long-term Res. Spaces (0.7/Unit)	289	Long-term Spaces @ Mezz Level	210
Short-term Vis. Spaces @ Grade	12	Long-term Spaces @ L1	79
Short-term Spaces @ Grade	0	Short-term Spaces @ Grade	12
TOTAL REQUIRED:	301	TOTAL PROVIDED:	289
289 indoor long-term spaces (Bike Room @ L1 & Mezz) + 12 short-term outdoor spaces located next to Entrances @ grade. Refer to A200, A201			
LOADING SPACE			
1 Type G Loading Space: Size: 13(L) X 4(W) X 7.5(H)	200mm reinforced concrete pad with 7.5m Ceiling Height for entire loading area for Type G loading spaces. Refer to A200 & A201 & A202 Drawings.		
1 Type B Loading Space: Size: 11(L) X 3.5(W) X 4.0(H)	200mm reinforced concrete pad with 7.5m Ceiling Height for entire loading area for Type B loading spaces. Refer to A200 & A201 & A202 Drawings.		
Both towers will share the type G for garbage pick up. All bins will be moved to type G area for pick up.			
Tower E will use Type B for moving purposes. Tower D will use the Type G for moving purposes			
GARBAGE ROOM			
Breakdown of main garbage room area:			
Bulk Storage	10.00 sqm		
Residential Garbage Room & Storage	120.00 sqm		Refer to A200 drawing
TOTAL:	130.00 sqm		
Waste Management Bins Bin Type Total Bins for all parcel B (413 units)			
Garbage	30y-bin	9	
Recycle	30y-bin	12	
Organics	30y-bin	5	
Breakdown of temporary garbage room area for Tower E:			
Bulk Storage	10.00 sqm		Refer to A200 drawing. The garbage bins will be temporarily stored in this area and will be moved to main garbage room Type G loading area prior to pick up day
Residential Garbage Room & Storage	65.00 sqm		
TOTAL:	75.00 sqm		

PROJECT STATISTICS PARCEL A			
m - Denotes Meters	min - Denotes Minimum		
sqm - Denotes Square Meters	max - Denotes Maximum		
PROJECT DATA			
Municipal Address of Subject Lands:	Parcel A - 130 Mountainview Rd N, Halton Hills, Ontario		
Zoning:	Halton Hills Zoning By-Law No. 2010-0057		
Proposed Use:	Mixed Use Residential		
OCB Building Classification:	New Construction - Part 3 - Group C - Residential Use - Sprinklered		
Number of Buildings:	3 Towers on shared podium		
Proposed F.S.I.:	7.61 (GFA + Project Site)		
Parcel Area:	7820.00 sqm *** Project Site		
Established Grade:	254.70 m CDG (Canadian Geodetic Datum)		
Survey Benchmark: Elevations shown hereon are geodetic and are derived from the control station 08B20178040 having an elevation of 272.174m (cgvd-1928/1978). Refer to Survey for clarification.			
BUILDING HEIGHT			
Tower A	Height to Top of Residential Roof	59.30 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	17.00	65.30 m
Tower B	Height to Top of Residential Roof	74.75 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	22.00	80.75 m
Tower C	Height to Top of Residential Roof	59.30 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	17.00	65.30 m
Note: Building height excludes mechanical penthouses up to 6.0m, chimney vents, skylights, antennae, elevator machine rooms and parapet walls, and is measured from the established grade.			
PROPOSED AREAS			
Residential GFA:	60,840.00 sqm * GFA as defined by Halton Hills Zoning By-Law No. 2010-0057		
Non-Res GFA:	525.00 sqm		
Total GFA:	61,365.00 sqm		
At Grade Condition:	6,088.40 sqm *** Building Footprint including cantilevered areas		
Landscaped Open Space:	1,480.00 sqm *** Project Site Soil Landscaping + Hard Landscaping areas		
Paved Surface Area:	271.00 sqm *** Driveway + Parking lot		
RESIDENTIAL UNIT MIX			
Unit Type	Unit Count	Typical Unit Size	Percent
Studio	52	42.00 sqm	3.3%
1 Bedroom	325	42.00 sqm	45.5%
2 Bedroom	307	42.00 sqm	45.8%
3 Bedroom	16	42.00 sqm	2.4%
TOTAL:	670		
AMENITY			
Indoor Amenity Provided:	Area	1340.00 2.0 m ² per unit	
Outdoor Amenity Provided:	Area	1340.00 2.0 m ² per unit	
Total Indoor & Outdoor Provided Amenity:	Area	2,680.00 4.0 m ² per unit	
PARKING SPACE			
REQUIRED (as per B185)	Parking Spaces	PROVIDED	Parking Spaces
Total Parking Spaces Required:	0	Total Parking Spaces Provided:	670
Breakdown of parking space by use allocation:			
Residential (0.0 per Unit)	0	Residential (0.0 per Unit)	603
Visitors (0.1 per Unit)	0	Visitors (0.1 per Unit)	67
No. of parking Spaces to be Accessible	0	Total Accessible Spaces 2 vs +14 Res	16
Breakdown of Provided parking space by location:			
Above Grade Parking	173		
Below Grade Parking	497		
BICYCLE PARKING SPACE			
REQUIRED Bicycle Parking Space	Spaces	PROVIDED Bicycle Parking Space	Spaces
Long-term Res. Spaces (0.7/Unit)	469	Long-term Spaces @ Mezz Level	150
Short-term Vis. Spaces @ Grade	12	Long-term Spaces @ L1	319
Non-Res Spaces @ Grade	2	Short-term Spaces @ Grade	12
TOTAL REQUIRED:	481	TOTAL PROVIDED:	481
469 indoor long-term spaces (Bike Room @ L1 & Mezz) + 12 outdoor spaces (10 short-term Res+2 Non-Res) located next to Entrances @ grade. Refer to A200, A201			
LOADING SPACE			
1 Type G Loading Space Provided (1 per Tower): Size: 13(L) X 4(W) X 7.5(H)	200mm reinforced concrete pad with 7.5m Ceiling Height for entire loading area. Refer to A200 & A201 & A202 Drawings.		
1 Type B Loading Space: Size: 11(L) X 3.5(W) X 4.0(H)	200mm reinforced concrete pad with 7.5m Ceiling Height for entire loading area. Refer to A200 & A201 & A202 Drawings.		

PROJECT STATISTICS PARCEL C			
m - Denotes Meters	min - Denotes Minimum		
sqm - Denotes Square Meters	max - Denotes Maximum		
PROJECT DATA			
Municipal Address of Subject Lands:	Parcel C - 130 Mountainview Rd N, Halton Hills, Ontario		
Zoning:	Halton Hills Zoning By-Law No. 2010-0057		
Proposed Use:	Residential		
OCB Building Classification:	New Construction - Part 3 - Group C - Residential Use - Sprinklered		
Number of Buildings:	1 Tower and 1 Mid-Rise building on shared podium		
Proposed F.S.I.:	3.96 (GFA + Project Site)		
Parcel Area:	8880.00 sqm *** Project Site		
Established Grade:	272.10 m CDG (Canadian Geodetic Datum)		
Survey Benchmark: Elevations shown hereon are geodetic and are derived from the control station 08B20178040 having an elevation of 272.174m (cgvd-1928/1978). Refer to Survey for clarification.			
BUILDING HEIGHT			
Tower F	Height to Top of Residential Roof	75.30 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	22.00	81.30 m
Building G	Height to Top of Residential Roof	43.30 m	Height to Top of MPH Roof
	No. of Storeys Proposed:	12.00	49.35 m
Note: Building height excludes mechanical penthouses up to 6.0m, chimney vents, skylights, antennae, elevator machine rooms and parapet walls, and is measured from the established grade.			
PROPOSED AREAS			
Residential GFA:	35,134.00 sqm * GFA as defined by Halton Hills Zoning By-Law No. 2010-0057		
Non-Res GFA:	4788.00 sqm *** Building Footprint including cantilevered areas		
Landscaped Open Space:	34,110.00 sqm *** Project Site Soil Landscaping + Hard Landscaping + P.D.P.		
Paved Surface Area:	692.00 sqm *** Driveway + Parking lot		
RESIDENTIAL UNIT MIX			
Unit Type	Unit Count	Typical Unit Size	Percent
Studio	50	42.00 sqm	5.0%
1 Bedroom	153	42.00 sqm	38.4%
2 Bedroom	199	42.00 sqm	50.0%
3 Bedroom	28	42.00 sqm	6.5%
TOTAL:	328		
AMENITY			
Indoor Amenity Provided:	Area	706.00 2.0 m ² per unit	
Outdoor Amenity Provided:	Area	706.00 2.0 m ² per unit	
Total Indoor & Outdoor Provided Amenity:	Area	1,412.00 4.0 m ² per unit	
PARKING SPACE			
REQUIRED (as per B185)	Parking Spaces	PROVIDED	Parking Spaces
Total Parking Spaces Required:	0	Total Parking Spaces Provided:	328
Breakdown of parking space by use allocation:			
Residential (0.0 per Unit)	0	Residential (0.0 per Unit)	358
Visitors (0.1 per Unit)	0	Visitors (0.1 per Unit)	40
No. of parking Spaces to be Accessible	0	Total Accessible Spaces 1 vs +9 Res	10
Breakdown of Provided parking space by location:			
Above Grade Parking	318		
Below Grade Parking	60		
BICYCLE PARKING SPACE			
REQUIRED Bicycle Parking Space	Spaces	PROVIDED Bicycle Parking Space	Spaces
Long-term Res. Spaces (0.7/Unit)	279	Long-term Spaces @ Mezz Level	279
Short-term Vis. Spaces @ Grade	0	Long-term Spaces @ L1	0
Short-term Spaces @ Grade	0	Short-term Spaces @ Grade	0
TOTAL REQUIRED:	279	TOTAL PROVIDED:	279
279 indoor long-term spaces (Bike Room @ Mezz) + 0 short-term outdoor spaces located next to Entrances @ grade. Refer to A200, A201			
LOADING SPACE			
1 Type G Loading Space: Size: 13(L) X 4(W) X 7.5(H)	200mm reinforced concrete pad with 7.5m Ceiling Height for entire loading area for Type G loading spaces. Refer to A200 & A201 & A202 Drawings.		
1 Type B Loading Space: Size: 11(L) X 3.5(W) X 4.0(H)	200mm reinforced concrete pad with 7.5m Ceiling Height for entire loading area for Type B loading spaces. Refer to A200 & A201 & A202 Drawings.		
Waste Management Bins Bin Type Total Bins for all parcel C (328 units)			
Garbage	30y-bin	9	
Recycle	30y-bin	12	
Organics	30y-bin	5	

1 CONTEXT PLAN
A001 Scale: 1 : 2000



PARCEL A PROVIDED PARKING SPACES BREAKDOWN					
PARKING SPACE COUNT BY LEVEL					
LEVEL	REG	Vis	ACCESSIBLE	Vis	TOTAL
P3	152	0	2	0	154
P2	188	0	2	0	190
P1	84	65	2	2	153
Ground level	0	0	0	0	0
L2 Parking	49	0	2	0	51
L3 Parking	57	0	3	0	60
L4 Parking	59	0	3	0	62
TOTAL	589	65	14	2	670

PARKING SPACE COUNT BY ASSIGNMENT				
Parking Space	Assignment	REG	ACCESSIBLE	TOTAL
VISITOR	V	65	2	67
RESIDENTIAL	R	589	14	603
TOTAL		654	16	670

ACCESSIBLE PARKING SPACE COUNT/LOCATION				
Level	Type	Assignment	TOTAL	
P3,P2,P1,L2,L3,L4	BF 3900x5500	R	8	
GF	BF 3900x5500	V	1	
L2,L3,L4	BF 490			

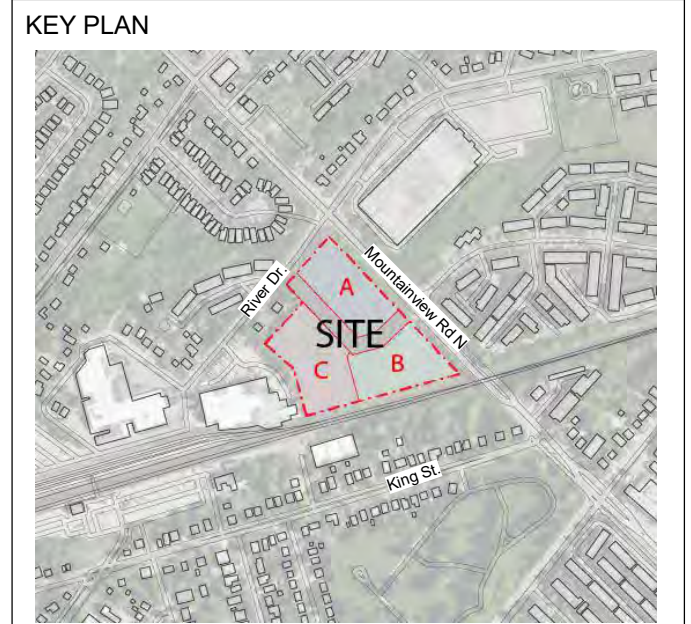
PARCEL A																									
Floor	Building GCA (m²)	Parking GCA (m²)	GFA (m²)	Unit Mix					Unit Mix Breakdown																
				S	1B	2B	TH / Duplex 2B	3B	Total	S (< 450 ft²)	1B (450-530 ft²)	1B+D (18A) (530-590 ft²)	1B+D (28A) (590-705 ft²)	2B (705-725 ft²)	2B+D (28A) (725-1030 ft²)	TH / Duplex 2B (> 850 ft²)	3B (> 850 ft²)								
MPH	634	0	634																						
LEVEL 22	750	0	750	0	3	7	0	0	10	0	1	1	1	4	3	0	0	0	0	0					
LEVEL 21	750	0	750	0	3	7	0	0	10	0	1	1	1	4	3	0	0	0	0	0					
LEVEL 20	750	0	750	0	3	7	0	0	10	0	1	1	1	4	3	0	0	0	0	0					
LEVEL 19	750	0	750	0	3	7	0	0	10	0	1	1	1	4	3	0	0	0	0	0					
LEVEL 18	750	0	750	0	3	7	0	0	10	0	1	1	1	4	3	0	0	0	0	0					
LEVEL 17	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 16	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 15	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 14	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 13	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 12	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 11	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 10	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 9	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 8	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 7	2,350	0	2,350	2	15	17	0	0	34	2	5	3	7	6	11	0	0	0	0	0					
LEVEL 6	3,978	0	4,058	0	28	17	0	7	52	0	4	2	21	4	13	0	7	5	0	7					
LEVEL 5	3,978	0	4,058	0	24	5	0	5	34	0	4	0	19	2	3	0	5	0	5	5					
LEVEL 4	6,158	2,535	3,702	0	32	10	0	1	43	0	6	24	2	1	9	0	1	9	0	1					
LEVEL 3	6,158	2,535	3,702	0	32	10	0	1	43	0	6	24	2	1	9	0	1	9	0	1					
LEVEL 2	6,159	2,502	3,737	0	24	9	9	1	43	0	5	18	1	1	8	0	1	8	0	1					
MEZZANINE	1,809	0	1,809	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0					
LEVEL 1	5,887	287	5,600	0	5	0	19	1	25	0	0	0	5	0	0	0	1	5	0	1					
P1	7,575	5,856	1,799	0	0	0	6	0	6	0	0	0	0	0	0	0	0	0	0	0					
P2	7,896	7,351	545																						
P3	6,510	6,222	289																						
Subtotal																									
Total	86,348	27,289	59,619	22	325	273	34	16	670	22	85	111	127	95	178	0	16	3.3%	12.7%	16.6%	19.0%	14.2%	26.6%	0.0%	2.4%

PARCEL B																									
Floor	Building GCA (m²)	Parking GCA (m²)	GFA (m²)	Unit Mix					Unit Mix Breakdown																
				S	1B	2B	TH / Duplex 2B	3B	Total	S (< 450 ft²)	1B (450-530 ft²)	1B+D (18A) (530-590 ft²)	1B+D (28A) (590-705 ft²)	2B (705-725 ft²)	2B+D (28A) (725-1030 ft²)	TH / Duplex 2B (> 850 ft²)	3B (> 850 ft²)								
MPH	383	0	383																						
LEVEL 22	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0					
LEVEL 21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0					
LEVEL 20	800	0	800	0	11	2	0	0	13	0	6	2	3	0	2	0	0	0	0	0					
LEVEL 19	800	0	800	0	11	2	0	0	13	0	6	2	3	0	2	0	0	0	0	0					
LEVEL 18	800	0	800	0	11	2	0	0	13	0	6	2	3	0	2	0	0	0	0	0					
LEVEL 17	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 16	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 15	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 14	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 13	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 12	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 11	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 10	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 9	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 8	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 7	1,549	0	1,549	1	17	5	0	1	24	1	8	2	7	2	3	0	1	1	1	1					
LEVEL 6	2,649	0	2,729	0	9	19	0	4	32	0	0	0	8	2	17	0	4	2	4	2					
LEVEL 5	2,661	0	2,741	0	6	15	0	2	23	0	0	0	6	1	14	0	2	2	2	2					
LEVEL 4	5,013	3,385	1,708	0	8	6	0	1	15	0	2	5	1	0	6	0	1	1	1	1					
LEVEL 3	5,396	3,768	1,708	0	8	6	0	1	15	0	2	5	1	0	6	0	1	1	1	1					
LEVEL 2	5,473	3,771	1,782	0	8	6	0	1	15	0	2	5	1	0	6	0	1	1	1	1					
MEZZANINE	1,030	0	1,030	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0					
LEVEL 1	5,305	2,824	2,561	0	0	5	0	0	5	0	0	0	0	0	5	0	0	0	0	0					
P1	3,501	2,917	665	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0					
P2	0	0	0																						
P3	0	0	0																						
Subtotal																									
Total	50,855	16,664	34,750	11	259	123	0	20	413	11	112	43	103	25	93	0	20	2.7%	27.1%	10.4%	24.9%	6.1%	22.5%	0.0%	4.8%

PARCEL C																				
Floor	Building GCA (m²)	Parking GCA (m²)	GFA (m²)	Unit Mix					Unit Mix Breakdown											
				S	1B	2B	TH / Duplex 2B	3B	Total	S (< 450 ft²)	1B (450-530 ft²)	1B+D (18A) (530-590 ft²)	1B+D (28A) (590-705 ft²)	2B (705-725 ft²)	2B+D (28A) (725-1030 ft²)	TH / Duplex 2B (> 850 ft²)	3B (> 850 ft²)			
MPH	196	0	196																	
LEVEL 22	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 21	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 20	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 19	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 18	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 17	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 16	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 15	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 14	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 13	749	0	749	1	6	3	0	1	11	1	2	0	4	2	1	0	1	1	1	1
LEVEL 12	2,044	0	2,044	1	8	17	0	1	27	1	2	2	4	3	14	0	1	1	1	1
LEVEL 11	2,059	0	2,059	1	8	17	0	1	27	1	2	2	4	3	14	0	1	1	1	1
LEVEL 10	2,188	0	2,188	1	8	19	0	1	29	1	2	1	5	3	16	0	1	1	1	1
LEVEL 9	2,188	0	2,188	1	8	19	0	1	29	1	2	1	5	3	16	0	1	1	1	1
LEVEL 8	2,333	0	2,333	1	8	21	0	1	31	1	2	1	4	4	17	0	1	1	1	1
LEVEL 7	2,333	0	2,333	1	8	21	0	1	31	1	2	1	5	3	18	0	1	1	1	1
LEVEL 6	3,029	0	3,109	1	7	23	0	4	35	1	2	1	4	1	22	0	4	4	4	4
LEVEL 5	3,029	0	3,109	0	2	21	0	3	26	0	0	0	2	1	20	0	3	3	3	3
LEVEL 4	4,790	3,455	1,414	1	12	0	0	1	14	1	0	11	1	0	0	0	1	1	1	1
LEVEL 3	4,790	3,455	1,414	1	12	0	0	1	14	1	0	11	1	0	0	0	1	1	1	1
LEVEL 2	4,826	3,405	1,501	1	12	0	0	1	14	1	0	11	1	0	0	0	0	0	0	0
MEZZANINE	1,541	0	1,541	0	3															

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Arcadis Architects (Canada) Inc.



ISSUES

No.	DESCRIPTION	DATE
1	Issued for OPA/ZBA	2024-11-22

- Notes:**
- For landscape information - Refer to Drawings Prepared by Adesso Design Inc.
 - For grading and Servicing Information - Refer to Drawings prepared by Urbantech Consulting.
 - For TIS, autoturns, curb radii and fire access route, refer to drawings prepared by GHD.
 - The building will be sprinklered.
 - 10 m² bulk storage & Garbage room is provided for each parcel, H: min 2.5m clear and equipped with Hose bib, floor drain and climate control.
 - Bi-sorter chutes (garbage, recyclable and organic) with compactor & waste containers under each chute at garbage room are provided for each Tower.
 - The loading bay is equipped with a type G loading bay for resi. moving, delivery & garbage pick up.
 - Loading area has a minimum length of 13m, width of 4m and with a vertical clearance of at least 7.5m. The entire area and staging area are constructed of 200mm reinforced concrete and have a grade of no more than 2%.
 - Flashing warning light to be activated when trucks enter and exit the site, the system to remain activated during the city garbage collection activity and until the truck exits the site. A trained on-site staff member will be available to maneuver bins for the collection driver and also act as a flagman when the truck is reversing in the event the on-site staff is unavailable at the time the city collection vehicle arrives at the site, the collection vehicle will leave the site and not return until the next scheduled collection day.
 - The portion of the proposed fire route located over the parking garage will be designed to support the 500 KPA loading requirement identified in NFPA 1901.
 - Fire access route has minimum 5 m overhead clearance.
 - For survey credit, refer to survey drawing prepared by Cunningham McConnell.
 - Survey Benchmark: Elevations shown hereon are geodetic and are derived from the control station 00820178040 having an elevation of 272.174m (cgvd-1928:1978).



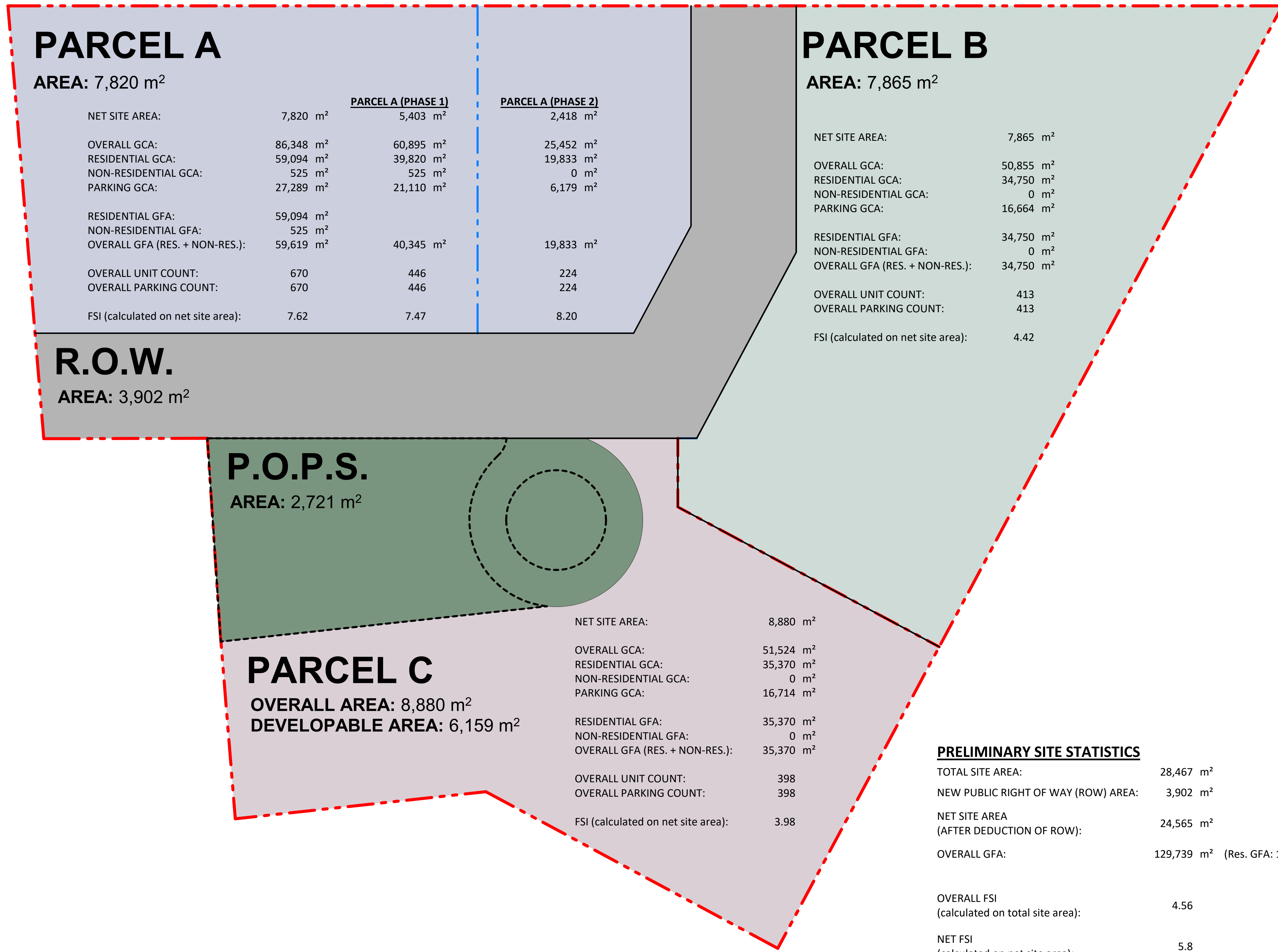
PROJECT
 130 MOUNTAINVIEW RD N
 GEORGETOWN, ON
 L7G 3P8

SEAL

Project North
 True North

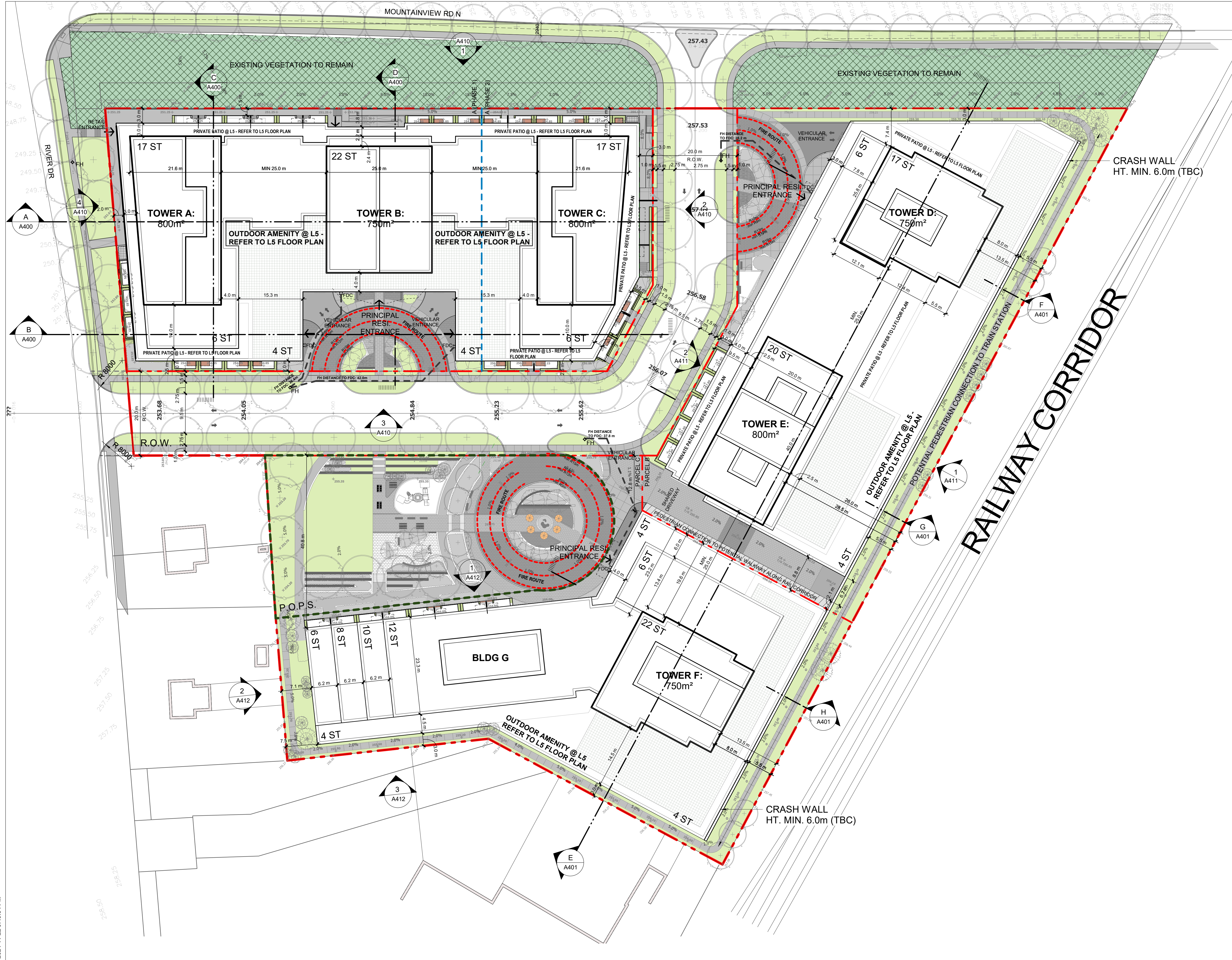
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PROJECT MGR: KE	APPROVED BY: GR

SHEET TITLE PROPOSED PROPERTY OWNERSHIP PLAN	SCALE 1:400
SHEET NUMBER A003	DATE 2024-11-22
PROJ. NO. 147388	SCALE CHECK 1/1



PRELIMINARY SITE STATISTICS

TOTAL SITE AREA:	28,467 m ²
NEW PUBLIC RIGHT OF WAY (ROW) AREA:	3,902 m ²
NET SITE AREA (AFTER DEDUCTION OF ROW):	24,565 m ²
OVERALL GFA:	129,739 m ² (Res. GFA: 129,214 m ² + Non-Res GFA: 525 m ²)
OVERALL FSI (calculated on total site area):	4.56
NET FSI (calculated on net site area):	5.8
TOTAL NUMBER OF UNITS:	1,481 (A: 670 + B: 413 + C: 398)
TOTAL NUMBER OF PARKING:	1,481 (A: 670 + B: 413 + C: 398)



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ISSUES

No	DESCRIPTION	DATE
1	Issued for OPA/ZBA	2024-11-22

- Notes:
- For Landscaping Information - Refer to Drawings prepared by Adesso Design Inc.
 - For grading and Servicing Information - Refer to Drawings prepared by Urbantech Consulting.
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 - The building will be sprinklered.
 - 10 m² bulk storage & Garbage room is provided for each parcel, H: min 2.5m clear and equipped with Hose bib, floor drain and climate control.
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 - Survey Benchmark: Elevations shown hereon are geodetic and are derived from the control station 00820178040 having an elevation of 272.174m (cgvd-1928:1978).

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PROJECT
 130 MOUNTAINVIEW RD N
 GEORGETOWN, ON
 L7G 3P8

SEAL

Project North
 True North

DRAWN BY: RT
 PROJECT MGR: KE

CHECKED BY: KE
 APPROVED BY: GR

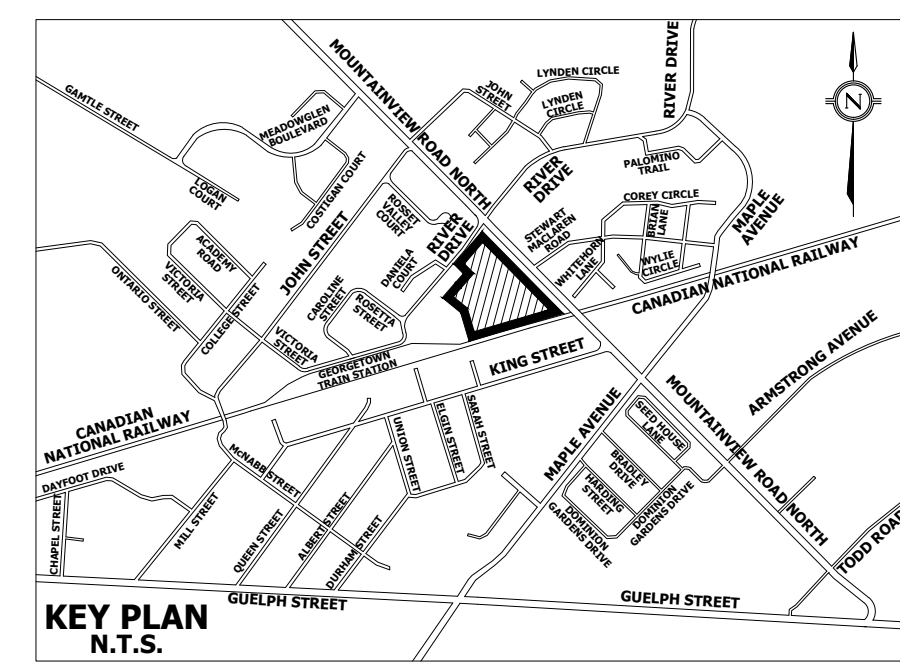
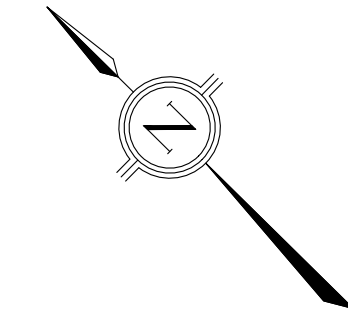
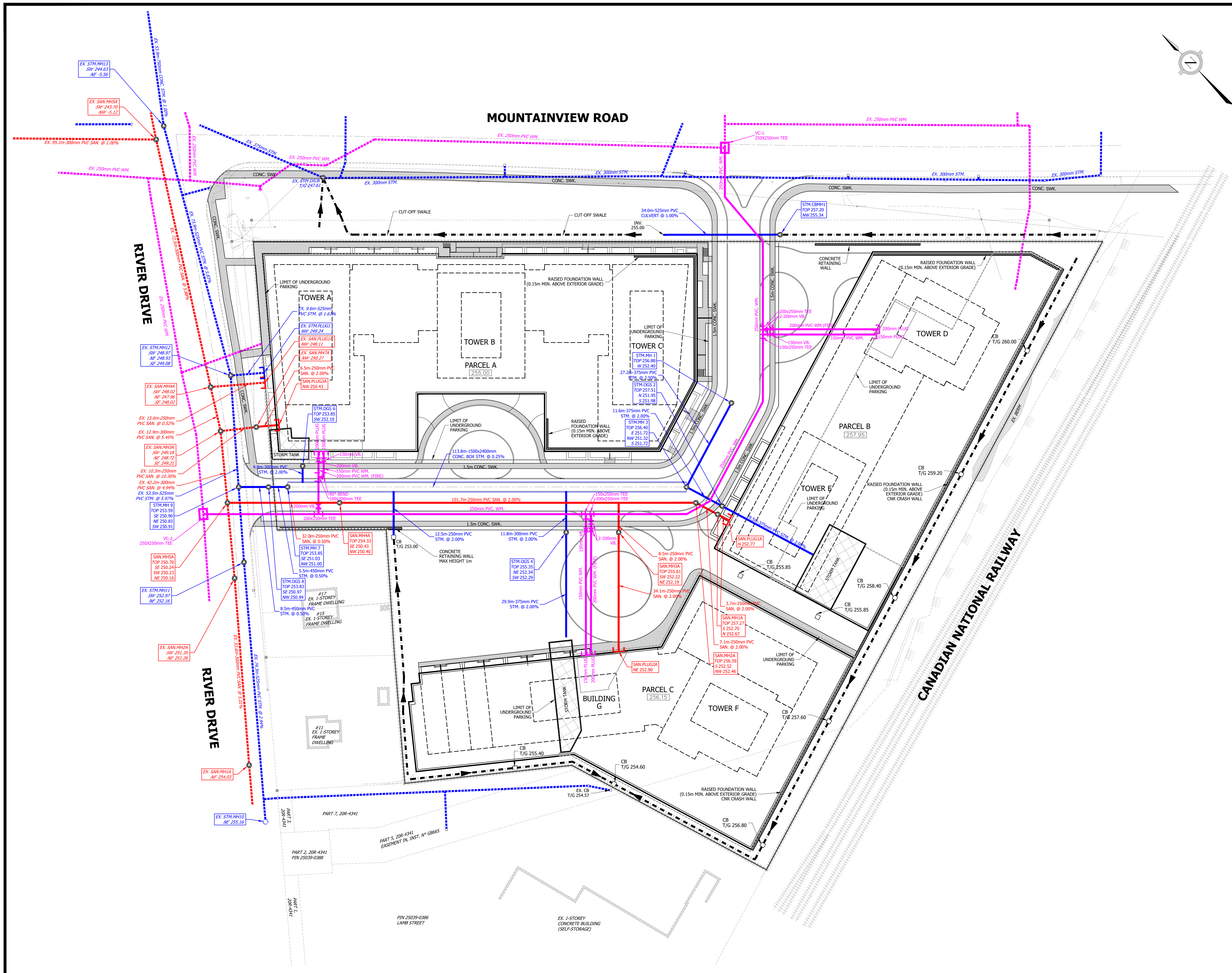
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SCALE: DATE: 2024-11-22

SHEET NUMBER: **A100** PROJ. NO: 147388

APPENDIX B

Urbantech Drawings and Figures



- LEGEND**
- LIMIT OF PROPERTY
 - PROPOSED SANITARY SEWER
 - EXISTING SANITARY SEWER
 - PROPOSED STORM SEWER
 - EXISTING STORM SEWER
 - PROPOSED WATERMAIN SEWER
 - EXISTING WATERMAIN SEWER
 - PROPOSED SWALE

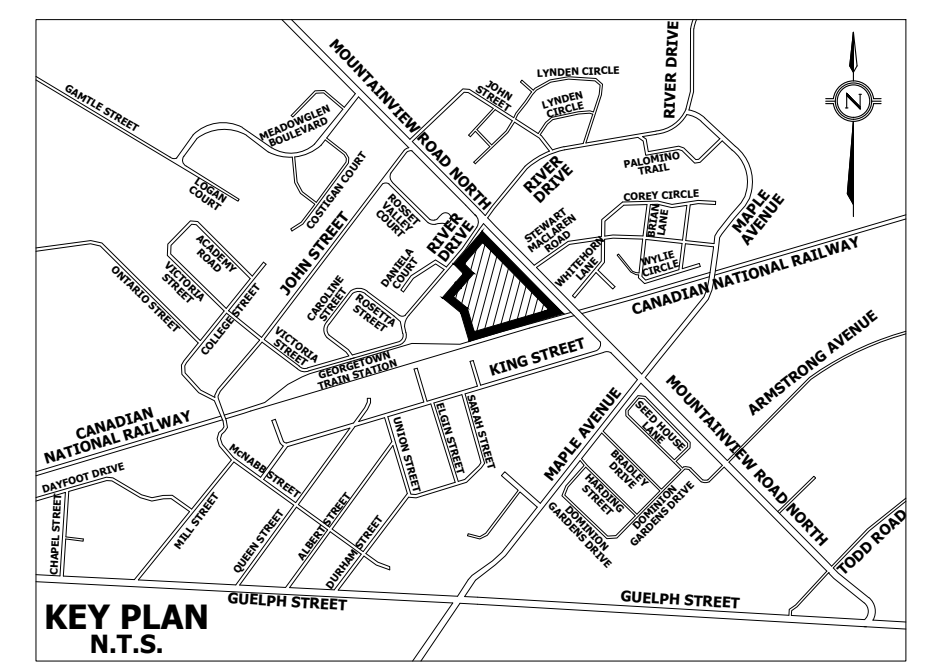
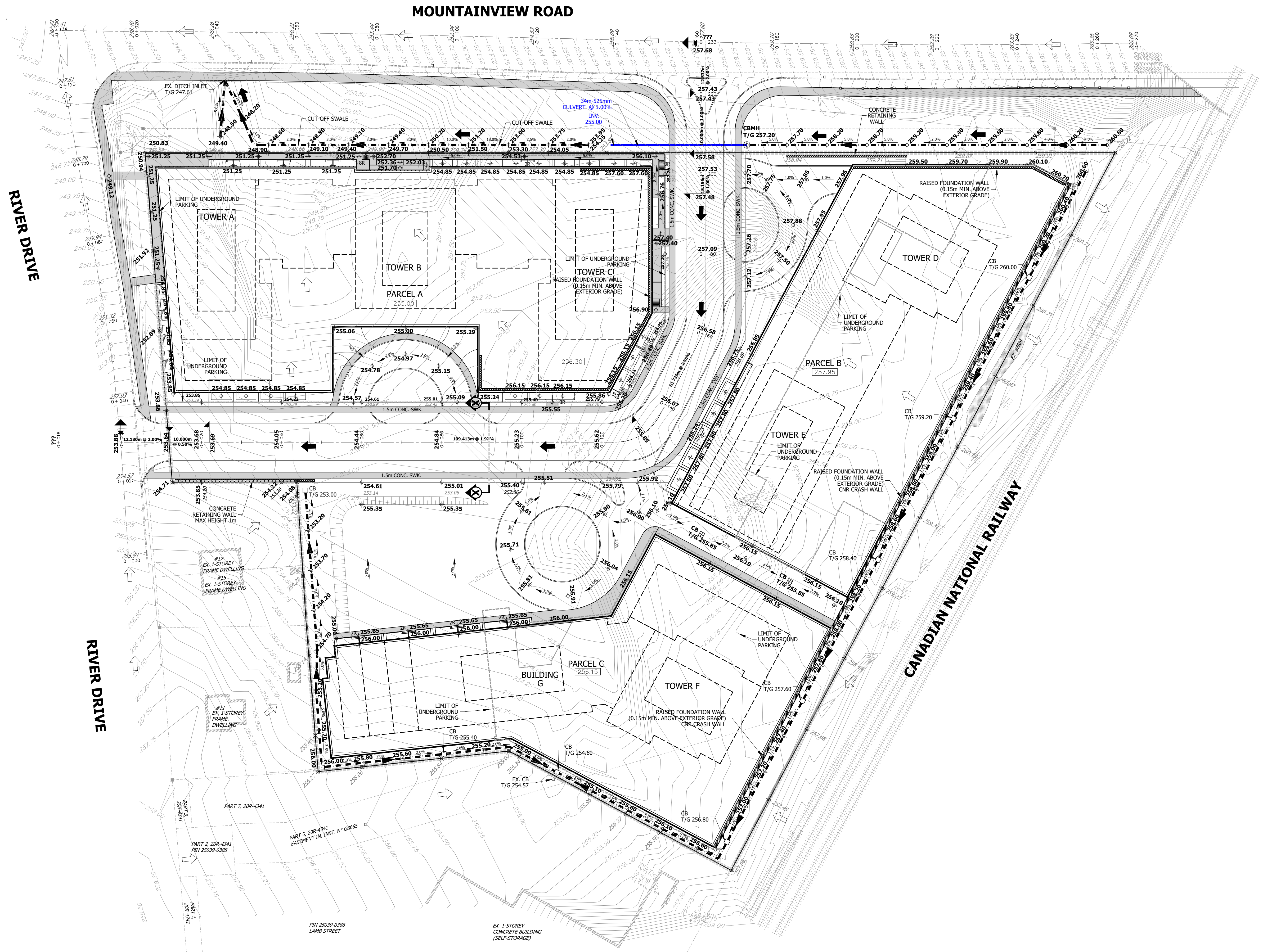
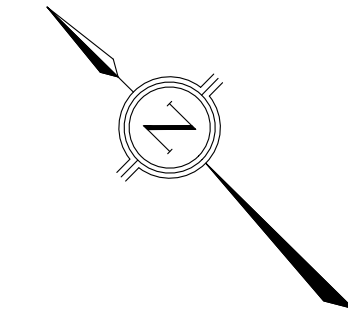
BENCHMARK NOTE
 ALL ELEVATIONS SHOWN HEREON ARE GEODETIC AND WERE DERIVED FROM THE CONTROL STATION 00R20178040
 ELEV. = 272.174m
 TOPOGRAPHIC SURVEY PREPARED BY CUNNINGHAM MCCONNELL LIMITED,
 NOVEMBER 15, 2023.

Urbantech® Consulting
 A Division of Leighton-Zec West Ltd.
 2030 Bristol Circle, Suite 105
 Oakville, ON. L6H 0H2
 TEL. 905.829.8818 • urbantech.com

130 MOUNTAINVIEW ROAD NORTH
TOWN OF HALTON HILLS

SERVICING PLAN

PROJECT No.	DATE	SCALE	DWG No.
23-763	NOV, 2024	1:500	SERV-1



LEGEND

- LIMIT OF PROPERTY
- PROPOSED ELEVATION
- EXISTING ELEVATION
- PROPOSED & EXISTING ELEVATION
- FINISH FLOOR ELEVATION
- MAXIMUM 3:1 (UNLESS OTHERWISE NOTED)
- PROPOSED OVERLAND FLOW ROUTE
- EXISTING OVERLAND FLOW ROUTE
- EXISTING CONTOUR & ELEVATION
- PROPOSED SWALE
- SECTION MARKER (REFER TO DWG.DET-1 FOR DETAILS)

BENCHMARK NOTE
ALL ELEVATIONS SHOWN HEREON ARE GEODETIC AND WERE DERIVED FROM THE CONTROL STATION 00R20178040 ELEV.=272.174m
TOPOGRAPHIC SURVEY PREPARED BY CUNNINGHAM MCCONNELL LIMITED, NOVEMBER 15, 2023.

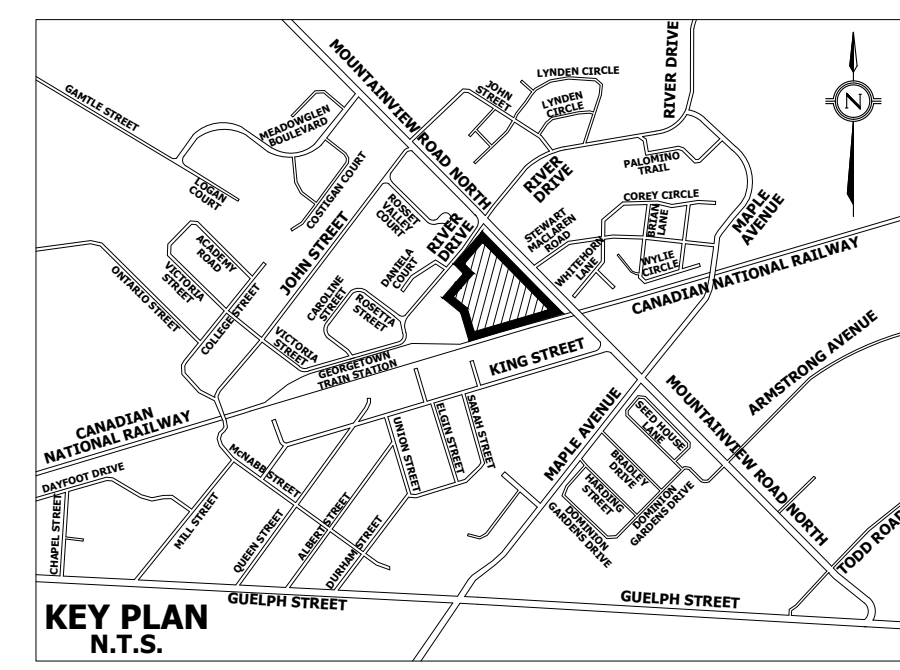
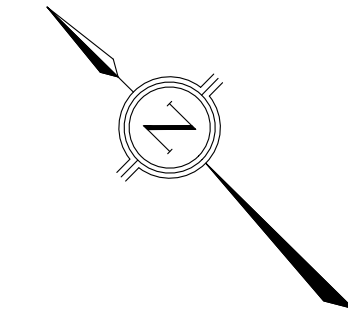
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Oakville, ON. L6H 0H2
TEL 905.829.8818 • urbantech.com

130 MOUNTAINVIEW ROAD NORTH
TOWN OF HALTON HILLS

GRADING PLAN

PROJECT No.	DATE	SCALE	DWG No.
23-763	NOV, 2024	1:500	GRD-1

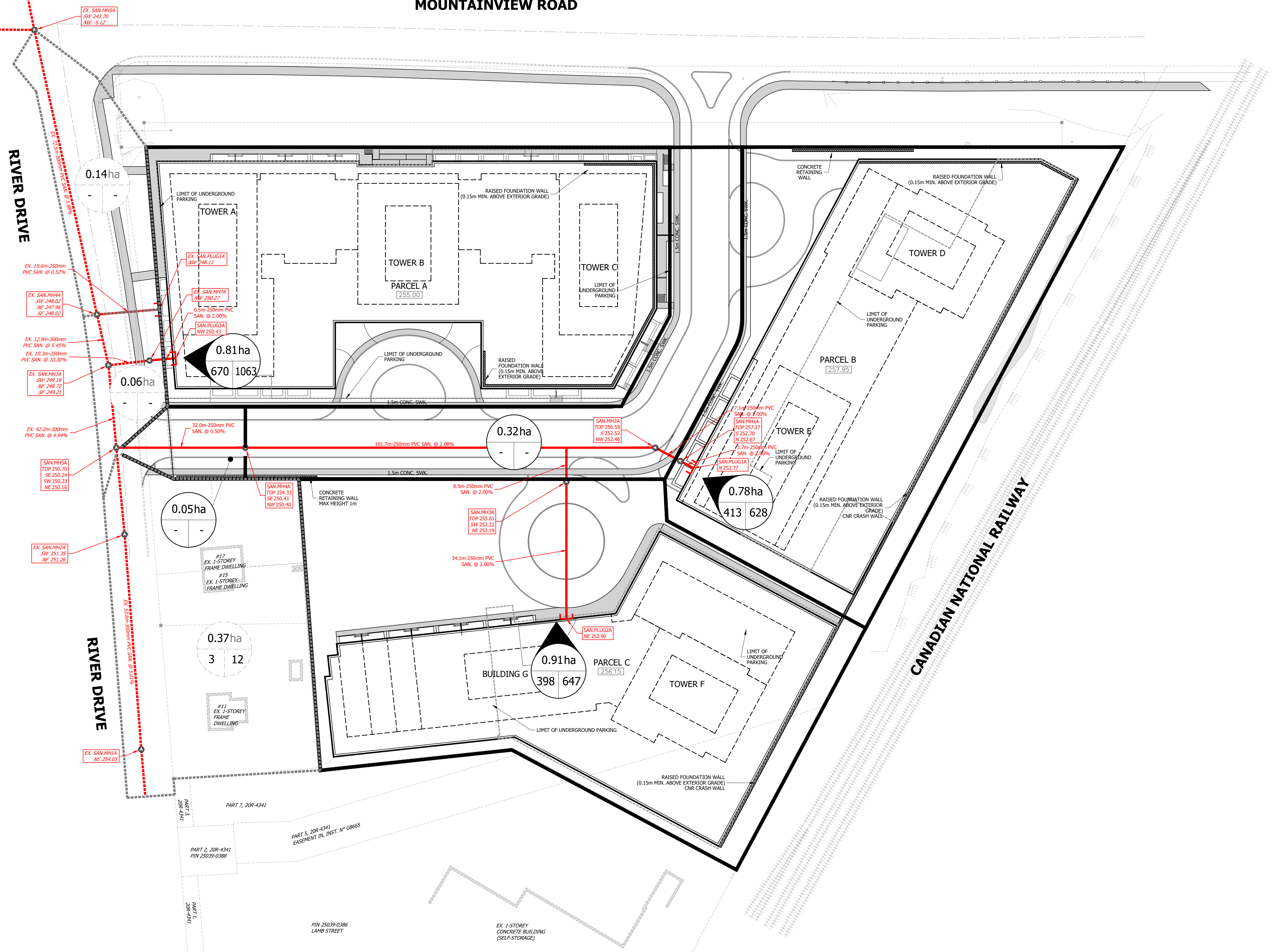
P:\PROJECTS\23-763-130 MOUNTAINVIEW RD. (GLM&C-GEORGETOWN)\DRAWINGS\200 - GRADING\GRD-1-PRELIMINARY GRADING PLAN.DWG



MOUNTAINVIEW ROAD

RIVER DRIVE

CANADIAN NATIONAL RAILWAY



LEGEND

- LIMIT OF PROPERTY
- PROPOSED SANITARY SEWER
- EXISTING SANITARY SEWER
- SANITARY MANHOLE ID
PROPOSED GROUND ELEVATION
PROPOSED SEWER INVERTS
- EXISTING SANITARY MANHOLE ID
EXISTING GROUND ELEVATION
EXISTING SEWER INVERTS
- PROPOSED DRAINAGE AREA
55 18
PROPOSED POPULATION
PROPOSED # OF UNITS
- EXISTING DRAINAGE AREA
55 18
EXISTING POPULATION
EXISTING # OF UNITS
- PROPOSED SANITARY DRAINAGE BOUNDARY
- EXISTING SANITARY DRAINAGE BOUNDARY

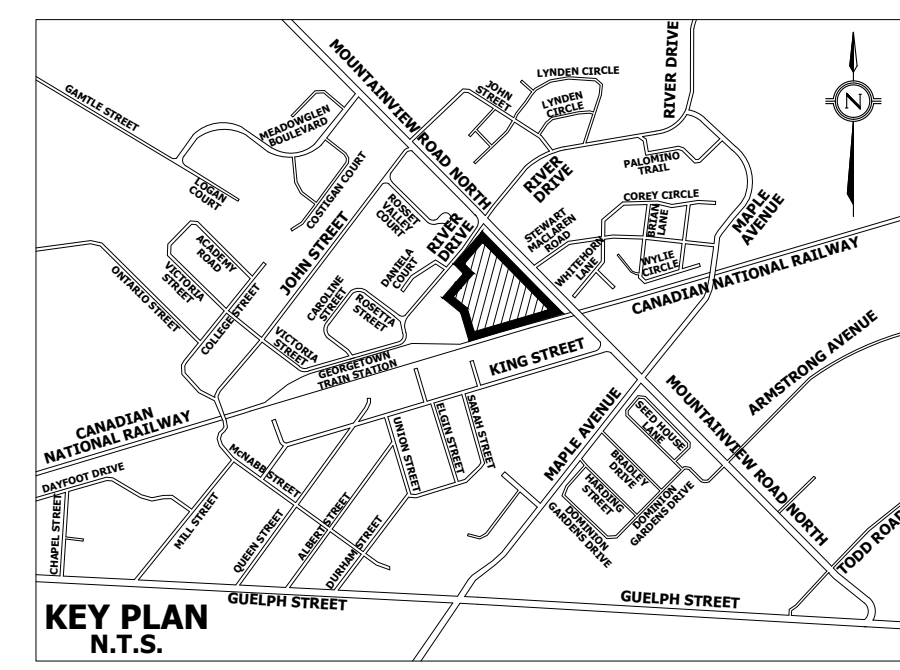
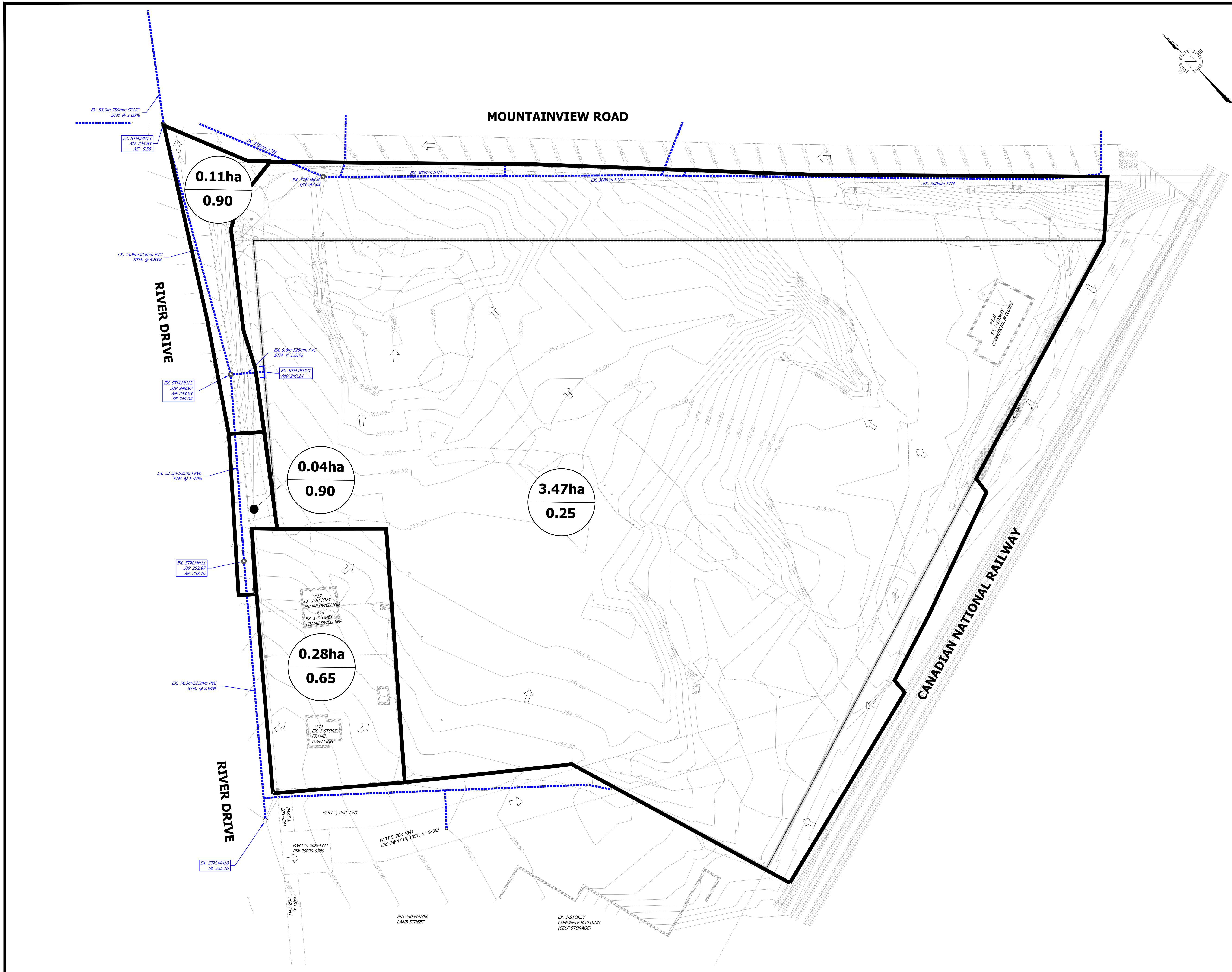
BENCHMARK NOTE
 ALL ELEVATIONS SHOWN HEREON ARE GEODETIC AND WERE DERIVED FROM THE CONTROL STATION 00R20178040
 ELEV. = 272.174m
 TOPOGRAPHIC SURVEY PREPARED BY CUNNINGHAM MCCONNELL LIMITED,
 NOVEMBER 15, 2023.

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130 MOUNTAINVIEW ROAD NORTH
TOWN OF HALTON HILLS

SANITARY DRAINAGE PLAN

PROJECT No.	DATE	SCALE	DWG No.
23-763	NOV, 2024	1:500	SAN-1



LEGEND

- LIMIT OF PROPERTY
- EXISTING STORM SEWER
- EXISTING STORM MANHOLE ID
- EXISTING GROUND ELEVATION
- EXISTING SEWER INVERTS
- EXISTING DRAINAGE AREA
- EXISTING RUNOFF COEFFICIENT
- EXISTING STORM DRAINAGE BOUNDARY
- EXISTING OVERLAND FLOW ROUTE
- EXISTING CONTOUR & ELEVATION

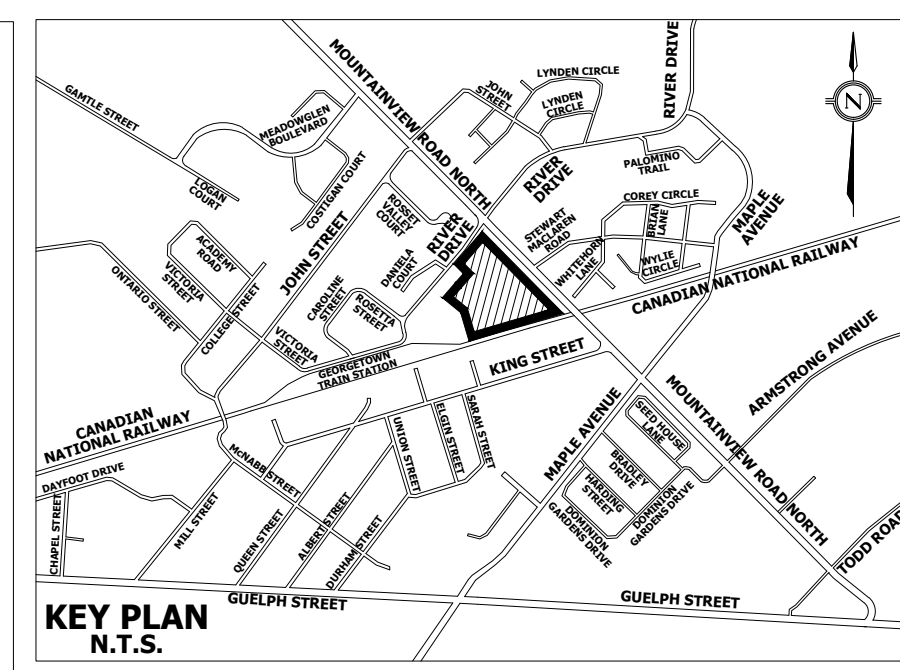
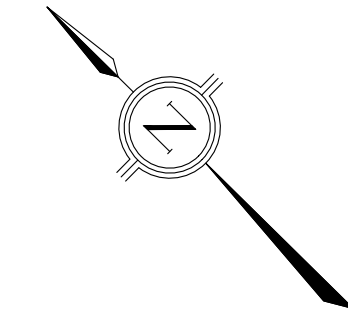
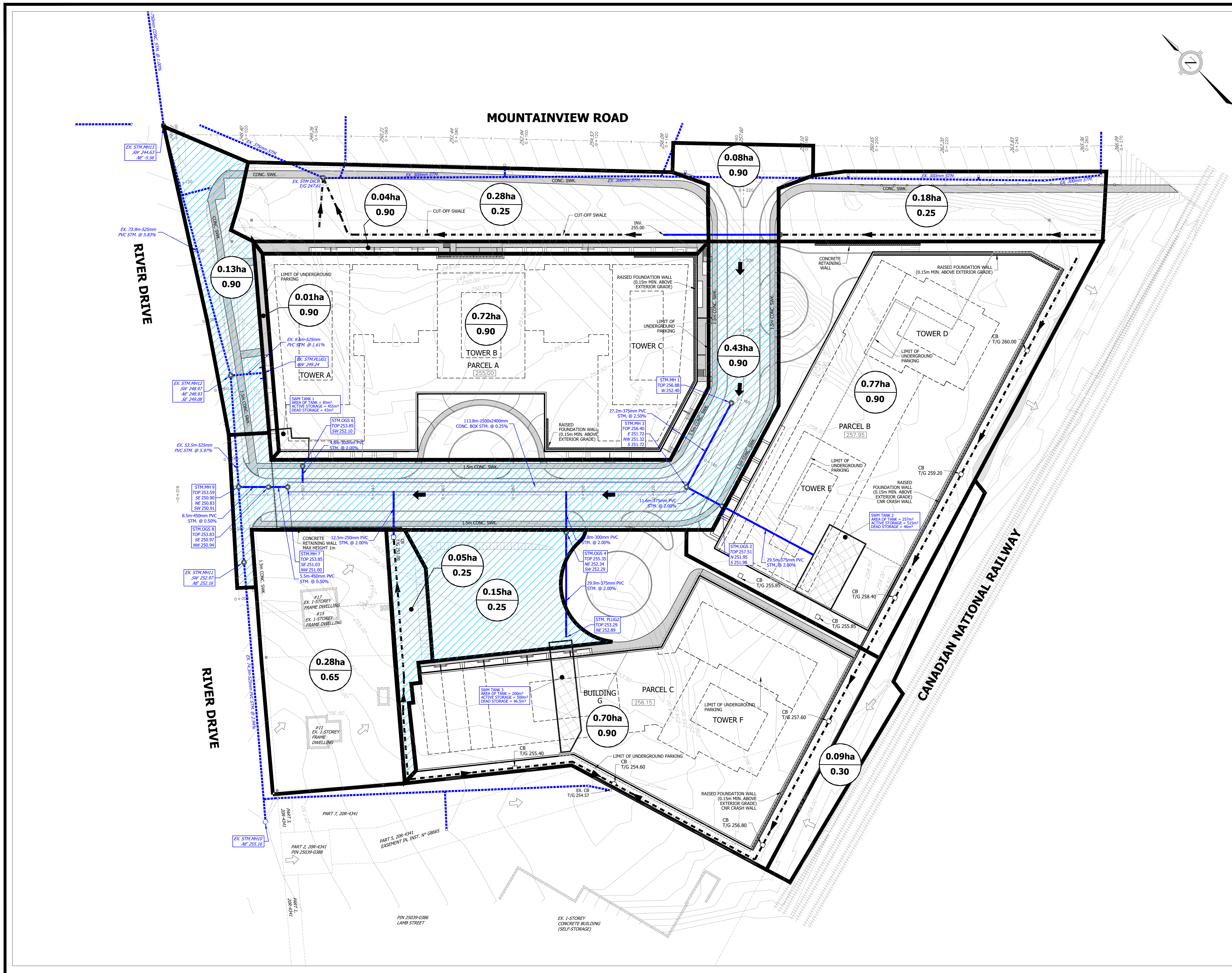
BENCHMARK NOTE
 ALL ELEVATIONS SHOWN HEREON ARE GEODETIC AND WERE DERIVED FROM THE CONTROL STATION 00R20178040 ELEV. = 272.174m
 TOPOGRAPHIC SURVEY PREPARED BY CUNNINGHAM MCCONNELL LIMITED, NOVEMBER 15, 2023.

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 Oakville, ON. L6H 0H2
 TEL 905.829.8818 • urbantech.com

**130 MOUNTAINVIEW ROAD NORTH
 TOWN OF HALTON HILLS**

**PRE-DEVELOPMENT STORM
 DRAINAGE PLAN**

PROJECT No.	DATE	SCALE	DWG No.
23-763	NOV, 2024	1:500	STM-1



LEGEND

- LIMIT OF PROPERTY
- PROPOSED STORM SEWER
- EXISTING STORM SEWER
- STORM MANHOLE ID
- PROPOSED GROUND ELEVATION
- PROPOSED SEWER INVERTS
- EXISTING STORM MANHOLE ID
- EXISTING GROUND ELEVATION
- EXISTING SEWER INVERTS
- PROPOSED DRAINAGE AREA
- PROPOSED RUNOFF COEFFICIENT
- PROPOSED STORM DRAINAGE BOUNDARY
- EXISTING OVERLAND FLOW ROUTE
- PROPOSED OVERLAND FLOW ROUTE
- EXISTING CONTOUR & ELEVATION
- PROPOSED SWALE
- MAXIMUM 3:1 (UNLESS OTHERWISE NOTED)

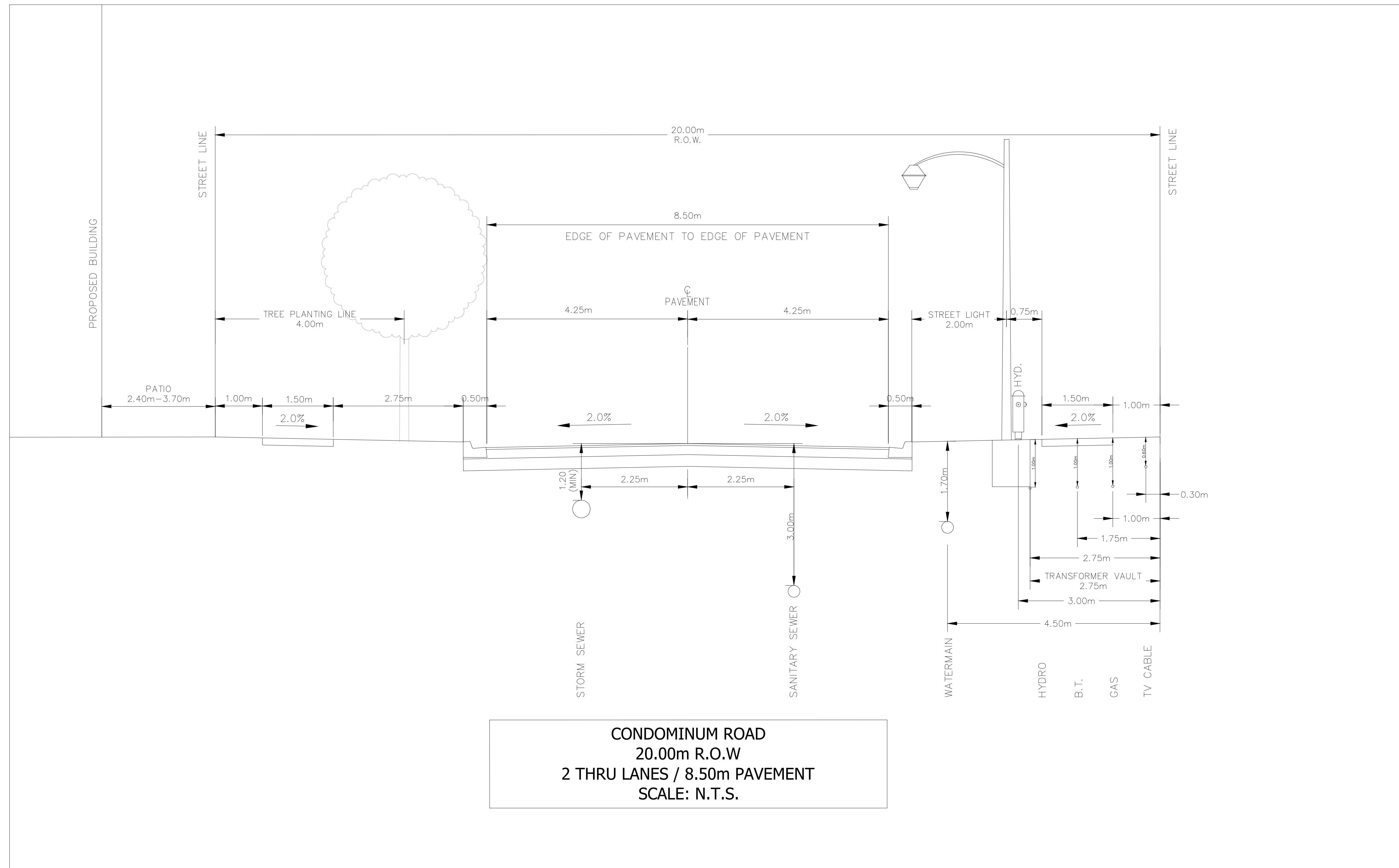
BENCHMARK NOTE
 ALL ELEVATIONS SHOWN HEREON ARE GEODETIC AND WERE DERIVED FROM THE CONTROL STATION 00R20178040 ELEV. = 272.174m
 TOPOGRAPHIC SURVEY PREPARED BY CUNNINGHAM MCCONNELL LIMITED, NOVEMBER 15, 2023.

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130 MOUNTAINVIEW ROAD NORTH
TOWN OF HALTON HILLS

POST-DEVELOPMENT STORM DRAINAGE PLAN

PROJECT No.	DATE	SCALE	DWG No.
23-763	NOV, 2024	1:500	STM-2



CONDOMINIUM ROAD
20.00m R.O.W
2 THRU LANES / 8.50m PAVEMENT
SCALE: N.T.S.

BENCHMARK NOTE
 ALL ELEVATIONS SHOWN HEREON ARE GEODETIC AND WERE DERIVED FROM THE CONTROL STATION 00820178040 ELEV.=272.174m
 TOPOGRAPHIC SURVEY PREPARED BY CUNNINGHAM MCCONNELL LIMITED, NOVEMBER 15, 2023.

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130 MOUNTAINVIEW ROAD NORTH
TOWN OF HALTON HILLS

R.O.W. DETAILS

PROJECT No.	DATE	SCALE	DWG No.
23-763	NOV, 2024	N.T.S.	DET-1

APPENDIX C

- C1 Stormwater Management Calculations
- C2 Sanitary Demand Calculations



Time to Peak Calculations											
Project Name: 130 Mountainview Rd N Municipality: Town of Halton Hills Project No.: 23-763						Prepared by: S.C. Checked by: J.P.O Date: 2024-10-16					

Sub-Catchment Name	Area (Ha)	Runoff Coefficient (C)	Length (m)	Upstream Elevation (m)	Downstream Elevation (m)	Slope Sw (%)	Tc (min)	Tp (min)	Tp (hr)	Total length (m)	Average slope (%)	Total Tp (hr)
Open Space	0.09	0.3	81.5	260	257	3.68	15	10	0.17	81.5	3.68	0.17
Park	0.14	0.25	53	254.25	252.75	2.83	14	10	0.16	53	2.83	0.16
Uncontrolled	0.18	0.25	94	261.5	256.5	5.32	15	10	0.17	94	5.32	0.17
Uncontrolled	0.28	0.25	129	256	250.5	4.26	20	13	0.22	129	4.26	0.22
Additional Area beside Park	0.05	0.25	68	256	253	4.41	14	9	0.16	68	4.41	0.16
Existing	3.75	0.25	242	258.5	248.5	4.13	27	18	0.30	242	4.13	0.30

Note: Calculation is based on the Airport Method.

1.3.3 Airport Method



For catchments where the runoff coefficient, C, is less than 0.40, the Airport formula may provide a better estimate of the time of concentration. This method was developed for airfields and calculates time of concentration as a function of runoff coefficient, length, and slope as follows:

$$t_c = \frac{3.26 * (1.1 - C) * L^{0.5}}{S_w^{0.33}} \quad (2)$$

where:
 t_c = time of concentration (min)
 C = runoff coefficient
 L = catchment length, (m)
 S_w = catchment slope (%)

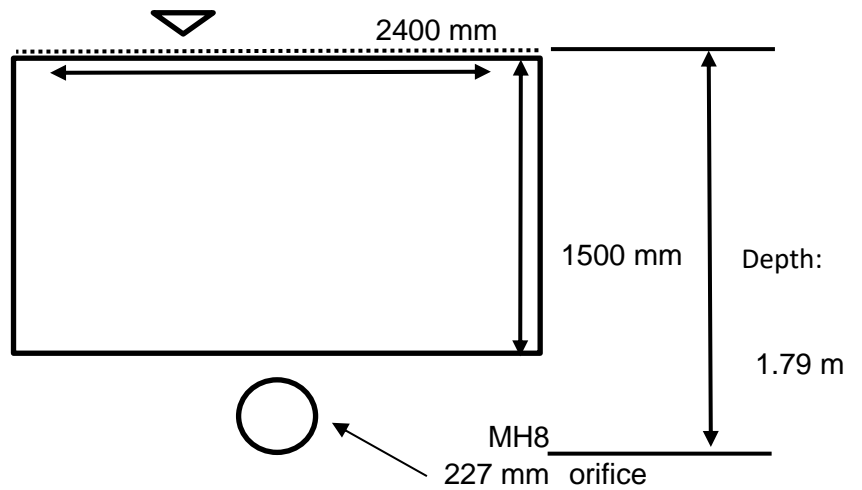
CALCULATION OF STORAGE SIZE AND RATING CURVE BASED ON ORIFICE SIZE
Super Pipe



Project Name: 130 Mountainview Rd N
Municipality: Town of Halton Hills
Project No.: 23-763
Date: 15-Oct-24

Prepared by: J.P.O
Checked by: S.H
Submission #: 1

Elevation (m)	Height (m)	Volume (m ³)	Flow rate (m ³ /s)
100.00	0.00	0.00	0.000
100.89	0.89	206.46	0.098
101.79	1.79	412.92	0.144



Calculation of Orifice	
$A = \pi D^2 \div 4$	0.040 m ²
$h = \text{Depth of Tank} - \left(\frac{D}{2}\right)$	1.67 m
C	0.6
$2g = 2 \times 9.81$	19.62 m/s ²
$Q = CA\sqrt{2gh}$	0.144 m ³ /s
Q target (from VH model)	0.143 m³/s
Q and Q target are matched	
Therefore, orifice is 227 mm	

Name	Description	Result
SUPER PIPE	Depth	1.79 m
	Provided Volume	413 m ³
	Required Volume from VO6	413 m ³

Invert: **100.00 m**
Orifice size: **227 mm**

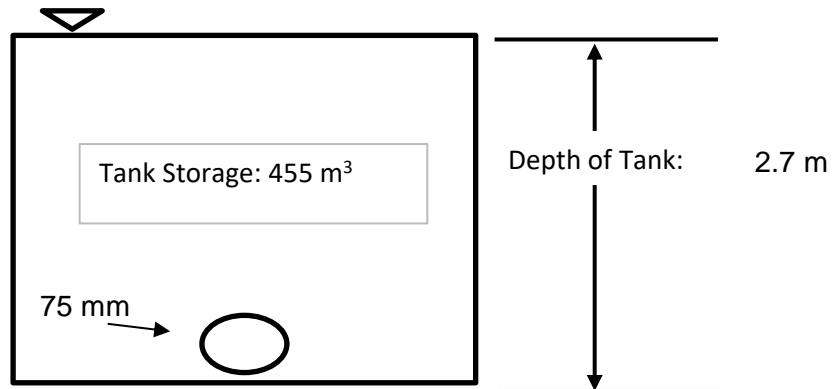
**CALCULATION OF STORAGE SIZE AND RATING CURVE BASED ON ORIFICE SIZE
TANK 1**



Project Name: 130 Mountainview Rd N
Municipality: Town of Halton Hills
Project No.: 23-763
Date: 15-Oct-24

Prepared by: J.P.O
Checked by: S.H.
Submission #: 1

Elevation (m)	Height (m)	Volume (m ³)	Flow rate (m ³ /s)
84.10	0.00	0.00	0.000
84.25	0.15	25.47	0.000
84.40	0.30	50.93	0.008
84.55	0.45	76.40	0.010
84.70	0.60	101.87	0.012
84.85	0.75	127.33	0.014
85.00	0.90	152.80	0.015
85.15	1.05	178.26	0.016
85.30	1.20	203.73	0.017
85.45	1.35	229.20	0.018
85.60	1.50	254.66	0.019
85.75	1.65	280.13	0.020
85.90	1.80	305.60	0.021
86.05	1.95	331.06	0.022
86.20	2.10	356.53	0.023
86.35	2.25	382.00	0.024
86.50	2.40	407.46	0.025
86.65	2.55	432.93	0.025
86.78	2.68	455.00	0.026



Calculation of Orifice

$$A = \pi D^2 \div 4 = 0.004 \text{ m}^2$$

$$h = \text{Depth of Tank} - \left(\frac{D}{2}\right) = 2.643 \text{ m}$$

$$(1000\text{mm}) - \left(\frac{123}{2}\right) = 2.643 \text{ m}$$

$$C = 0.6$$

$$2g = 2 \times 9.81 = 19.62 \text{ m/s}^2$$

$$Q = CA\sqrt{2gh} = 0.026 \text{ m}^3/\text{s}$$

Q target (from VH model) = **0.026 m³/s**
 Q and Q target are matched
 Therefore, orifice is 75 mm

Invert: **100.00 m***
 Orifice size: **75 mm**

Name	Description	Result
TANK 1	Quantity control only	
	Storage tank footprint	170 m²
	Depth of Storage Tank	2.70 m
	Required Volume from VO6	455 m ³
	*Actual invert elevation currently undetermined	

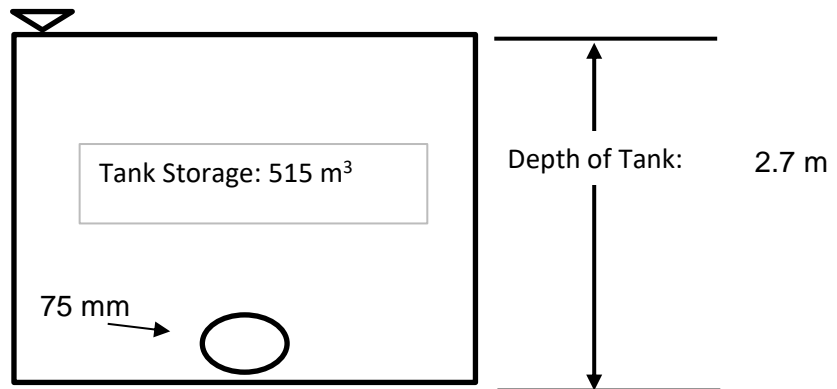
**CALCULATION OF STORAGE SIZE AND RATING CURVE BASED ON ORIFICE SIZE
TANK 2**



Project Name: 130 Mountainview Rd N
Municipality: Town of Halton Hills
Project No.: 23-763
Date: 15-Oct-24

Prepared by: J.P.O
Checked by: S.H.
Submission #: 1

Elevation (m)	Height (m)	Volume (m ³)	Flow rate (m ³ /s)
84.10	0.00	0.00	0.000
84.25	0.15	27.87	0.000
84.40	0.30	55.75	0.008
84.55	0.45	83.62	0.010
84.70	0.60	111.49	0.012
84.85	0.75	139.37	0.014
85.00	0.90	167.24	0.015
85.15	1.05	195.11	0.016
85.30	1.20	222.99	0.017
85.45	1.35	250.86	0.018
85.60	1.50	278.73	0.019
85.75	1.65	306.60	0.020
85.90	1.80	334.48	0.021
86.05	1.95	362.35	0.022
86.20	2.10	390.22	0.023
86.35	2.25	418.10	0.024
86.50	2.40	445.97	0.025
86.65	2.55	473.84	0.025
86.78	2.68	498.00	0.026



Calculation of Orifice

$$A = \pi D^2 \div 4 = 0.004 \text{ m}^2$$

$$h = \text{Depth of Tank} - \left(\frac{D}{2}\right) = 2.643 \text{ m}$$

$$(1000\text{mm}) - \left(\frac{123}{2}\right) = 2.643 \text{ m}$$

$$C = 0.82$$

$$2g = 2 \times 9.81 = 19.62 \text{ m/s}^2$$

$$Q = CA\sqrt{2gh} = 0.026 \text{ m}^3/\text{s}$$

Q target (from VH model) = **0.026 m³/s**
 Q and Q target are matched
 Therefore, orifice is 75 mm

Invert: **100.00 m***
 Orifice size: **75 mm**

Name	Description	Result
TANK 2	Quantity control only	
	Storage tank footprint	186 m²
	Depth of Storage Tank	2.70 m
	Required Volume from VO6	498 m ³
	*Actual invert elevation currently undetermined	

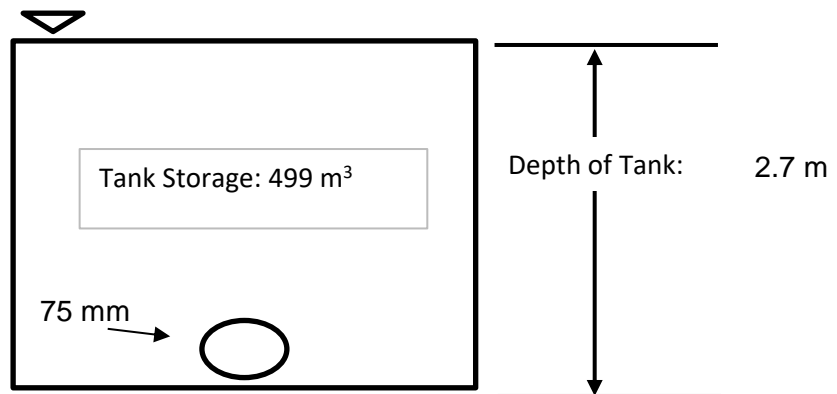
**CALCULATION OF STORAGE SIZE AND RATING CURVE BASED ON ORIFICE SIZE
TANK 3**



Project Name: 130 Mountainview Rd N
Municipality: Town of Halton Hills
Project No.: 23-763
Date: 15-Oct-24

Prepared by: J.P.O
Checked by: S.H.
Submission #: 1

Elevation (m)	Height (m)	Volume (m³)	Flow rate (m³/s)
84.10	0.00	0.00	0.000
84.25	0.15	27.93	0.000
84.40	0.30	55.86	0.008
84.55	0.45	83.79	0.010
84.70	0.60	111.72	0.012
84.85	0.75	139.65	0.014
85.00	0.90	167.57	0.015
85.15	1.05	195.50	0.016
85.30	1.20	223.43	0.017
85.45	1.35	251.36	0.018
85.60	1.50	279.29	0.019
85.75	1.65	307.22	0.020
85.90	1.80	335.15	0.021
86.05	1.95	363.08	0.022
86.20	2.10	391.01	0.023
86.35	2.25	418.94	0.024
86.50	2.40	446.87	0.025
86.65	2.55	474.79	0.025
86.78	2.68	499.00	0.026



Calculation of Orifice

$$A = \pi D^2 \div 4 = 0.004 \text{ m}^2$$

$$h = \text{Depth of Tank} - \left(\frac{D}{2}\right) = 2.643 \text{ m}$$

$$(1000\text{mm}) - \left(\frac{123}{2}\right) = 2.643 \text{ m}$$

$$C = 0.82$$

$$2g = 2 \times 9.81 = 19.62 \text{ m/s}^2$$

$$Q = CA\sqrt{2gh} = 0.026 \text{ m}^3/\text{s}$$

Q target (from VH model) = **0.026 m³/s**
 Q and Q target are matched
 Therefore, orifice is 75 mm

Invert: **100.00 m***
 Orifice size: **75 mm**

Name	Description	Result
TANK 3	Quantity control only	
	Storage tank footprint	186 m²
	Depth of Storage Tank	2.70 m
	Required Volume from VO6	499 m³
	*Actual invert elevation currently undetermined	

Stormceptor® EF Sizing Report

Imbrium® Systems

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

10/17/2024

Province:	Ontario
City:	Halton Hills
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	130 Mountainview Rd N
Project Number:	65993
Designer Name:	Shania Chhom
Designer Company:	Urbantech
Designer Email:	schhom@urbantech.com
Designer Phone:	905-829-6911
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	Tank 1
------------	--------

Drainage Area (ha):	0.72
---------------------	------

% Imperviousness:	99.00
-------------------	-------

Runoff Coefficient 'c': 0.89

Particle Size Distribution:	CA ETV
-----------------------------	--------

Target TSS Removal (%):	60.0
-------------------------	------

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	20.02
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	Yes
Upstream Orifice Control Flow Rate to Stormceptor (L/s):	26.00
Peak Conveyance (maximum) Flow Rate (L/s):	
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	525
Estimated Average Annual Sediment Volume (L/yr):	427

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EFO4	53
EFO6	60
EFO8	64
EFO10	67
EFO12	69

Recommended Stormceptor EFO Model: **EFO6**

Estimated Net Annual Sediment (TSS) Load Reduction (%): **60**

Water Quality Runoff Volume Capture (%): **> 90**



Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► Stormceptor® EF and Stormceptor® EFO are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► Stormceptor® EF and EFO remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The Canadian ETV PSD shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5



Stormceptor® EF Sizing Report

Upstream Flow Controlled Results

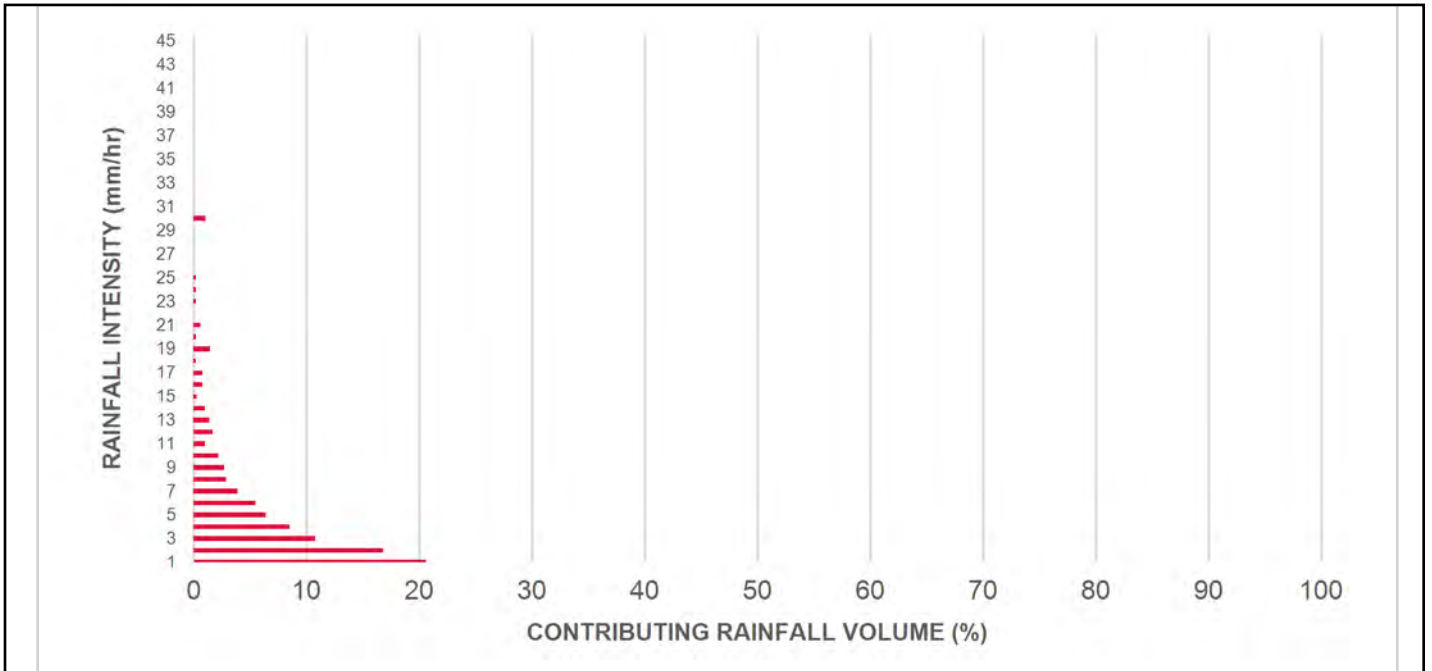
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.5	8.5	0.89	54.0	20.0	70	6.0	6.0
1.00	20.6	29.1	1.79	107.0	41.0	70	14.5	20.5
2.00	16.8	45.9	3.58	215.0	82.0	64	10.8	31.3
3.00	10.8	56.7	5.37	322.0	122.0	61	6.5	37.8
4.00	8.5	65.2	7.16	429.0	163.0	57	4.8	42.6
5.00	6.4	71.6	8.95	537.0	204.0	54	3.5	46.1
6.00	5.5	77.0	10.74	644.0	245.0	53	2.9	49.0
7.00	3.9	81.0	12.53	752.0	286.0	52	2.0	51.0
8.00	2.9	83.9	14.32	859.0	327.0	50	1.5	52.5
9.00	2.7	86.5	16.10	966.0	367.0	49	1.3	53.8
10.00	2.2	88.7	17.89	1074.0	408.0	48	1.0	54.8
11.00	1.0	89.7	19.68	1181.0	449.0	47	0.5	55.3
12.00	1.7	91.3	21.47	1288.0	490.0	45	0.8	56.0
13.00	1.4	92.8	23.26	1396.0	531.0	44	0.6	56.7
14.00	7.2	100.0	25.05	1503.0	572.0	43	3.1	59.8
15.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
16.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
17.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
18.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
19.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
20.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
21.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
22.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
23.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
24.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
25.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
30.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
35.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
40.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
45.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.8
Estimated Net Annual Sediment (TSS) Load Reduction =								60 %

Climate Station ID: 6158731 Years of Rainfall Data: 20

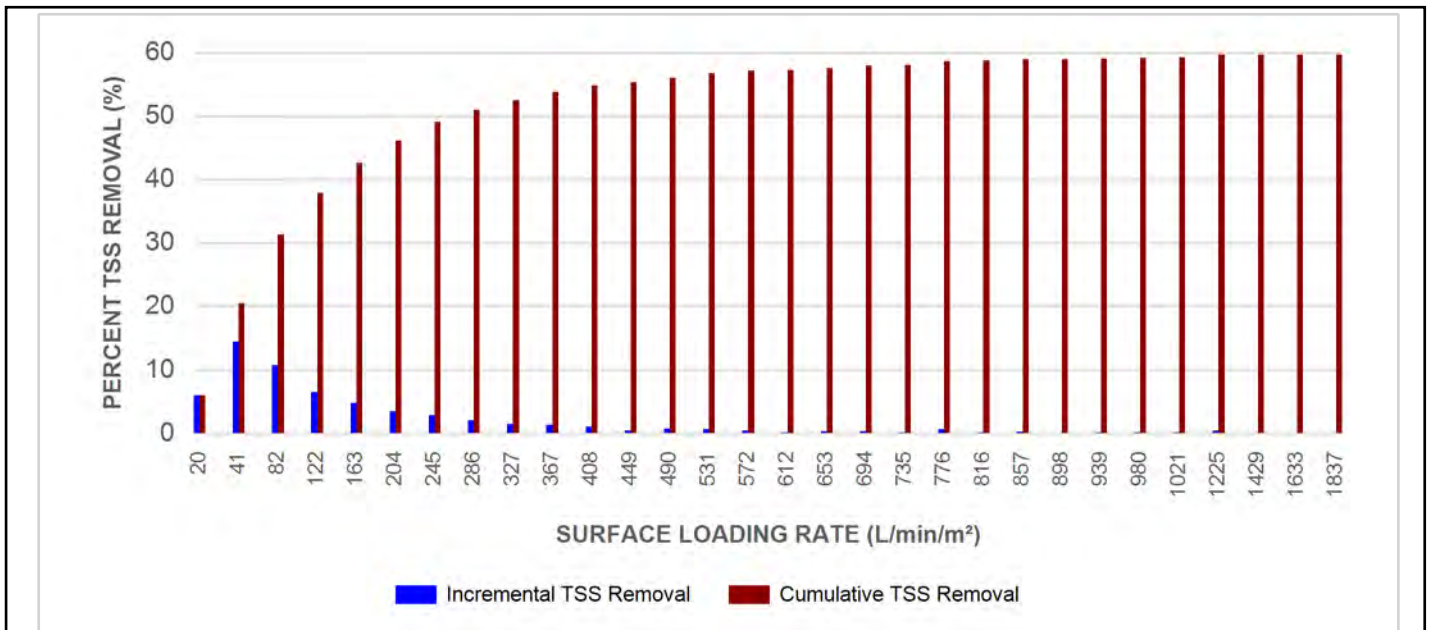


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

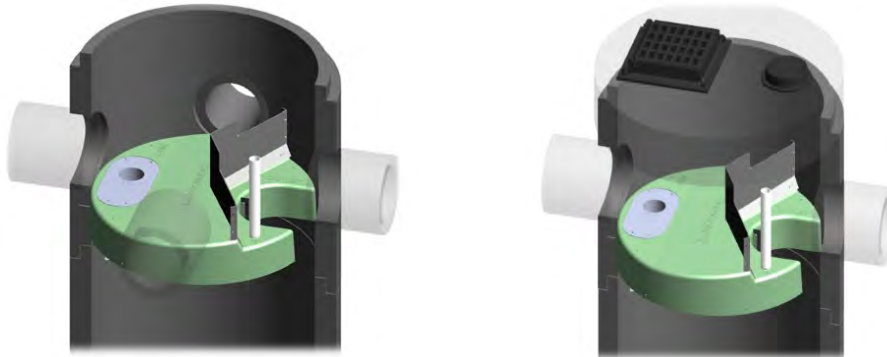
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

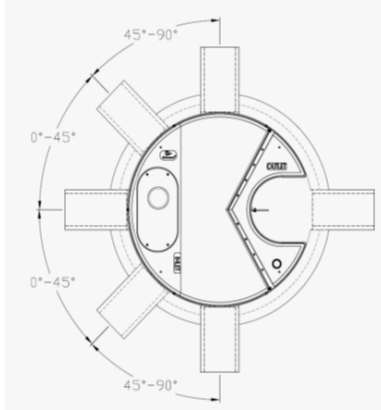
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure.

The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>



Stormceptor® EF Sizing Report

Table of TSS Removal vs Surface Loading Rate Based on Third-Party Test Results
Stormceptor® EFO

SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL
1	70	660	42	1320	35	1980	24
30	70	690	42	1350	35	2010	24
60	67	720	41	1380	34	2040	23
90	63	750	41	1410	34	2070	23
120	61	780	41	1440	33	2100	23
150	58	810	41	1470	32	2130	22
180	56	840	41	1500	32	2160	22
210	54	870	41	1530	31	2190	22
240	53	900	41	1560	31	2220	21
270	52	930	40	1590	30	2250	21
300	51	960	40	1620	29	2280	21
330	50	990	40	1650	29	2310	21
360	49	1020	40	1680	28	2340	20
390	48	1050	39	1710	28	2370	20
420	47	1080	39	1740	27	2400	20
450	47	1110	38	1770	27	2430	20
480	46	1140	38	1800	26	2460	19
510	45	1170	37	1830	26	2490	19
540	44	1200	37	1860	26	2520	19
570	43	1230	37	1890	25	2550	19
600	42	1260	36	1920	25	2580	18
630	42	1290	36	1950	24	2600	26



**STANDARD PERFORMANCE SPECIFICATION FOR
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall



Stormceptor® EF Sizing Report

remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

Stormceptor® **EF** Sizing Report

assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.

Stormceptor® EF Sizing Report

Imbrium® Systems

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

10/17/2024

Province:	Ontario
City:	Halton Hills
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	130 Mountainview Rd N
Project Number:	65993
Designer Name:	Shania Chhom
Designer Company:	Urbantech
Designer Email:	schhom@urbantech.com
Designer Phone:	905-829-6911
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	Tank 2
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Drainage Area (ha):	0.77
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% Imperviousness:	99.00
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Runoff Coefficient 'c': 0.89

Particle Size Distribution:	CA ETV
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Target TSS Removal (%):	60.0
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Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	21.41
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	Yes
Upstream Orifice Control Flow Rate to Stormceptor (L/s):	26.00
Peak Conveyance (maximum) Flow Rate (L/s):	
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	561
Estimated Average Annual Sediment Volume (L/yr):	456

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EFO4	52
EFO6	60
EFO8	64
EFO10	66
EFO12	69

Recommended Stormceptor EFO Model: **EFO6**

Estimated Net Annual Sediment (TSS) Load Reduction (%): **60**

Water Quality Runoff Volume Capture (%): **> 90**



Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► Stormceptor® EF and Stormceptor® EFO are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► Stormceptor® EF and EFO remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The Canadian ETV PSD shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5



Stormceptor® EF Sizing Report

Upstream Flow Controlled Results

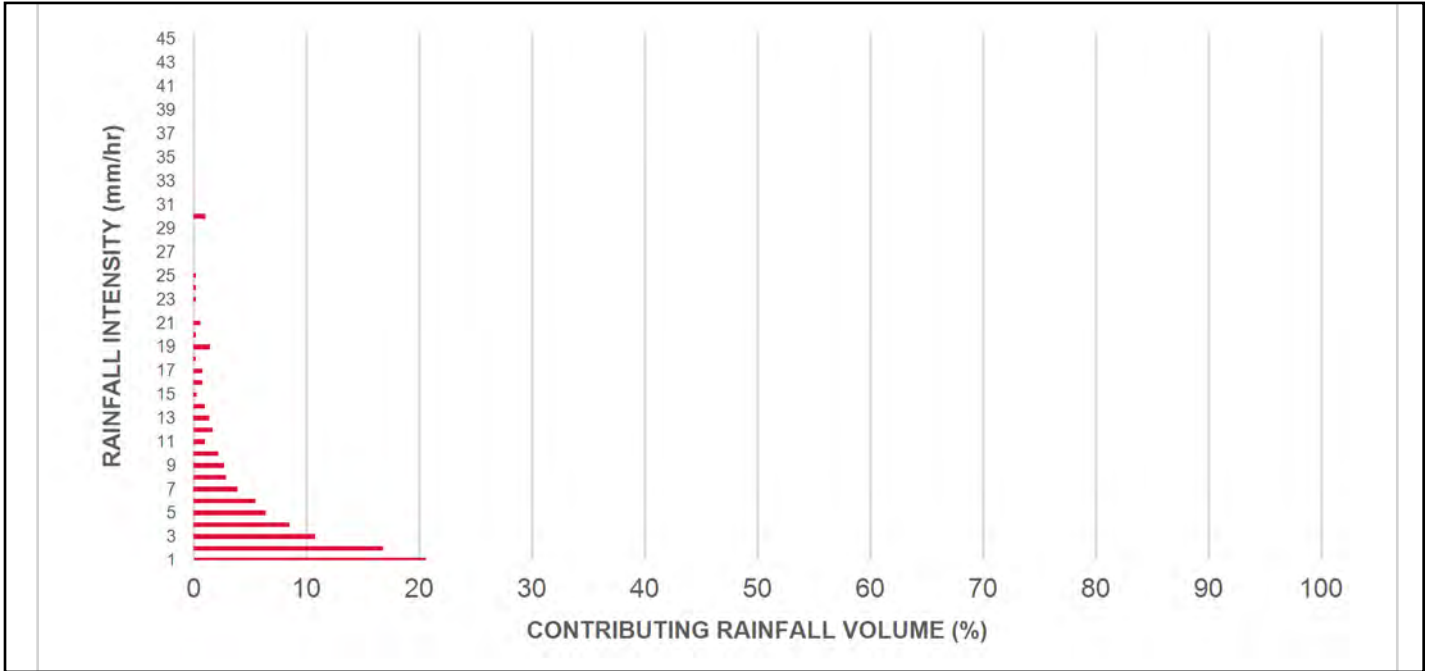
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.5	8.5	0.96	57.0	22.0	70	6.0	6.0
1.00	20.6	29.1	1.91	115.0	44.0	70	14.5	20.5
2.00	16.8	45.9	3.83	230.0	87.0	64	10.8	31.3
3.00	10.8	56.7	5.74	344.0	131.0	60	6.4	37.7
4.00	8.5	65.2	7.65	459.0	175.0	57	4.8	42.5
5.00	6.4	71.6	9.57	574.0	218.0	54	3.4	45.9
6.00	5.5	77.0	11.48	689.0	262.0	52	2.8	48.8
7.00	3.9	81.0	13.40	804.0	306.0	51	2.0	50.8
8.00	2.9	83.9	15.31	919.0	349.0	50	1.4	52.2
9.00	2.7	86.5	17.22	1033.0	393.0	48	1.3	53.5
10.00	2.2	88.7	19.14	1148.0	437.0	47	1.0	54.5
11.00	1.0	89.7	21.05	1263.0	480.0	46	0.4	55.0
12.00	1.7	91.3	22.96	1378.0	524.0	44	0.7	55.7
13.00	8.7	100.0	24.88	1493.0	568.0	43	3.7	59.5
14.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
15.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
16.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
17.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
18.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
19.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
20.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
21.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
22.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
23.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
24.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
25.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
30.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
35.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
40.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
45.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.5
Estimated Net Annual Sediment (TSS) Load Reduction =								59 %

Climate Station ID: 6158731 Years of Rainfall Data: 20

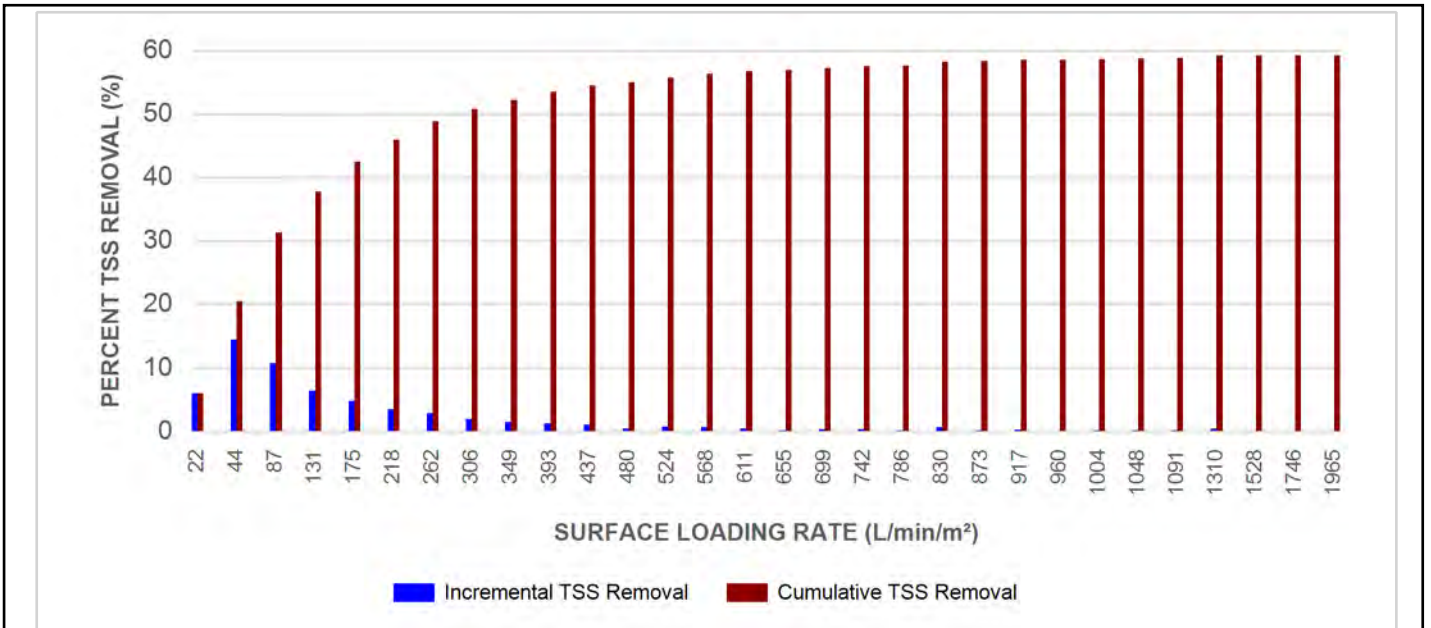


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

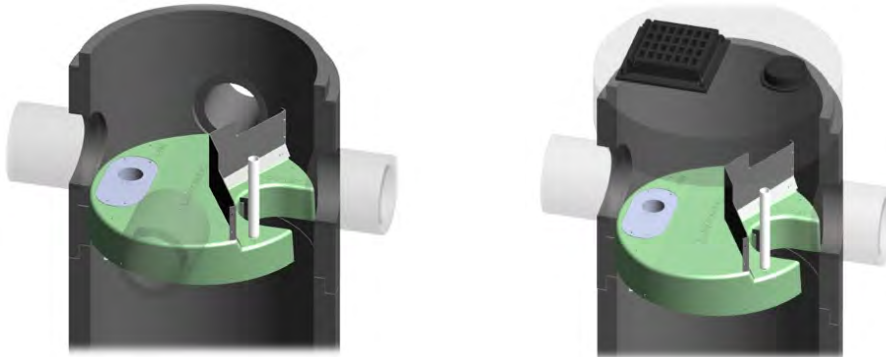
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

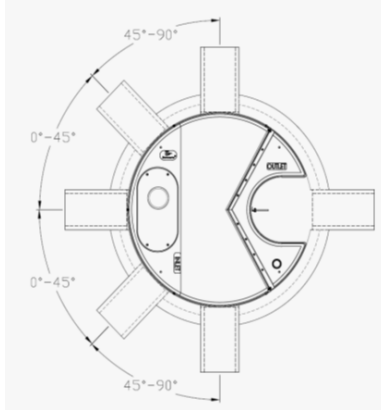
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure.

The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>



Stormceptor® EF Sizing Report

Table of TSS Removal vs Surface Loading Rate Based on Third-Party Test Results
Stormceptor® EFO

SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL
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150	58	810	41	1470	32	2130	22
180	56	840	41	1500	32	2160	22
210	54	870	41	1530	31	2190	22
240	53	900	41	1560	31	2220	21
270	52	930	40	1590	30	2250	21
300	51	960	40	1620	29	2280	21
330	50	990	40	1650	29	2310	21
360	49	1020	40	1680	28	2340	20
390	48	1050	39	1710	28	2370	20
420	47	1080	39	1740	27	2400	20
450	47	1110	38	1770	27	2430	20
480	46	1140	38	1800	26	2460	19
510	45	1170	37	1830	26	2490	19
540	44	1200	37	1860	26	2520	19
570	43	1230	37	1890	25	2550	19
600	42	1260	36	1920	25	2580	18
630	42	1290	36	1950	24	2600	26



**STANDARD PERFORMANCE SPECIFICATION FOR
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall



Stormceptor® EF Sizing Report

remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

Stormceptor® EF Sizing Report

assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.

Stormceptor® EF Sizing Report

Imbrium® Systems

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

10/17/2024

Province:	Ontario
City:	Halton Hills
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	130 Mountainview Rd N
Project Number:	65993
Designer Name:	Shania Chhom
Designer Company:	Urbantech
Designer Email:	schhom@urbantech.com
Designer Phone:	905-829-6911
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	Tank 3
------------	--------

Drainage Area (ha):	0.80
Runoff Coefficient 'c':	0.83

Particle Size Distribution:	CA ETV
Target TSS Removal (%):	60.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	20.65
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	Yes
Upstream Orifice Control Flow Rate to Stormceptor (L/s):	26.00
Peak Conveyance (maximum) Flow Rate (L/s):	
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	520
Estimated Average Annual Sediment Volume (L/yr):	423

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EFO4	52
EFO6	60
EFO8	64
EFO10	67
EFO12	69

Recommended Stormceptor EFO Model: **EFO6**

Estimated Net Annual Sediment (TSS) Load Reduction (%): **60**

Water Quality Runoff Volume Capture (%): **> 90**



Stormceptor® **EF** Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► Stormceptor® EF and Stormceptor® EFO are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► Stormceptor® EF and EFO remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The Canadian ETV PSD shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5



Stormceptor® EF Sizing Report

Upstream Flow Controlled Results

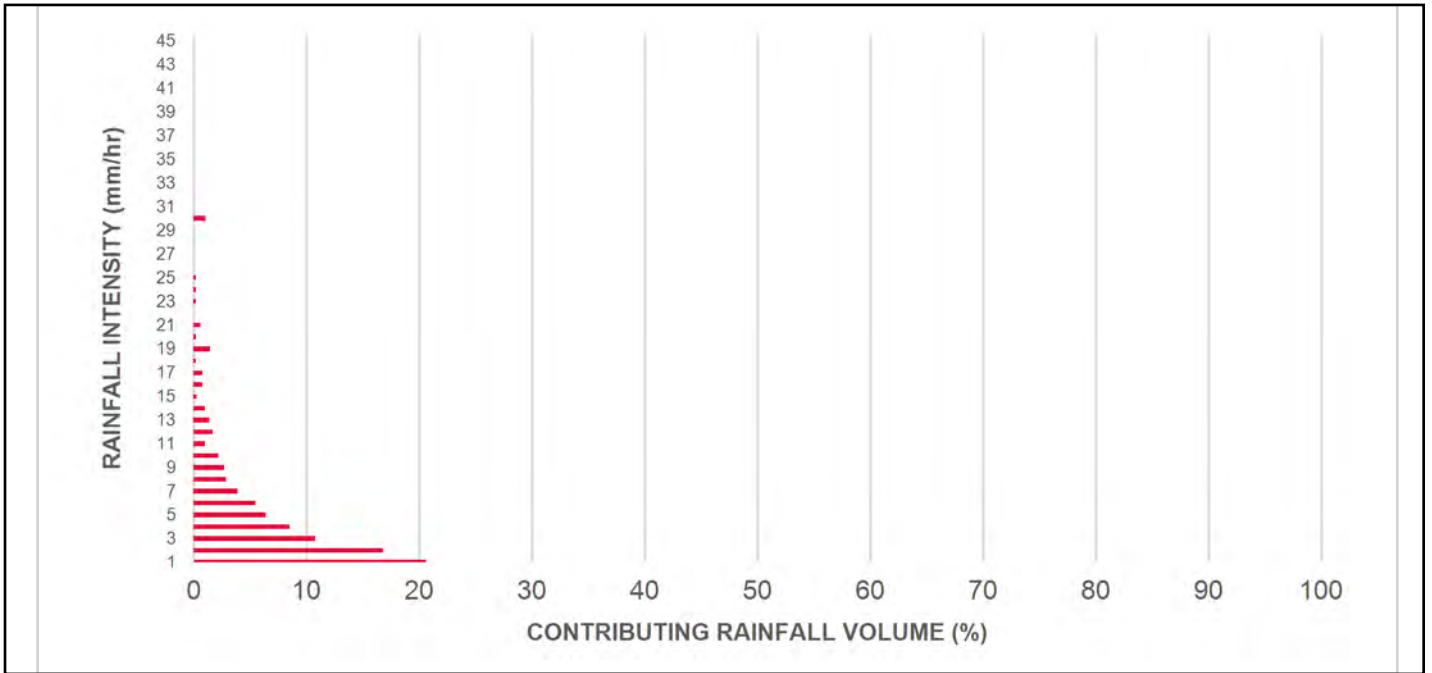
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.5	8.5	0.92	55.0	21.0	70	6.0	6.0
1.00	20.6	29.1	1.85	111.0	42.0	70	14.5	20.5
2.00	16.8	45.9	3.69	222.0	84.0	64	10.8	31.3
3.00	10.8	56.7	5.54	332.0	126.0	61	6.5	37.8
4.00	8.5	65.2	7.38	443.0	168.0	57	4.8	42.6
5.00	6.4	71.6	9.23	554.0	211.0	54	3.4	46.1
6.00	5.5	77.0	11.08	665.0	253.0	53	2.9	48.9
7.00	3.9	81.0	12.92	775.0	295.0	51	2.0	51.0
8.00	2.9	83.9	14.77	886.0	337.0	50	1.5	52.4
9.00	2.7	86.5	16.61	997.0	379.0	49	1.3	53.7
10.00	2.2	88.7	18.46	1108.0	421.0	47	1.0	54.8
11.00	1.0	89.7	20.31	1218.0	463.0	46	0.5	55.2
12.00	1.7	91.3	22.15	1329.0	505.0	45	0.7	56.0
13.00	1.4	92.8	24.00	1440.0	547.0	44	0.6	56.6
14.00	7.2	100.0	25.84	1551.0	590.0	42	3.1	59.6
15.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
16.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
17.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
18.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
19.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
20.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
21.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
22.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
23.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
24.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
25.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
30.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
35.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
40.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
45.00	0.0	100.0	26.00	1560.0	593.0	42	0.0	59.6
Estimated Net Annual Sediment (TSS) Load Reduction =								60 %

Climate Station ID: 6158731 Years of Rainfall Data: 20

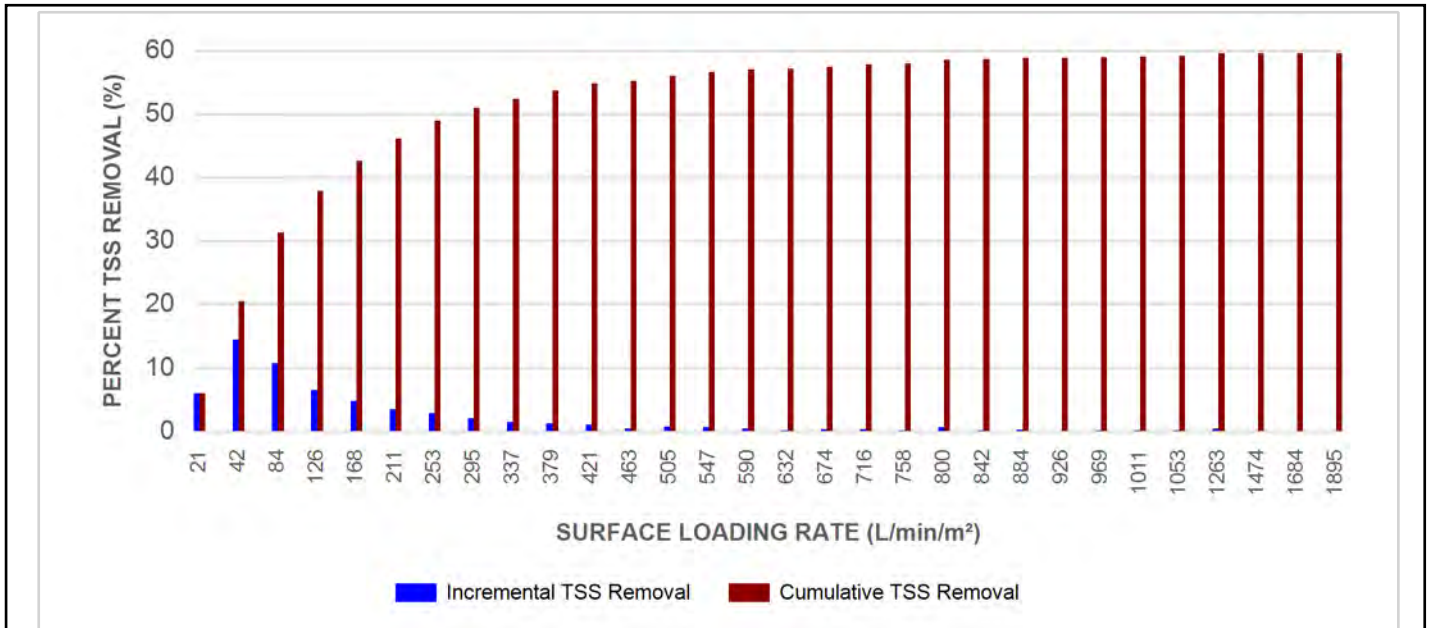


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

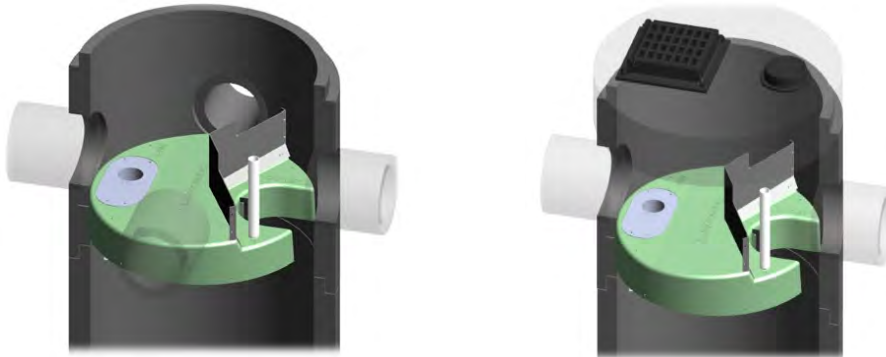
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

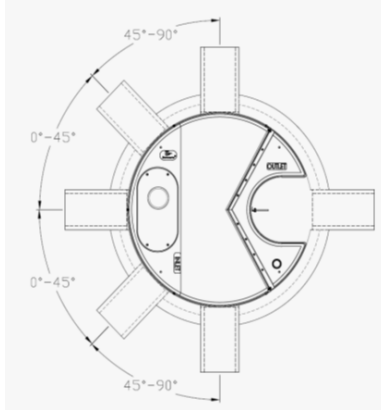
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1. For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>



Stormceptor® EF Sizing Report

Table of TSS Removal vs Surface Loading Rate Based on Third-Party Test Results
Stormceptor® EFO

SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL
1	70	660	42	1320	35	1980	24
30	70	690	42	1350	35	2010	24
60	67	720	41	1380	34	2040	23
90	63	750	41	1410	34	2070	23
120	61	780	41	1440	33	2100	23
150	58	810	41	1470	32	2130	22
180	56	840	41	1500	32	2160	22
210	54	870	41	1530	31	2190	22
240	53	900	41	1560	31	2220	21
270	52	930	40	1590	30	2250	21
300	51	960	40	1620	29	2280	21
330	50	990	40	1650	29	2310	21
360	49	1020	40	1680	28	2340	20
390	48	1050	39	1710	28	2370	20
420	47	1080	39	1740	27	2400	20
450	47	1110	38	1770	27	2430	20
480	46	1140	38	1800	26	2460	19
510	45	1170	37	1830	26	2490	19
540	44	1200	37	1860	26	2520	19
570	43	1230	37	1890	25	2550	19
600	42	1260	36	1920	25	2580	18
630	42	1290	36	1950	24	2600	26



**STANDARD PERFORMANCE SPECIFICATION FOR
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall



Stormceptor® EF Sizing Report

remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

Stormceptor® EF Sizing Report

assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.

Stormceptor® EF Sizing Report

Imbrium® Systems

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

10/17/2024

Province:	Ontario
City:	Halton Hills
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	130 Mountainview Rd N
Project Number:	65993
Designer Name:	Shania Chhom
Designer Company:	Urbantech
Designer Email:	schhom@urbantech.com
Designer Phone:	905-829-6911
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	Tank 4
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Drainage Area (ha):	3.19
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Runoff Coefficient 'c':	0.82
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Particle Size Distribution:	CA ETV
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Target TSS Removal (%):	60.0
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Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	81.34
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	Yes
Upstream Orifice Control Flow Rate to Stormceptor (L/s):	144.00
Peak Conveyance (maximum) Flow Rate (L/s):	
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	2070
Estimated Average Annual Sediment Volume (L/yr):	1683

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EFO4	35
EFO6	45
EFO8	52
EFO10	56
EFO12	61

Recommended Stormceptor EFO Model: **EFO12**

Estimated Net Annual Sediment (TSS) Load Reduction (%): **61**

Water Quality Runoff Volume Capture (%): **> 90**



Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► Stormceptor® EF and Stormceptor® EFO are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► Stormceptor® EF and EFO remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The Canadian ETV PSD shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5



Stormceptor® EF Sizing Report

Upstream Flow Controlled Results

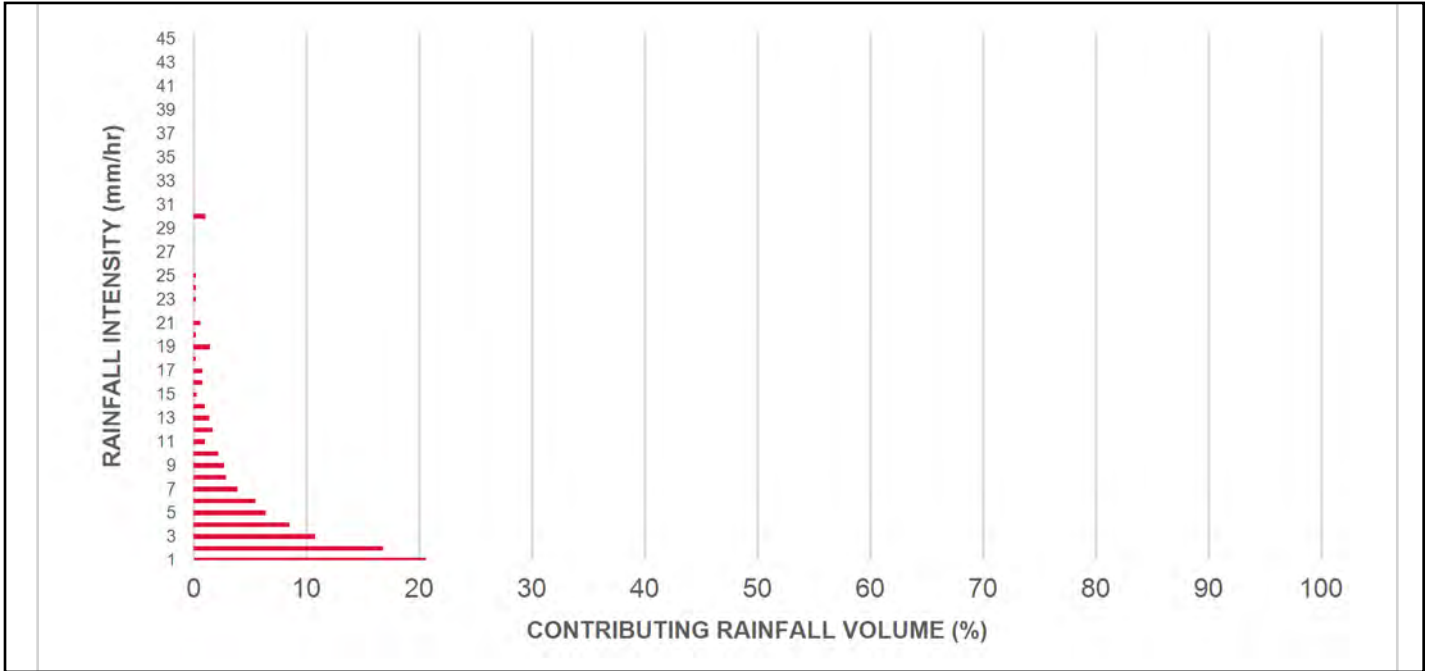
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.5	8.5	3.64	218.0	17.0	70	6.0	6.0
1.00	20.6	29.1	7.27	436.0	35.0	70	14.5	20.5
2.00	16.8	45.9	14.54	873.0	70.0	66	11.0	31.5
3.00	10.8	56.7	21.82	1309.0	105.0	62	6.7	38.2
4.00	8.5	65.2	29.09	1745.0	140.0	59	5.0	43.2
5.00	6.4	71.6	36.36	2182.0	175.0	57	3.6	46.9
6.00	5.5	77.0	43.63	2618.0	209.0	54	2.9	49.8
7.00	3.9	81.0	50.90	3054.0	244.0	53	2.1	51.9
8.00	2.9	83.9	58.18	3491.0	279.0	52	1.5	53.4
9.00	2.7	86.5	65.45	3927.0	314.0	51	1.4	54.7
10.00	2.2	88.7	72.72	4363.0	349.0	50	1.1	55.8
11.00	1.0	89.7	79.99	4799.0	384.0	49	0.5	56.3
12.00	1.7	91.3	87.26	5236.0	419.0	48	0.8	57.1
13.00	1.4	92.8	94.54	5672.0	454.0	47	0.7	57.7
14.00	1.0	93.7	101.81	6108.0	489.0	46	0.4	58.2
15.00	0.3	94.0	109.08	6545.0	524.0	44	0.1	58.3
16.00	0.8	94.8	116.35	6981.0	558.0	44	0.3	58.6
17.00	0.8	95.7	123.62	7417.0	593.0	42	0.4	59.0
18.00	0.2	95.8	130.89	7854.0	628.0	42	0.1	59.1
19.00	4.2	100.0	138.17	8290.0	663.0	42	1.7	60.8
20.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
21.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
22.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
23.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
24.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
25.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
30.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
35.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
40.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
45.00	0.0	100.0	144.00	8640.0	691.0	42	0.0	60.8
Estimated Net Annual Sediment (TSS) Load Reduction =								61 %

Climate Station ID: 6158731 Years of Rainfall Data: 20

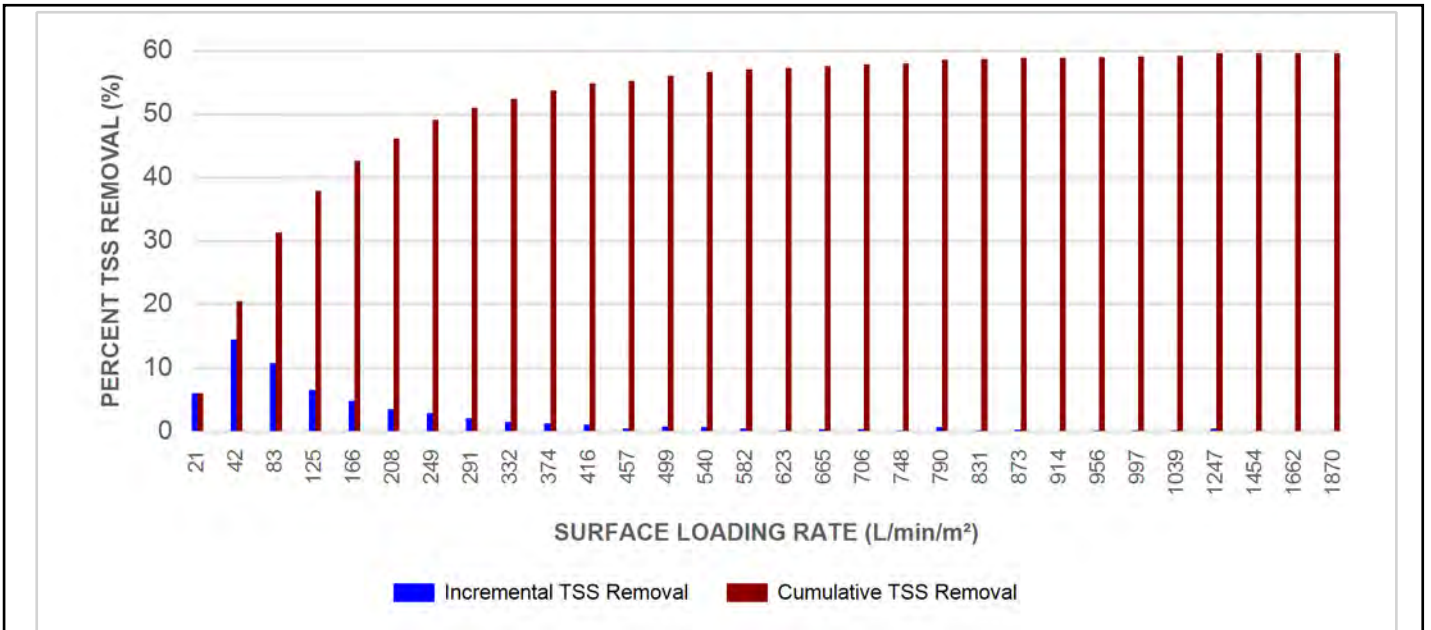


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

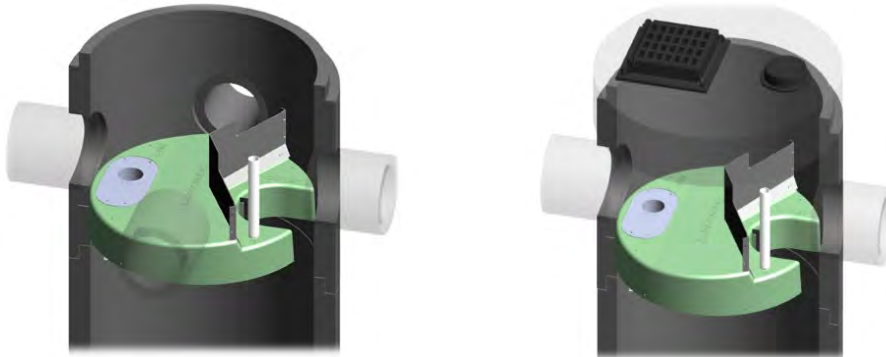
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

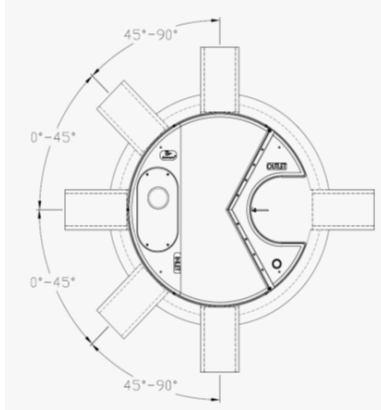
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1. For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>



Stormceptor® EF Sizing Report

Table of TSS Removal vs Surface Loading Rate Based on Third-Party Test Results
Stormceptor® EFO

SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL	SLR (L/min/m ²)	TSS % REMOVAL
1	70	660	42	1320	35	1980	24
30	70	690	42	1350	35	2010	24
60	67	720	41	1380	34	2040	23
90	63	750	41	1410	34	2070	23
120	61	780	41	1440	33	2100	23
150	58	810	41	1470	32	2130	22
180	56	840	41	1500	32	2160	22
210	54	870	41	1530	31	2190	22
240	53	900	41	1560	31	2220	21
270	52	930	40	1590	30	2250	21
300	51	960	40	1620	29	2280	21
330	50	990	40	1650	29	2310	21
360	49	1020	40	1680	28	2340	20
390	48	1050	39	1710	28	2370	20
420	47	1080	39	1740	27	2400	20
450	47	1110	38	1770	27	2430	20
480	46	1140	38	1800	26	2460	19
510	45	1170	37	1830	26	2490	19
540	44	1200	37	1860	26	2520	19
570	43	1230	37	1890	25	2550	19
600	42	1260	36	1920	25	2580	18
630	42	1290	36	1950	24	2600	26



**STANDARD PERFORMANCE SPECIFICATION FOR
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall



Stormceptor® EF Sizing Report

remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

Stormceptor® **EF** Sizing Report

assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.

Proposed Scenario

```

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*****
V V I SSSSS U U A L (v 6.2.2016)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL
V W I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

```

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***** D E T A I L E D O U T P U T *****

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 ca99-c591-486d-bbf8-d7e084a76a2f\scena

DATE: 10-15-2024

TIME: 11:03:48

USER:

COMMENTS: _____

```

*****
** SIMULATION : 100yr_24hr Chicago **
*****

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-----
| CHICAGO STORM | IDF curve parameters: A=1777.200
| Ptotal=130.89 mm | B= 9.000
| | C= 0.795
-----

```

used in: INTENSITY = A / (t + B)^C

Duration of storm = 24.00 hrs
 Storm time step = 10.00 min
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	1.17	6.00	3.96	12.00	3.49	18.00	1.66
0.17	1.19	6.17	4.31	12.17	3.37	18.17	1.63
0.33	1.21	6.33	4.74	12.33	3.27	18.33	1.61
0.50	1.23	6.50	5.27	12.50	3.17	18.50	1.59
0.67	1.26	6.67	5.95	12.67	3.07	18.67	1.57
0.83	1.28	6.83	6.87	12.83	2.99	18.83	1.55
1.00	1.31	7.00	8.17	13.00	2.91	19.00	1.53
1.17	1.33	7.17	10.16	13.17	2.83	19.17	1.52
1.33	1.36	7.33	13.62	13.33	2.76	19.33	1.50
1.50	1.39	7.50	21.15	13.50	2.69	19.50	1.48
1.67	1.42	7.67	50.35	13.67	2.62	19.67	1.46
1.83	1.45	7.83	171.05	13.83	2.56	19.83	1.45
2.00	1.49	8.00	65.47	14.00	2.51	20.00	1.43
2.17	1.52	8.17	35.30	14.17	2.45	20.17	1.41
2.33	1.56	8.33	24.06	14.33	2.40	20.33	1.40
2.50	1.60	8.50	18.29	14.50	2.35	20.50	1.38
2.67	1.64	8.67	14.80	14.67	2.30	20.67	1.37
2.83	1.69	8.83	12.47	14.83	2.25	20.83	1.35
3.00	1.74	9.00	10.80	15.00	2.21	21.00	1.34
3.17	1.79	9.17	9.55	15.17	2.17	21.17	1.33
3.33	1.84	9.33	8.57	15.33	2.13	21.33	1.31
3.50	1.90	9.50	7.79	15.50	2.09	21.50	1.30
3.67	1.96	9.67	7.15	15.67	2.05	21.67	1.29
3.83	2.03	9.83	6.61	15.83	2.02	21.83	1.27
4.00	2.11	10.00	6.16	16.00	1.98	22.00	1.26
4.17	2.19	10.17	5.77	16.17	1.95	22.17	1.25
4.33	2.27	10.33	5.42	16.33	1.92	22.33	1.24
4.50	2.37	10.50	5.13	16.50	1.89	22.50	1.23
4.67	2.47	10.67	4.86	16.67	1.86	22.67	1.22
4.83	2.59	10.83	4.63	16.83	1.83	22.83	1.21
5.00	2.72	11.00	4.41	17.00	1.80	23.00	1.19
5.17	2.86	11.17	4.22	17.17	1.78	23.17	1.18
5.33	3.03	11.33	4.05	17.33	1.75	23.33	1.17
5.50	3.21	11.50	3.89	17.50	1.73	23.50	1.16
5.67	3.43	11.67	3.74	17.67	1.70	23.67	1.15
5.83	3.67	11.83	3.61	17.83	1.68	23.83	1.14

```

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| CALIB |
| NASHYD ( 0026) | Area (ha)= 0.28 Curve Number (CN)= 84.0
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```

|ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
|-----| U.H. Tp(hrs)= 0.22

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

----- TRANSFORMED HYETOGRAPH -----

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.35	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.05	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31

3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21
5.000	2.59	11.000	4.63	17.000	1.83	23.00	1.21
5.083	2.72	11.083	4.41	17.083	1.80	23.08	1.19
5.167	2.72	11.167	4.41	17.167	1.80	23.17	1.19
5.250	2.86	11.250	4.22	17.250	1.78	23.25	1.18
5.333	2.86	11.333	4.22	17.333	1.78	23.33	1.18
5.417	3.03	11.417	4.05	17.417	1.75	23.42	1.17
5.500	3.03	11.500	4.05	17.500	1.75	23.50	1.17
5.583	3.21	11.583	3.89	17.583	1.73	23.58	1.16
5.667	3.21	11.667	3.89	17.667	1.73	23.67	1.16
5.750	3.43	11.750	3.74	17.750	1.70	23.75	1.15
5.833	3.43	11.833	3.74	17.833	1.70	23.83	1.15
5.917	3.67	11.917	3.61	17.917	1.68	23.92	1.14
6.000	3.67	12.000	3.61	18.000	1.68	24.00	1.14

Unit Hyd Qpeak (cms) = 0.049

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PEAK FLOW (cms) = 0.053 (i)
TIME TO PEAK (hrs) = 8.167
RUNOFF VOLUME (mm) = 90.816
TOTAL RAINFALL (mm) = 130.887
RUNOFF COEFFICIENT = 0.694

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(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| CALIB |
| NASHYD ( 0029) | Area (ha)= 0.18 Curve Number (CN)= 84.0
|ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
|-----| U.H. Tp(hrs)= 0.17

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NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.36	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.04	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29

3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21
5.000	2.59	11.000	4.63	17.000	1.83	23.00	1.21
5.083	2.72	11.083	4.41	17.083	1.80	23.08	1.19
5.167	2.72	11.167	4.41	17.167	1.80	23.17	1.19
5.250	2.86	11.250	4.22	17.250	1.78	23.25	1.18
5.333	2.86	11.333	4.22	17.333	1.78	23.33	1.18
5.417	3.03	11.417	4.05	17.417	1.75	23.42	1.17
5.500	3.03	11.500	4.05	17.500	1.75	23.50	1.17
5.583	3.21	11.583	3.89	17.583	1.73	23.58	1.16
5.667	3.21	11.667	3.89	17.667	1.73	23.67	1.16
5.750	3.43	11.750	3.74	17.750	1.70	23.75	1.15
5.833	3.43	11.833	3.74	17.833	1.70	23.83	1.15
5.917	3.67	11.917	3.61	17.917	1.68	23.92	1.14
6.000	3.67	12.000	3.61	18.000	1.68	24.00	1.14

Unit Hyd Qpeak (cms)= 0.040

PEAK FLOW (cms)= 0.038 (i)

TIME TO PEAK (hrs)= 8.083

RUNOFF VOLUME (mm)= 90.611

TOTAL RAINFALL (mm)= 130.887

RUNOFF COEFFICIENT = 0.692

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Dir. Conn.(%)
STANDHYD (0028)	0.08	99.00
ID= 1 DT= 5.0 min		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.08	0.00
Dep. Storage (mm)	1.00	5.00
Average Slope (%)	1.00	2.00
Length (m)	23.09	40.00
Mannings n	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.36	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.04	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30

3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21
5.000	2.59	11.000	4.63	17.000	1.83	23.00	1.21
5.083	2.72	11.083	4.41	17.083	1.80	23.08	1.19
5.167	2.72	11.167	4.41	17.167	1.80	23.17	1.19
5.250	2.86	11.250	4				

CALIB
STANDHYD (0037)
ID= 1 DT= 5.0 min

Area (ha)= 0.04
Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.04	0.00
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	16.33	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.35	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.05	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38

2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21
5.000	2.59	11.000	4.63	17.000	1.83	23.00	1.21
5.083	2.72	11.083	4.41	17.083	1.80	23.08	1.19
5.167	2.72	11.167	4.41	17.167	1.80	23.17	1.19
5.250	2.86	11.250	4.22	17.250	1.78	23.25	1.18
5.333	2.86	11.333	4.22	17.333	1.78	23.33	1.18
5.417	3.03	11.417	4.05	17.417	1.75	23.42	1.17
5.500	3.03	11.500	4.05	17.500	1.75	23.50	1.17
5.583	3.21	11.583	3.89	17.583	1.73	23.58	1.16
5.667	3.21	11.667	3.89	17.667	1.73	23.67	1.16
5.750	3.43	11.750	3.74	17.750	1.70	23.75	1.15
5.833	3.43	11.833	3.74	17.833	1.70	23.83	1.15
5.917	3.67	11.917	3.61	17.917	1.68	23.92	1.14
6.000	3.67	12.000	3.61	18.000	1.68	24.00	1.14

Max. Eff. Inten. (mm/hr)= 171.05
over (min)= 5.00
Storage Coeff. (min)= 0.69 (ii) 1.60 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.34

PEAK FLOW (cms)= 0.02
TIME TO PEAK (hrs)= 8.00

TOTALS
0.019 (iii)
8.00

RUNOFF VOLUME (mm)= 129.89 96.10 129.55
TOTAL RAINFALL (mm)= 130.89 130.89 130.89
RUNOFF COEFFICIENT = 0.99 0.73 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0038)
ID= 1 DT= 5.0 min

Area (ha)= 0.01
Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.01	0.00
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	8.16	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48

1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.35	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.05	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22	

5.833	3.43	11.833	3.74	17.833	1.70	23.83	1.15
5.917	3.67	11.917	3.61	17.917	1.68	23.92	1.14
6.000	3.67	12.000	3.61	18.000	1.68	24.00	1.14

Max.Eff.Inten.(mm/hr)= 171.05 129.03
over (min) = 5.00 5.00
Storage Coeff. (min)= 0.46 (ii) 1.36 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.33

TOTALS
PEAK FLOW (cms)= 0.00 0.00 0.005 (iii)
TIME TO PEAK (hrs)= 7.92 8.00 8.00
RUNOFF VOLUME (mm)= 129.89 96.10 107.29
TOTAL RAINFALL (mm)= 130.89 130.89 130.89
RUNOFF COEFFICIENT = 0.99 0.73 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0030)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0026):	0.28	0.053	8.17	90.82
+ ID2= 2 (0028):	0.08	0.038	8.00	129.50
=====				
ID = 3 (0030):	0.36	0.076	8.00	99.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0030):	0.36	0.076	8.00	99.41
+ ID2= 2 (0029):	0.18	0.038	8.00	90.61
=====				
ID = 1 (0030):	0.54	0.108	8.00	96.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0030):	0.54	0.108	8.00	96.48
+ ID2= 2 (0037):	0.04	0.019	8.00	129.55
=====				
ID = 3 (0030):	0.58	0.127	8.00	98.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0030):	0.58	0.127	8.00	98.76
+ ID2= 2 (0038):	0.01	0.005	8.00	107.29
=====				
ID = 1 (0030):	0.59	0.132	8.00	98.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (0031)				
ID= 1 DT= 5.0 min	Area (ha)=	0.09	Curve Number (CN)=	84.0
	Ia (mm)=	5.00	# of Linear Res.(N)=	3.00
	U.H. Tp(hrs)=	0.17		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66	0.167	1.17	6.167	3.96
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63	0.333	1.19	6.333	4.31
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61	0.500	1.21	6.500	4.74
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59	0.667	1.23	6.667	5.27
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57	0.833	1.26	6.833	5.95
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55	1.000	1.28	7.000	6.87
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53	1.167	1.31	7.167	8.17
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52				

1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.36	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.04	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21
5.000	2.59	11.000	4.63	17.000	1.83	23.00	1.21
5.083	2.72	11.083	4.41	17.083	1.80	23.08	1.19
5.167	2.72	11.167	4.41	17.167	1.80	23.17	1.19
5.250	2.86	11.250	4.22	17.250	1.78	23.25	1.18
5.333	2.86	11.333	4.22	17.333	1.78	23.33	1.18
5.417	3.03	11.417	4.05	17.417	1.75	23.42	1.17

5.500	3.03	11.500	4.05	17.500	1.75	23.50	1.17
5.583	3.21	11.583	3.89	17.583	1.73	23.58	1.16
5.667	3.21	11.667	3.89	17.667	1.73	23.67	1.16
5.750	3.43	11.750	3.74	17.750	1.70	23.75	1.15
5.833	3.43	11.833	3.74	17.833	1.70	23.83	1.15
5.917	3.67	11.917	3.61	17.917	1.68	23.92	1.14
6.000	3.67	12.000	3.61	18.000	1.68	24.00	1.14

Unit Hyd Qpeak (cms)= 0.020
PEAK FLOW (cms)= 0.019 (i)
TIME TO PEAK (hrs)= 8.083
RUNOFF VOLUME (mm)= 90.609
TOTAL RAINFALL (mm)= 130.887
RUNOFF COEFFICIENT = 0.692

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANDHYD (0018)				
ID= 1 DT= 5.0 min	Area (ha)=	0.71	Total Imp(%)=	99.00
	Dir. Conn.(%)=	99.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.70	0.01
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	68.80	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66	0.167	1.17	6.167	3.96
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63	0.333	1.19	6.333	4.31
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61	0.500	1.21	6.500	4.74
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59	0.667	1.23	6.667	5.27
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57	0.833	1.26	6.833	5.95
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55	1.000	1.28	7.000	6.87
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53				

1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.36	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.04	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21
5.000	2.59	11.000	4.63	17.000	1.83	23.00	1.21
5.083	2.72	11.083	4.41	17.083	1.80	23.08	1.19
5.167	2.72	11.167	4.41	17.167	1.80	23.17	1.19
5.250	2.86	11.250	4.22	17.250	1.78	23.25	1.18

5.333	2.86	11.333	4.22	17.333	1.78	23.33	1.18
5.417	3.03	11.417	4.05	17.417	1.75	23.42	1.17
5.500	3.03	11.500	4.05	17.500	1.75	23.50	1.17
5.583	3.21	11.583	3.89	17.583	1.73	23.58	1.16
5.667	3.21	11.667	3.89	17.667	1.73	23.67	1.16
5.750	3.43	11.750	3.74	17.750	1.70	23.75	1.15
5.833	3.43	11.833	3.74	17.833	1.70	23.83	1.15
5.917	3.67	11.917	3.61	17.917	1.68	23.92	1.14
6.000	3.67	12.000	3.61	18.000	1.68	24.00	1.14

Max.Eff.Inten.(mm/hr)= 171.05
over (min)= 5.00
Storage Coeff. (min)= 1.65 (ii) 2.55 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.32 0.29

TOTALS
PEAK FLOW (cms)= 0.33 0.00 0.336 (iii)
TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 129.89 90.94 129.50
TOTAL RAINFALL (mm)= 130.89 130.89 130.89
RUNOFF COEFFICIENT = 0.99 0.69 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0032)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0018):	0.71	0.336	8.00	129.50
+ ID2= 2 (0031):	0.09	0.019	8.00	90.61

ID = 3 (0032):	0.80	0.352	8.00	125.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR(0019)				
OVERFLOW IS OFF				
IN= 2----> OUT= 1	OUTFLOW	STORAGE	OUTFLOW	STORAGE
DT= 5.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.0190	0.0279
	0.0000	0.0028	0.0200	0.0307

0.0080	0.0056	0.0210	0.0335
0.0100	0.0084	0.0220	0.0363
0.0120	0.0112	0.0230	0.0391
0.0140	0.0140	0.0240	0.0419
0.0150	0.0168	0.0250	0.0447
0.0160	0.0196	0.0250	0.0475
0.0170	0.0223	0.0260	0.0499
0.0180	0.0251	0.0000	0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (0032) 0.800 0.352 8.00 125.12
OUTFLOW: ID= 1 (0019) 0.800 0.026 9.08 121.53

PEAK FLOW REDUCTION [Qout/Qin](%)= 7.38
TIME SHIFT OF PEAK FLOW (min)= 65.00
MAXIMUM STORAGE USED (ha.m.)= 0.0499

CALIB				
NASHYD (0036)	Area (ha)=	0.14	Curve Number (CN)=	84.0
ID= 1 DT= 5.0 min	Ia (mm)=	5.00	# of Linear Res. (N)=	3.00
	U.H. Tp(hrs)=	0.16		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48

1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.36	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.04	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21
5.000	2.59	11.000	4.63	17.000	1.83	23.00	1.21
5.083	2.72	11.083	4.41	17.083	1.80	23.08	1.19
5.167	2.72	11.167	4.41	17.167	1.80	23.17	1.19

5.833 3.43 | 11.833 3.74 | 17.833 1.70 | 23.83 1.15
 5.917 3.67 | 11.917 3.61 | 17.917 1.68 | 23.92 1.14
 6.000 3.67 | 12.000 3.61 | 18.000 1.68 | 24.00 1.14

Unit Hyd Qpeak (cms)= 0.033

PEAK FLOW (cms)= 0.031 (i)
 TIME TO PEAK (hrs)= 8.083
 RUNOFF VOLUME (mm)= 90.525
 TOTAL RAINFALL (mm)= 130.887
 RUNOFF COEFFICIENT = 0.692

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0039) Area (ha)= 0.05 Curve Number (CN)= 84.0
 ID= 1 DT= 5.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
 U.H. Tp(hrs)= 0.16

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.35	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45

2.000 1.45 | 8.000 171.04 | 14.000 2.56 | 20.00 1.45
 2.083 1.49 | 8.083 65.47 | 14.083 2.51 | 20.08 1.43
 2.167 1.49 | 8.167 65.47 | 14.167 2.51 | 20.17 1.43
 2.250 1.52 | 8.250 35.30 | 14.250 2.45 | 20.25 1.41
 2.333 1.52 | 8.333 35.30 | 14.333 2.45 | 20.33 1.41
 2.417 1.56 | 8.417 24.06 | 14.417 2.40 | 20.42 1.40
 2.500 1.56 | 8.500 24.06 | 14.500 2.40 | 20.50 1.40
 2.583 1.60 | 8.583 18.29 | 14.583 2.35 | 20.58 1.38
 2.667 1.60 | 8.667 18.29 | 14.667 2.35 | 20.67 1.38
 2.750 1.64 | 8.750 14.80 | 14.750 2.30 | 20.75 1.37
 2.833 1.64 | 8.833 14.80 | 14.833 2.30 | 20.83 1.37
 2.917 1.69 | 8.917 12.47 | 14.917 2.25 | 20.92 1.35
 3.000 1.69 | 9.000 12.47 | 15.000 2.25 | 21.00 1.35
 3.083 1.74 | 9.083 10.80 | 15.083 2.21 | 21.08 1.34
 3.167 1.74 | 9.167 10.80 | 15.167 2.21 | 21.17 1.34
 3.250 1.79 | 9.250 9.55 | 15.250 2.17 | 21.25 1.33
 3.333 1.79 | 9.333 9.55 | 15.333 2.17 | 21.33 1.33
 3.417 1.84 | 9.417 8.57 | 15.417 2.13 | 21.42 1.31
 3.500 1.84 | 9.500 8.57 | 15.500 2.13 | 21.50 1.31
 3.583 1.90 | 9.583 7.79 | 15.583 2.09 | 21.58 1.30
 3.667 1.90 | 9.667 7.79 | 15.667 2.09 | 21.67 1.30
 3.750 1.96 | 9.750 7.15 | 15.750 2.05 | 21.75 1.29
 3.833 1.96 | 9.833 7.15 | 15.833 2.05 | 21.83 1.29
 3.917 2.03 | 9.917 6.61 | 15.917 2.02 | 21.92 1.27
 4.000 2.03 | 10.000 6.61 | 16.000 2.02 | 22.00 1.27
 4.083 2.11 | 10.083 6.16 | 16.083 1.98 | 22.08 1.26
 4.167 2.11 | 10.167 6.16 | 16.167 1.98 | 22.17 1.26
 4.250 2.19 | 10.250 5.77 | 16.250 1.95 | 22.25 1.25
 4.333 2.19 | 10.333 5.77 | 16.333 1.95 | 22.33 1.25
 4.417 2.27 | 10.417 5.42 | 16.417 1.92 | 22.42 1.24
 4.500 2.27 | 10.500 5.42 | 16.500 1.92 | 22.50 1.24
 4.583 2.37 | 10.583 5.13 | 16.583 1.89 | 22.58 1.23
 4.667 2.37 | 10.667 5.13 | 16.667 1.89 | 22.67 1.23
 4.750 2.47 | 10.750 4.86 | 16.750 1.86 | 22.75 1.22
 4.833 2.47 | 10.833 4.86 | 16.833 1.86 | 22.83 1.22
 4.917 2.59 | 10.917 4.63 | 16.917 1.83 | 22.92 1.21
 5.000 2.59 | 11.000 4.63 | 17.000 1.83 | 23.00 1.21
 5.083 2.72 | 11.083 4.41 | 17.083 1.80 | 23.08 1.19
 5.167 2.72 | 11.167 4.41 | 17.167 1.80 | 23.17 1.19
 5.250 2.86 | 11.250 4.22 | 17.250 1.78 | 23.25 1.18
 5.333 2.86 | 11.333 4.22 | 17.333 1.78 | 23.33 1.18
 5.417 3.03 | 11.417 4.05 | 17.417 1.75 | 23.42 1.17
 5.500 3.03 | 11.500 4.05 | 17.500 1.75 | 23.50 1.17
 5.583 3.21 | 11.583 3.89 | 17.583 1.73 | 23.58 1.16
 5.667 3.21 | 11.667 3.89 | 17.667 1.73 | 23.67 1.16
 5.750 3.43 | 11.750 3.74 | 17.750 1.70 | 23.75 1.15
 5.833 3.43 | 11.833 3.74 | 17.833 1.70 | 23.83 1.15
 5.917 3.67 | 11.917 3.61 | 17.917 1.68 | 23.92 1.14

Unit Hyd Qpeak (cms)= 0.012

PEAK FLOW (cms)= 0.011 (i)
 TIME TO PEAK (hrs)= 8.083
 RUNOFF VOLUME (mm)= 90.522
 TOTAL RAINFALL (mm)= 130.887
 RUNOFF COEFFICIENT = 0.692

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0001) Area (ha)= 0.27
 ID= 1 DT= 5.0 min Total Imp(%)= 64.00 Dir. Conn.(%)= 64.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.17 0.10
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 42.43 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46

1.833 1.42 | 7.833 50.36 | 13.833 2.62 | 19.83 1.46
 1.917 1.45 | 7.917 171.05 | 13.917 2.56 | 19.92 1.45
 2.000 1.45 | 8.000 171.04 | 14.000 2.56 | 20.00 1.45
 2.083 1.49 | 8.083 65.47 | 14.083 2.51 | 20.08 1.43
 2.167 1.49 | 8.167 65.47 | 14.167 2.51 | 20.17 1.43
 2.250 1.52 | 8.250 35.30 | 14.250 2.45 | 20.25 1.41
 2.333 1.52 | 8.333 35.30 | 14.333 2.45 | 20.33 1.41
 2.417 1.56 | 8.417 24.06 | 14.417 2.40 | 20.42 1.40
 2.500 1.56 | 8.500 24.06 | 14.500 2.40 | 20.50 1.40
 2.583 1.60 | 8.583 18.29 | 14.583 2.35 | 20.58 1.38
 2.667 1.60 | 8.667 18.29 | 14.667 2.35 | 20.67 1.38
 2.750 1.64 | 8.750 14.80 | 14.750 2.30 | 20.75 1.37
 2.833 1.64 | 8.833 14.80 | 14.833 2.30 | 20.83 1.37
 2.917 1.69 | 8.917 12.47 | 14.917 2.25 | 20.92 1.35
 3.000 1.69 | 9.000 12.47 | 15.000 2.25 | 21.00 1.35
 3.083 1.74 | 9.083 10.80 | 15.083 2.21 | 21.08 1.34
 3.167 1.74 | 9.167 10.80 | 15.167 2.21 | 21.17 1.34
 3.250 1.79 | 9.250 9.55 | 15.250 2.17 | 21.25 1.33
 3.333 1.79 | 9.333 9.55 | 15.333 2.17 | 21.33 1.33
 3.417 1.84 | 9.417 8.57 | 15.417 2.13 | 21.42 1.31
 3.500 1.84 | 9.500 8.57 | 15.500 2.13 | 21.50 1.31
 3.583 1.90 | 9.583 7.79 | 15.583 2.09 | 21.58 1.30
 3.667 1.90 | 9.667 7.79 | 15.667 2.09 | 21.67 1.30
 3.750 1.96 | 9.750 7.15 | 15.750 2.05 | 21.75 1.29
 3.833 1.96 | 9.833 7.15 | 15.833 2.05 | 21.83 1.29
 3.917 2.03 | 9.917 6.61 | 15.917 2.02 | 21.92 1.27
 4.000 2.03 | 10.000 6.61 | 16.000 2.02 | 22.00 1.27
 4.083 2.11 | 10.083 6.16 | 16.083 1.98 | 22.08 1.26
 4.167 2.11 | 10.167 6.16 | 16.167 1.98 | 22.17 1.26
 4.250 2.19 | 10.250 5.77 | 16.250 1.95 | 22.25 1.25
 4.333 2.19 | 10.333 5.77 | 16.333 1.95 | 22.33 1.25
 4.417 2.27 | 10.417 5.42 | 16.417 1.92 | 22.42 1.24
 4.500 2.27 | 10.500 5.42 | 16.500 1.92 | 22.50 1.24
 4.583 2.37 | 10.583 5.13 | 16.583 1.89 | 22.58 1.23
 4.667 2.37 | 10.667 5.13 | 16.667 1.89 | 22.67 1.23
 4.750 2.47 | 10.750 4.86 | 16.750 1.86 | 22.75 1.22
 4.833 2.47 | 10.833 4.86 | 16.833 1.86 | 22.83 1.22
 4.917 2.59 | 10.917 4.63 | 16.917 1.83 | 22.92 1.21
 5.000 2.59 | 11.000 4.63 | 17.000 1.83 | 23.00 1.21
 5.083 2.72 | 11.083 4.41 | 17.083 1.80 | 23.08 1.19
 5.167 2.72 | 11.167 4.41 | 17.167 1.80 | 23.17 1.19
 5.250 2.86 | 11.250 4.22 | 17.250 1.78 | 23.25 1.18
 5.333 2.86 | 11.333 4.22 | 17.333 1.78 | 23.33 1.18
 5.417 3.03 | 11.417 4.05 | 17.417 1.75 | 23.42 1.17
 5.500 3.03 | 11.500 4.05 | 17.500 1.75 | 23.50 1.17
 5.583 3.21 | 11.583 3.89 | 17.583 1.73 | 23.58 1.16
 5.667 3.21 | 11.667 3.89 | 17.667 1.73 | 23.67 1.16
 5.750 3.43 | 11.750 3.74 | 17.750 1.70 | 23.75 1.15
 5.833 3.43 | 11.833 3.74 | 17.833 1.70 | 23.83 1.15
 5.917 3.67 | 11.917 3.61 | 17.917 1.68 | 23.92 1.14

6.000 3.67 |12.000 3.61 |18.000 1.68 | 24.00 1.14

Max.Eff.Inten.(mm/hr)= 171.05 122.16
 over (min) 5.00 10.00
 Storage Coeff. (min)= 1.23 (ii) 5.76 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.15

TOTALS

PEAK FLOW (cms)= 0.08 0.03 0.106 (iii)
 TIME TO PEAK (hrs)= 8.00 8.00 8.00
 RUNOFF VOLUME (mm)= 129.89 90.94 115.85
 TOTAL RAINFALL (mm)= 130.89 130.89 130.89
 RUNOFF COEFFICIENT = 0.99 0.69 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | STANDHYD (0014) | Area (ha)= 0.72
 | ID= 1 DT= 5.0 min | Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.71 0.01
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 69.28 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57

0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.36	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.04	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0014) 0.720 0.341 8.00 129.50
 OUTFLOW : ID= 1 (0020) 0.720 0.026 8.92 125.86

PEAK FLOW REDUCTION [Qout/Qin](%)= 7.63
 TIME SHIFT OF PEAK FLOW (min)= 55.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0455

 | CALIB |
 | STANDHYD (0016) | Area (ha)= 0.77
 | ID= 1 DT= 5.0 min | Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.76 0.01
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 71.65 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46

5.000 2.59 |11.000 4.63 |17.000 1.83 | 23.00 1.21
 5.083 2.72 |11.083 4.41 |17.083 1.80 | 23.08 1.19
 5.167 2.72 |11.167 4.41 |17.167 1.80 | 23.17 1.19
 5.250 2.86 |11.250 4.22 |17.250 1.78 | 23.25 1.18
 5.333 2.86 |11.333 4.22 |17.333 1.78 | 23.33 1.18
 5.417 3.03 |11.417 4.05 |17.417 1.75 | 23.42 1.17
 5.500 3.03 |11.500 4.05 |17.500 1.75 | 23.50 1.17
 5.583 3.21 |11.583 3.89 |17.583 1.73 | 23.58 1.16
 5.667 3.21 |11.667 3.89 |17.667 1.73 | 23.67 1.16
 5.750 3.43 |11.750 3.74 |17.750 1.70 | 23.75 1.15
 5.833 3.43 |11.833 3.74 |17.833 1.70 | 23.83 1.15
 5.917 3.67 |11.917 3.61 |17.917 1.68 | 23.92 1.14
 6.000 3.67 |12.000 3.61 |18.000 1.68 | 24.00 1.14

Max.Eff.Inten.(mm/hr)= 171.05 122.16
 over (min) 5.00 5.00
 Storage Coeff. (min)= 1.65 (ii) 2.56 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.32 0.29

TOTALS

PEAK FLOW (cms)= 0.34 0.00 0.341 (iii)
 TIME TO PEAK (hrs)= 8.00 8.00 8.00
 RUNOFF VOLUME (mm)= 129.89 90.94 129.50
 TOTAL RAINFALL (mm)= 130.89 130.89 130.89
 RUNOFF COEFFICIENT = 0.99 0.69 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | RESERVOIR(0020) | OVERFLOW IS OFF
 | IN= 2---> OUT= 1 |
DT= 5.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	0.0190	0.0255
0.0000	0.0025	0.0200	0.0280
0.0080	0.0051	0.0210	0.0306
0.0100	0.0076	0.0220	0.0331
0.0120	0.0102	0.0230	0.0357
0.0140	0.0127	0.0240	0.0382
0.0150	0.0153	0.0250	0.0407
0.0160	0.0178	0.0250	0.0433
0.0170	0.0204	0.0260	0.0459
0.0180	0.0229	0.0000	0.0000

1.833	1.42	7.833	50.36	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.04	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37
2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21
5.000	2.59	11.000	4.63	17.000	1.83	23.00	1.21
5.083	2.72	11.083	4.41	17.083	1.80	23.08	1.19
5.167	2.72	11.167	4.41	17.167	1.80	23.17	1.19
5.250	2.86	11.250	4.22	17.250	1.78	23.25	1.18
5.333	2.86	11.333	4.22	17.333	1.78	23.33	1.18
5.417	3.03	11.417	4.05	17.417	1.75	23.42	1.17
5.500	3.03	11.500	4.05	17.500	1.75	23.50	1.17
5.583	3.21	11.583	3.89	17.583	1.73	23.58	1.16
5.667	3.21	11.667	3.89	17.667	1.73	23.67	1.16
5.750	3.43	11.750	3.74	17.750	1.70	23.75	1.15
5.833	3.43	11.833	3.74	17.833	1.70	23.83	1.15
5.917	3.67	11.917	3.61	17.917	1.68	23.92	1.14

6.000	3.67	12.000	3.61	18.000	1.68	24.00	1.14
Max.Eff.Inten.(mm/hr)=	171.05	122.16					
over (min)	5.00	5.00					
Storage Coeff. (min)=	1.69 (ii)	2.59 (ii)					
Unit Hyd. Tpeak (min)=	5.00	5.00					
Unit Hyd. peak (cms)=	0.32	0.29					
PEAK FLOW (cms)=	0.36	0.00					*TOTALS*
TIME TO PEAK (hrs)=	8.00	8.00					0.364 (iii)
RUNOFF VOLUME (mm)=	129.89	90.94					8.00
TOTAL RAINFALL (mm)=	130.89	130.89					129.49
RUNOFF COEFFICIENT =	0.99	0.69					130.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0017)				OVERFLOW IS OFF			
IN= 2----> OUT= 1							
DT= 5.0 min							
	OUTFLOW (cms)	STORAGE (ha.m.)		OUTFLOW (cms)	STORAGE (ha.m.)		
	0.0000	0.0000		0.0190	0.0279		
	0.0000	0.0028		0.0200	0.0307		
	0.0080	0.0056		0.0210	0.0334		
	0.0100	0.0084		0.0220	0.0362		
	0.0120	0.0111		0.0230	0.0390		
	0.0140	0.0139		0.0240	0.0418		
	0.0150	0.0167		0.0250	0.0446		
	0.0160	0.0195		0.0250	0.0474		
	0.0170	0.0223		0.0260	0.0498		
	0.0180	0.0251		0.0000	0.0000		
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)			
INFLOW : ID= 2 (0016)	0.770	0.364	8.00	129.50			
OUTFLOW: ID= 1 (0017)	0.770	0.026	9.00	125.77			
PEAK FLOW REDUCTION [Qout/Qin](%)=	7.14						
TIME SHIFT OF PEAK FLOW (min)=	60.00						
MAXIMUM STORAGE USED (ha.m.)=	0.0498						

CALIB STANDHYD (0021)	Area (ha)=	0.44
ID= 1 DT= 5.0 min	Total Imp(%)=	99.00
	Dir. Conn.(%)=	99.00
Surface Area (ha)=	IMPERVIOUS	0.44
Dep. Storage (mm)=	PERVIOUS (i)	0.00
Average Slope (%)=		1.00
Length (m)=		54.16
Mannings n =		0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	6.083	3.96	12.083	3.49	18.08	1.66
0.167	1.17	6.167	3.96	12.167	3.49	18.17	1.66
0.250	1.19	6.250	4.31	12.250	3.37	18.25	1.63
0.333	1.19	6.333	4.31	12.333	3.37	18.33	1.63
0.417	1.21	6.417	4.74	12.417	3.27	18.42	1.61
0.500	1.21	6.500	4.74	12.500	3.27	18.50	1.61
0.583	1.23	6.583	5.27	12.583	3.17	18.58	1.59
0.667	1.23	6.667	5.27	12.667	3.17	18.67	1.59
0.750	1.26	6.750	5.95	12.750	3.07	18.75	1.57
0.833	1.26	6.833	5.95	12.833	3.07	18.83	1.57
0.917	1.28	6.917	6.87	12.917	2.99	18.92	1.55
1.000	1.28	7.000	6.87	13.000	2.99	19.00	1.55
1.083	1.31	7.083	8.17	13.083	2.91	19.08	1.53
1.167	1.31	7.167	8.17	13.167	2.91	19.17	1.53
1.250	1.33	7.250	10.16	13.250	2.83	19.25	1.52
1.333	1.33	7.333	10.16	13.333	2.83	19.33	1.52
1.417	1.36	7.417	13.62	13.417	2.76	19.42	1.50
1.500	1.36	7.500	13.62	13.500	2.76	19.50	1.50
1.583	1.39	7.583	21.15	13.583	2.69	19.58	1.48
1.667	1.39	7.667	21.15	13.667	2.69	19.67	1.48
1.750	1.42	7.750	50.35	13.750	2.62	19.75	1.46
1.833	1.42	7.833	50.36	13.833	2.62	19.83	1.46
1.917	1.45	7.917	171.05	13.917	2.56	19.92	1.45
2.000	1.45	8.000	171.04	14.000	2.56	20.00	1.45
2.083	1.49	8.083	65.47	14.083	2.51	20.08	1.43
2.167	1.49	8.167	65.47	14.167	2.51	20.17	1.43
2.250	1.52	8.250	35.30	14.250	2.45	20.25	1.41
2.333	1.52	8.333	35.30	14.333	2.45	20.33	1.41
2.417	1.56	8.417	24.06	14.417	2.40	20.42	1.40
2.500	1.56	8.500	24.06	14.500	2.40	20.50	1.40
2.583	1.60	8.583	18.29	14.583	2.35	20.58	1.38
2.667	1.60	8.667	18.29	14.667	2.35	20.67	1.38
2.750	1.64	8.750	14.80	14.750	2.30	20.75	1.37

2.833	1.64	8.833	14.80	14.833	2.30	20.83	1.37
2.917	1.69	8.917	12.47	14.917	2.25	20.92	1.35
3.000	1.69	9.000	12.47	15.000	2.25	21.00	1.35
3.083	1.74	9.083	10.80	15.083	2.21	21.08	1.34
3.167	1.74	9.167	10.80	15.167	2.21	21.17	1.34
3.250	1.79	9.250	9.55	15.250	2.17	21.25	1.33
3.333	1.79	9.333	9.55	15.333	2.17	21.33	1.33
3.417	1.84	9.417	8.57	15.417	2.13	21.42	1.31
3.500	1.84	9.500	8.57	15.500	2.13	21.50	1.31
3.583	1.90	9.583	7.79	15.583	2.09	21.58	1.30
3.667	1.90	9.667	7.79	15.667	2.09	21.67	1.30
3.750	1.96	9.750	7.15	15.750	2.05	21.75	1.29
3.833	1.96	9.833	7.15	15.833	2.05	21.83	1.29
3.917	2.03	9.917	6.61	15.917	2.02	21.92	1.27
4.000	2.03	10.000	6.61	16.000	2.02	22.00	1.27
4.083	2.11	10.083	6.16	16.083	1.98	22.08	1.26
4.167	2.11	10.167	6.16	16.167	1.98	22.17	1.26
4.250	2.19	10.250	5.77	16.250	1.95	22.25	1.25
4.333	2.19	10.333	5.77	16.333	1.95	22.33	1.25
4.417	2.27	10.417	5.42	16.417	1.92	22.42	1.24
4.500	2.27	10.500	5.42	16.500	1.92	22.50	1.24
4.583	2.37	10.583	5.13	16.583	1.89	22.58	1.23
4.667	2.37	10.667	5.13	16.667	1.89	22.67	1.23
4.750	2.47	10.750	4.86	16.750	1.86	22.75	1.22
4.833	2.47	10.833	4.86	16.833	1.86	22.83	1.22
4.917	2.59	10.917	4.63	16.917	1.83	22.92	1.21
5.000	2.59	11.000	4.63	17.000	1.83	23.00	1.21
5.083	2.72	11.083	4.41	17.083	1.80	23.08	1.19
5.167	2.72	11.167	4.41	17.167	1.80	23.17	1.19
5.250	2.86	11.250	4.22	17.250	1.78	23.25	1.18
5.333	2.86	11.333	4.22	17.333	1.78	23.33	1.18
5.417	3.03	11.417	4.05	17.417	1.75	23.42	1.17
5.500	3.03	11.500	4.05	17.500	1.75	23.50	1.17
5.583	3.21	11.583	3.89	17.583	1.73	23.58</	

RUNOFF COEFFICIENT = 0.99 0.69 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0001):	0.27	0.106	8.00	115.85
+ ID2= 2 (0017):	0.77	0.026	9.00	125.77
=====				
ID = 3 (0022):	1.04	0.126	8.00	123.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	1.04	0.126	8.00	123.19
+ ID2= 2 (0019):	0.80	0.026	9.08	121.53
=====				
ID = 1 (0022):	1.84	0.146	8.00	122.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	1.84	0.146	8.00	122.29
+ ID2= 2 (0020):	0.72	0.026	8.92	125.86
=====				
ID = 3 (0022):	2.56	0.166	8.00	123.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	1.84	0.146	8.00	122.29
+ ID2= 2 (0020):	0.72	0.026	8.92	125.86
=====				
ID = 3 (0022):	2.56	0.166	8.00	123.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0035)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0002):	3.19	0.144	8.42	122.29
+ ID2= 2 (0030):	0.59	0.132	8.00	98.90
=====				
ID = 3 (0035):	3.78	0.249	8.17	118.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L (v 6.2.2016)

V V I SS U U A A L

V V I SS U U A A A A L

V V I SS U U A A L

V V I SSSS UUUU A A LLLLL

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voind.dat

Output filename:
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8103-5252-4bb3-a1a7-7c443e321e0c\scena

Summary filename:
C:\Users\schhom\AppData\Local\Civica\XH5\c4784722-8666-4e89-8a43-7d10dc9357ef\434c
8103-5252-4bb3-a1a7-7c443e321e0c\scena

DATE: 10-15-2024 TIME: 11:03:47

USER:

3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	2.56	0.166	8.00	123.42
+ ID2= 2 (0021):	0.44	0.208	8.00	129.49
=====				
ID = 1 (0022):	3.00	0.375	8.00	124.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	3.00	0.375	8.00	124.31
+ ID2= 2 (0036):	0.14	0.031	8.08	90.52
=====				
ID = 3 (0022):	3.14	0.402	8.00	122.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	3.14	0.402	8.00	122.81
+ ID2= 2 (0039):	0.05	0.011	8.08	90.52
=====				
ID = 1 (0022):	3.19	0.412	8.00	122.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0002)				
IN= 2--> OUT= 1				
DT= 5.0 min				
OVERFLOW IS OFF				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.1400	0.0413
	0.0900	0.0206	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (0022)	3.190	0.412	8.00	122.30
OUTFLOW: ID= 1 (0002)	3.190	0.144	8.42	122.29

PEAK FLOW REDUCTION [Qout/Qin](%)= 34.99
TIME SHIFT OF PEAK FLOW (min)= 25.00
MAXIMUM STORAGE USED (ha.m.)= 0.0413

COMMENTS:

** SIMULATION : 10yr_24hr Chicago **

| CHICAGO STORM | IDF curve parameters: A=1173.480
| Ptotal= 87.10 mm | B= 8.000
C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 24.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	0.78	6.00	2.62	12.00	2.31	18.00	1.10
0.17	0.79	6.17	2.85	12.17	2.24	18.17	1.09
0.33	0.81	6.33	3.13	12.33	2.17	18.33	1.07
0.50	0.82	6.50	3.48	12.50	2.10	18.50	1.06
0.67	0.84	6.67	3.92	12.67	2.04	18.67	1.05
0.83	0.85	6.83	4.52	12.83	1.98	18.83	1.03
1.00	0.87	7.00	5.36	13.00	1.93	19.00	1.02
1.17	0.89	7.17	6.65	13.17	1.88	19.17	1.01
1.33	0.91	7.33	8.89	13.33	1.83	19.33	1.00
1.50	0.93	7.50	13.75	13.50	1.79	19.50	0.99
1.67	0.95	7.67	32.93	13.67	1.74	19.67	0.97
1.83	0.97	7.83	118.25	13.83	1.70	19.83	0.96
2.00	0.99	8.00	42.97	14.00	1.66	20.00	0.95
2.17	1.01	8.17	22.94	14.17	1.63	20.17	0.94
2.33	1.04	8.33	15.63	14.33	1.59	20.33	0.93
2.50	1.07	8.50	11.90	14.50	1.56	20.50	0.92
2.67	1.09	8.67	9.65	14.67	1.53	20.67	0.91
2.83	1.12	8.83	8.15	14.83	1.50	20.83	0.90
3.00	1.16	9.00	7.07	15.00	1.47	21.00	0.89
3.17	1.19	9.17	6.26	15.17	1.44	21.17	0.88
3.33	1.23	9.33	5.62	15.33	1.42	21.33	0.88
3.50	1.27	9.50	5.12	15.50	1.39	21.50	0.87
3.67	1.31	9.67	4.70	15.67	1.37	21.67	0.86
3.83	1.35	9.83	4.35	15.83	1.34	21.83	0.85
4.00	1.40	10.00	4.06	16.00	1.32	22.00	0.84
4.17	1.45	10.17	3.80	16.17	1.30	22.17	0.83
4.33	1.51	10.33	3.58	16.33	1.28	22.33	0.83
4.50	1.57	10.50	3.38	16.50	1.26	22.50	0.82
4.67	1.64	10.67	3.21	16.67	1.24	22.67	0.81

4.83	1.72	10.83	3.06	16.83	1.22	22.83	0.80
5.00	1.81	11.00	2.92	17.00	1.20	23.00	0.80
5.17	1.90	11.17	2.79	17.17	1.18	23.17	0.79
5.33	2.01	11.33	2.68	17.33	1.17	23.33	0.78
5.50	2.13	11.50	2.57	17.50	1.15	23.50	0.78
5.67	2.27	11.67	2.48	17.67	1.13	23.67	0.77
5.83	2.43	11.83	2.39	17.83	1.12	23.83	0.76

CALIB
 NASHYD (0026) | Area (ha)= 0.28 Curve Number (CN)= 84.0
 ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
 U.H. Tp(hrs)= 0.22

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94

2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82
4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81
4.917	1.72	10.917	3.06	16.917	1.22	22.92	0.80
5.000	1.72	11.000	3.06	17.000	1.22	23.00	0.80
5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77
5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76

Unit Hyd Qpeak (cms)= 0.049
 PEAK FLOW (cms)= 0.029 (i)
 TIME TO PEAK (hrs)= 8.167
 RUNOFF VOLUME (mm)= 51.591

TOTAL RAINFALL (mm)= 87.103
 RUNOFF COEFFICIENT = 0.592

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0029) | Area (ha)= 0.18 Curve Number (CN)= 84.0
 ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
 U.H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92

2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82
4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81
4.917	1.72	10.917					

CALIB
STANDHYD (0028)
ID= 1 DT= 5.0 min

Area (ha)= 0.08
Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.08	0.00
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	23.09	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93

2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82
4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81
4.917	1.72	10.917	3.06	16.917	1.22	22.92	0.80
5.000	1.72	11.000	3.06	17.000	1.22	23.00	0.80
5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77
5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76

Max.Eff.Inten.(mm/hr)= 118.25
over (min) = 5.00
Storage Coeff. (min)= 0.99 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.34

TOTALS
0.026 (iii)

PEAK FLOW (cms)= 0.03 0.00

TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 86.10 51.66 85.76
TOTAL RAINFALL (mm)= 87.10 87.10 87.10
RUNOFF COEFFICIENT = 0.99 0.59 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0037)
ID= 1 DT= 5.0 min

Area (ha)= 0.04
Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.04	0.00
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	16.33	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00

1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38				

5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77
5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76
Max.Eff.Inten.(mm/hr)=	118.25		75.89				
over (min)	5.00		5.00				
Storage Coeff. (min)=	0.81 (ii)		1.86 (ii)				
Unit Hyd. Tpeak (min)=	5.00		5.00				
Unit Hyd. peak (cms)=	0.34		0.32				
PEAK FLOW (cms)=	0.01		0.00		0.013 (iii)		
TIME TO PEAK (hrs)=	8.00		8.00		8.00		
RUNOFF VOLUME (mm)=	86.10		56.18		83.33		
TOTAL RAINFALL (mm)=	87.10		87.10		87.10		
RUNOFF COEFFICIENT =	0.99		0.65		0.96		

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANDHYD (0038)	Area (ha)=	0.01		
ID= 1 DT= 5.0 min	Total Imp(%)=	99.00	Dir. Conn.(%)=	99.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.01	0.00
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	8.16	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07

0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.93	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82

4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81
4.917	1.72	10.917	3.06	16.917	1.22	22.92	0.80
5.000	1.72	11.000	3.06	17.000	1.22	23.00	0.80
5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77
5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76

Max.Eff.Inten.(mm/hr)=	118.25		75.89				
over (min)	5.00		5.00				
Storage Coeff. (min)=	0.53 (ii)		1.58 (ii)				
Unit Hyd. Tpeak (min)=	5.00		5.00				
Unit Hyd. peak (cms)=	0.34		0.33				
PEAK FLOW (cms)=	0.00		0.00		0.003 (iii)		
TIME TO PEAK (hrs)=	8.00		8.00		8.00		
RUNOFF VOLUME (mm)=	86.10		56.18		66.17		
TOTAL RAINFALL (mm)=	87.10		87.10		87.10		
RUNOFF COEFFICIENT =	0.99		0.65		0.76		

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0030)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0026):	0.28	0.029	8.17	51.59
+ ID2= 2 (0028):	0.08	0.026	8.00	85.76
ID = 3 (0030):	0.36	0.046	8.00	59.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0030):	0.36	0.046	8.00	59.18
+ ID2= 2 (0029):	0.18	0.021	8.08	51.47
ID = 1 (0030):	0.54	0.064	8.00	56.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0030):	0.54	0.064	8.00	56.61
+ ID2= 2 (0037):	0.04	0.013	8.00	83.33
ID = 3 (0030):	0.58	0.077	8.00	58.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0030):	0.58	0.077	8.00	58.46
+ ID2= 2 (0038):	0.01	0.003	8.00	66.17
ID = 1 (0030):	0.59	0.081	8.00	58.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (0031)	Area (ha)=	0.09	Curve Number (CN)=	84.0
ID= 1 DT= 5.0 min	Ia (mm)=	5.00	# of Linear Res.(N)=	3.00
	U.H. Tp(hrs)=	0.17		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10

0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83

4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82
4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81
4.917	1.72	10.917	3.06	16.917	1.22	22.92	0.80
5.000	1.72	11.000	3.06	17.000	1.22	23.00	0.80
5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77
5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76

Unit Hyd Qpeak (cms) = 0.020

PEAK FLOW (cms) = 0.011 (i)
 TIME TO PEAK (hrs) = 8.083
 RUNOFF VOLUME (mm) = 51.471
 TOTAL RAINFALL (mm) = 87.03
 RUNOFF COEFFICIENT = 0.591

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		Area (ha) =	0.71
STANDHYD (0018)		Total Imp(%) =	99.00
ID= 1 DT= 5.0 min		Dir. Conn.(%) =	99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.70 0.01
 Dep. Storage (mm) = 1.00 5.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 68.00 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr

0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84

4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82
4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81
4.917	1.72	10.917	3.06	16.917	1.22	22.92	0.80
5.000	1.72	11.000	3.06	17.000	1.22	23.00	0.80
5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75</	

+ ID2= 2 (0031): 0.09 0.011 8.08 51.47
 =====
 ID = 3 (0032): 0.80 0.240 8.00 81.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR(0019)		OVERFLOW IS OFF			
IN= 2--> OUT= 1		OUTFLOW (cms)		STORAGE (ha.m.)	
DT= 5.0 min		OUTFLOW (cms)		STORAGE (ha.m.)	
0.0000	0.0000	0.0190	0.0279		
0.0000	0.0028	0.0200	0.0307		
0.0000	0.0056	0.0210	0.0335		
0.0100	0.0084	0.0220	0.0363		
0.0120	0.0112	0.0230	0.0391		
0.0140	0.0140	0.0240	0.0419		
0.0150	0.0168	0.0250	0.0447		
0.0160	0.0196	0.0250	0.0475		
0.0170	0.0223	0.0260	0.0499		
0.0180	0.0251	0.0000	0.0000		
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0032)		0.800	0.240	8.00	81.90
OUTFLOW: ID= 1 (0019)		0.800	0.020	8.83	78.31
PEAK FLOW REDUCTION [Qout/Qin](%)= 8.51					
TIME SHIFT OF PEAK FLOW (min)= 50.00					
MAXIMUM STORAGE USED (ha.m.)= 0.0320					

CALIB		Area (ha)= 0.14		Curve Number (CN)= 84.0	
NASHYD (0036)		Ia (mm)= 5.00		# of Linear Res. (N)= 3.00	
ID= 1 DT= 5.0 min		U.H. Tp(hrs)= 0.16			

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07

0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82

4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81
4.917	1.72	10.917	3.06	16.917	1.22	22.92	0.80
5.000	1.72	11.000	3.06	17.000	1.22	23.00	0.80
5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77
5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76

Unit Hyd Qpeak (cms)= 0.033

PEAK FLOW (cms)= 0.017 (i)
 TIME TO PEAK (hrs)= 8.083
 RUNOFF VOLUME (mm)= 51.425
 TOTAL RAINFALL (mm)= 87.103
 RUNOFF COEFFICIENT = 0.590

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		Area (ha)= 0.05		Curve Number (CN)= 84.0	
NASHYD (0039)		Ia (mm)= 5.00		# of Linear Res. (N)= 3.00	
ID= 1 DT= 5.0 min		U.H. Tp(hrs)= 0.16			

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05

0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35</				

5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77
5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76

Unit Hyd Qpeak (cms)= 0.012

PEAK FLOW (cms)= 0.006 (i)
 TIME TO PEAK (hrs)= 8.083
 RUNOFF VOLUME (mm)= 51.420
 TOTAL RAINFALL (mm)= 87.103
 RUNOFF COEFFICIENT = 0.590

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0001) Area (ha)= 0.27
 ID= 1 DT= 5.0 min Total Imp(%)= 64.00 Dir. Conn.(%)= 64.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.17	0.10
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	42.43	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06

0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82
4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81

4.917	1.72	10.917	3.06	16.917	1.22	22.92	0.80
5.000	1.72	11.000	3.06	17.000	1.22	23.00	0.80
5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77
5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76

Max.Eff.Inten.(mm/hr)= 118.25 68.98
 over (min) 5.00 10.00
 Storage Coeff. (min)= 1.43 (ii) 9.62 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.11

PEAK FLOW (cms)= 0.06 0.01 0.067 (iii)
 TIME TO PEAK (hrs)= 8.00 8.08 8.00
 RUNOFF VOLUME (mm)= 86.10 51.66 73.68
 TOTAL RAINFALL (mm)= 87.10 87.10 87.10
 RUNOFF COEFFICIENT = 0.99 0.59 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0014) Area (ha)= 0.72
 ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.71	0.01
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	69.28	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.5					

3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82
4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81
4.917	1.72	10.917	3.06	16.917	1.22	22.92	0.80
5.000	1.72	11.000	3.06	17.000	1.22	23.00	0.80
5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77
5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76

Max.Eff.Inten.(mm/hr)= 118.25 68.98
over (min) 5.00 5.00
Storage Coeff. (min)= 1.92 (ii) 2.97 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.31 0.28

PEAK FLOW (cms)= 0.23 0.00
TIME TO PEAK (hrs)= 8.00 8.00
RUNOFF VOLUME (mm)= 86.10 51.66
TOTAL RAINFALL (mm)= 87.10 87.10
RUNOFF COEFFICIENT = 0.99 0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0020) OVERFLOW IS OFF

0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99
1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82
4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81

IN= 2---> OUT= 1			
DT= 5.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0255
0.0000	0.0025	0.0200	0.0280
0.0080	0.0051	0.0210	0.0306
0.0100	0.0076	0.0220	0.0331
0.0120	0.0102	0.0230	0.0357
0.0140	0.0127	0.0240	0.0382
0.0150	0.0153	0.0250	0.0407
0.0160	0.0178	0.0250	0.0433
0.0170	0.0204	0.0260	0.0455
0.0180	0.0229	0.0000	0.0000

AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
INFLOW : ID= 2 (0014) 0.720 0.235 8.00 85.76
OUTFLOW : ID= 1 (0020) 0.720 0.021 8.75 82.12

PEAK FLOW REDUCTION [Qout/Qin](%) = 8.79
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.0296

CALIB STANDHYD (0016) Area (ha)= 0.77
ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.76 0.01
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 71.65 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10	18.08	1.10	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10	18.17	1.10	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09	18.25	1.09	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09	18.33	1.09	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07	18.42	1.07	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07	18.50	1.07	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06	18.58	1.06	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06	18.67	1.06	18.67	1.06

Max.Eff.Inten.(mm/hr)= 118.25 68.98
over (min) 5.00 5.00
Storage Coeff. (min)= 1.96 (ii) 3.01 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.31 0.28

PEAK FLOW (cms)= 0.25 0.00
TIME TO PEAK (hrs)= 8.00 8.00
RUNOFF VOLUME (mm)= 86.10 51.66
TOTAL RAINFALL (mm)= 87.10 87.10
RUNOFF COEFFICIENT = 0.99 0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0017) OVERFLOW IS OFF			
IN= 2---> OUT= 1			
DT= 5.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0279
0.0000	0.0028	0.0200	0.0307
0.0080	0.0056	0.0210	0.0334
0.0100	0.0084	0.0220	0.0362
0.0120	0.0111	0.0230	0.0390
0.0140	0.0139	0.0240	0.0418
0.0150	0.0167	0.0250	0.0446
0.0160	0.0195	0.0250	0.0474
0.0170	0.0223	0.0260	0.0498

0.0180 0.0251 | 0.0000 0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (0016)	0.770	0.251	8.00	85.76
OUTFLOW: ID= 1 (0017)	0.770	0.021	8.83	82.03

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.22
 TIME SHIFT OF PEAK FLOW (min)= 50.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0324

CALIB
 STANDHYD (0021)
 ID= 1 DT= 5.0 min

Area (ha)= 0.44
 Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	0.44	0.00
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	54.16	40.00
Mannings n	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	0.78	6.083	2.62	12.083	2.31	18.08	1.10
0.167	0.78	6.167	2.62	12.167	2.31	18.17	1.10
0.250	0.79	6.250	2.85	12.250	2.24	18.25	1.09
0.333	0.79	6.333	2.85	12.333	2.24	18.33	1.09
0.417	0.81	6.417	3.13	12.417	2.17	18.42	1.07
0.500	0.81	6.500	3.13	12.500	2.17	18.50	1.07
0.583	0.82	6.583	3.48	12.583	2.10	18.58	1.06
0.667	0.82	6.667	3.48	12.667	2.10	18.67	1.06
0.750	0.84	6.750	3.92	12.750	2.04	18.75	1.05
0.833	0.84	6.833	3.92	12.833	2.04	18.83	1.05
0.917	0.85	6.917	4.52	12.917	1.98	18.92	1.03
1.000	0.85	7.000	4.52	13.000	1.98	19.00	1.03
1.083	0.87	7.083	5.36	13.083	1.93	19.08	1.02
1.167	0.87	7.167	5.36	13.167	1.93	19.17	1.02
1.250	0.89	7.250	6.65	13.250	1.88	19.25	1.01
1.333	0.89	7.333	6.65	13.333	1.88	19.33	1.01
1.417	0.91	7.417	8.89	13.417	1.83	19.42	1.00
1.500	0.91	7.500	8.89	13.500	1.83	19.50	1.00
1.583	0.93	7.583	13.75	13.583	1.79	19.58	0.99
1.667	0.93	7.667	13.75	13.667	1.79	19.67	0.99

1.750	0.95	7.750	32.93	13.750	1.74	19.75	0.97
1.833	0.95	7.833	32.94	13.833	1.74	19.83	0.97
1.917	0.97	7.917	118.25	13.917	1.70	19.92	0.96
2.000	0.97	8.000	118.24	14.000	1.70	20.00	0.96
2.083	0.99	8.083	42.97	14.083	1.66	20.08	0.95
2.167	0.99	8.167	42.97	14.167	1.66	20.17	0.95
2.250	1.01	8.250	22.94	14.250	1.63	20.25	0.94
2.333	1.01	8.333	22.94	14.333	1.63	20.33	0.94
2.417	1.04	8.417	15.63	14.417	1.59	20.42	0.93
2.500	1.04	8.500	15.63	14.500	1.59	20.50	0.93
2.583	1.07	8.583	11.90	14.583	1.56	20.58	0.92
2.667	1.07	8.667	11.90	14.667	1.56	20.67	0.92
2.750	1.09	8.750	9.65	14.750	1.53	20.75	0.91
2.833	1.09	8.833	9.65	14.833	1.53	20.83	0.91
2.917	1.12	8.917	8.15	14.917	1.50	20.92	0.90
3.000	1.12	9.000	8.15	15.000	1.50	21.00	0.90
3.083	1.16	9.083	7.07	15.083	1.47	21.08	0.89
3.167	1.16	9.167	7.07	15.167	1.47	21.17	0.89
3.250	1.19	9.250	6.26	15.250	1.44	21.25	0.88
3.333	1.19	9.333	6.26	15.333	1.44	21.33	0.88
3.417	1.23	9.417	5.62	15.417	1.42	21.42	0.88
3.500	1.23	9.500	5.62	15.500	1.42	21.50	0.88
3.583	1.27	9.583	5.12	15.583	1.39	21.58	0.87
3.667	1.27	9.667	5.12	15.667	1.39	21.67	0.87
3.750	1.31	9.750	4.70	15.750	1.37	21.75	0.86
3.833	1.31	9.833	4.70	15.833	1.37	21.83	0.86
3.917	1.35	9.917	4.35	15.917	1.34	21.92	0.85
4.000	1.35	10.000	4.35	16.000	1.34	22.00	0.85
4.083	1.40	10.083	4.06	16.083	1.32	22.08	0.84
4.167	1.40	10.167	4.06	16.167	1.32	22.17	0.84
4.250	1.45	10.250	3.80	16.250	1.30	22.25	0.83
4.333	1.45	10.333	3.80	16.333	1.30	22.33	0.83
4.417	1.51	10.417	3.58	16.417	1.28	22.42	0.83
4.500	1.51	10.500	3.58	16.500	1.28	22.50	0.83
4.583	1.57	10.583	3.38	16.583	1.26	22.58	0.82
4.667	1.57	10.667	3.38	16.667	1.26	22.67	0.82
4.750	1.64	10.750	3.21	16.750	1.24	22.75	0.81
4.833	1.64	10.833	3.21	16.833	1.24	22.83	0.81
4.917	1.72	10.917	3.06	16.917	1.22	22.92	0.80
5.000	1.72	11.000	3.06	17.000	1.22	23.00	0.80
5.083	1.81	11.083	2.92	17.083	1.20	23.08	0.80
5.167	1.81	11.167	2.92	17.167	1.20	23.17	0.80
5.250	1.90	11.250	2.79	17.250	1.18	23.25	0.79
5.333	1.90	11.333	2.79	17.333	1.18	23.33	0.79
5.417	2.01	11.417	2.68	17.417	1.17	23.42	0.78
5.500	2.01	11.500	2.68	17.500	1.17	23.50	0.78
5.583	2.13	11.583	2.57	17.583	1.15	23.58	0.78
5.667	2.13	11.667	2.57	17.667	1.15	23.67	0.78
5.750	2.27	11.750	2.48	17.750	1.13	23.75	0.77
5.833	2.27	11.833	2.48	17.833	1.13	23.83	0.77

5.917	2.43	11.917	2.39	17.917	1.12	23.92	0.76
6.000	2.43	12.000	2.39	18.000	1.12	24.00	0.76

Max.Eff.Inten.(mm/hr)= 118.25 over (min)= 5.00
 Storage Coeff. (min)= 1.65 (ii) 2.70 (ii)
 Unit Hyd. Tpeak (min)= 5.00
 Unit Hyd. peak (cms)= 0.32 0.29

TOTALS
 PEAK FLOW (cms)= 0.14 0.00 0.144 (iii)
 TIME TO PEAK (hrs)= 8.00 8.00
 RUNOFF VOLUME (mm)= 86.10 51.66 85.76
 TOTAL RAINFALL (mm)= 87.10 87.10 87.10
 RUNOFF COEFFICIENT = 0.99 0.59 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0022)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0001):	0.27	0.067	8.00	73.68
+ ID2= 2 (0017):	0.77	0.021	8.83	82.03
=====				
ID = 3 (0022):	1.04	0.084	8.00	79.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
 3 + 2 = 1

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0022):	1.04	0.084	8.00	79.86
+ ID2= 2 (0019):	0.80	0.020	8.83	78.31
=====				
ID = 1 (0022):	1.84	0.100	8.00	79.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0022):	1.84	0.100	8.00	79.19
+ ID2= 2 (0020):	0.72	0.021	8.75	82.12
=====				
ID = 3 (0022):	2.56	0.117	8.00	80.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
 3 + 2 = 1

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0022):	2.56	0.117	8.00	80.01
+ ID2= 2 (0021):	0.44	0.144	8.00	85.76
=====				
ID = 1 (0022):	3.00	0.260	8.00	80.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0022):	3.00	0.260	8.00	80.86
+ ID2= 2 (0036):	0.14	0.017	8.08	51.42
=====				
ID = 3 (0022):	3.14	0.275	8.00	79.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
 3 + 2 = 1

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0022):	3.14	0.275	8.00	79.54
+ ID2= 2 (0039):	0.05	0.006	8.08	51.42
=====				
ID = 1 (0022):	3.19	0.280	8.00	79.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR(0002) OVERFLOW IS OFF
 IN= 2----> OUT= 1
 DT= 5.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
---------	---------	---------	---------

```

-----
      (cms)   (ha.m.) | (cms)   (ha.m.)
      0.0000  0.0000 | 0.1440  0.0413
      0.0980  0.0206 | 0.0000  0.0000

      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
INFLOW : ID= 2 ( 0022)  3.190  0.280  8.00  79.10
OUTFLOW: ID= 1 ( 0002)  3.190  0.110  8.42  79.09

      PEAK FLOW REDUCTION [Qout/Qin](%)= 39.33
      TIME SHIFT OF PEAK FLOW (min)= 25.00
      MAXIMUM STORAGE USED (ha.m.)= 0.0263
  
```

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-----
| ADD HYD ( 0035) |
| 1 + 2 = 3 |
-----
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
      ID1= 1 ( 0002):  3.19  0.110  8.42  79.09
+      ID2= 2 ( 0030):  0.59  0.081  8.00  58.59
-----
      ID = 3 ( 0035):  3.78  0.172  8.17  75.89
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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=====
V V I SSSS U U A L (v 6.2.2016)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O O
O O T T H H Y Y M M O O O
OOO T T H H Y Y M M OOO
  
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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename:

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39

C:\Users\schhom\AppData\Local\Civica\VH5\c4784722-8666-4e89-8a43-7d10dc9357ef\2e61f049-4374-4e66-984f-c029384a4582\scena
Summary filename:
C:\Users\schhom\AppData\Local\Civica\VH5\c4784722-8666-4e89-8a43-7d10dc9357ef\2e61f049-4374-4e66-984f-c029384a4582\scena

DATE: 10-15-2024 TIME: 11:03:45
USER:

COMMENTS:

** SIMULATION : 25mm **

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| READ STORM | Filename: C:\Users\schhom\AppData\Local\Temp\
|             | e961c6e4-ab79-4906-8111-09011bd8104d\44e9be46
| Ptotal= 25.00 mm | Comments: 25MM24HRC_60min Edited 2012
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```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	0.35	6.00	0.95	12.00	0.87	18.00	0.47
1.00	0.38	7.00	1.80	13.00	0.74	19.00	0.44
2.00	0.42	8.00	8.37	14.00	0.66	20.00	0.41
3.00	0.48	9.00	2.23	15.00	0.59	21.00	0.39
4.00	0.56	10.00	1.38	16.00	0.54	22.00	0.37
5.00	0.70	11.00	1.05	17.00	0.50	23.00	0.36

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| CALIB |
| NASHYD ( 0026) | Area (ha)= 0.28 Curve Number (CN)= 84.0
| ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
|             | U.H. Tp(hrs)= 0.22
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NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.
---- TRANSFORMED HYETOGRAPH ----

4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36
5.667	0.70	11.667	1.05	17.667	0.50	23.67	0.36
5.750	0.70	11.750	1.05	17.750	0.50	23.75	0.36
5.833	0.70	11.833	1.05	17.833	0.50	23.83	0.36
5.917	0.70	11.917	1.05	17.917	0.50	23.92	0.36
6.000	0.70	12.000	1.05	18.000	0.50	24.00	0.36

Unit Hyd Qpeak (cms)= 0.049
PEAK FLOW (cms)= 0.001 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 5.838
TOTAL RAINFALL (mm)= 24.996
RUNOFF COEFFICIENT = 0.234
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| CALIB |
| NASHYD ( 0029) | Area (ha)= 0.18 Curve Number (CN)= 84.0
| ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
|             | U.H. Tp(hrs)= 0.17
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NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.
---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47

0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37

4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36
5.667	0.70	11.667	1.05	17.667	0.50	23.67	0.36
5.750	0.70	11.750	1.05	17.750	0.50	23.75	0.36
5.833	0.70	11.833	1.05	17.833	0.50	23.83	0.36
5.917	0.70	11.917	1.05	17.917	0.50	23.92	0.36
6.000	0.70	12.000	1.05	18.000	0.50	24.00	0.36

Unit Hyd Qpeak (cms) = 0.040
PEAK FLOW (cms) = 0.001 (i)
TIME TO PEAK (hrs) = 9.000
RUNOFF VOLUME (mm) = 5.824
TOTAL RAINFALL (mm) = 24.996
RUNOFF COEFFICIENT = 0.233

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha) = 0.08
STANDHYD (0028)	Total Imp(%) = 99.00
ID= 1 DT= 5.0 min	Dir. Conn.(%) = 99.00

Surface Area (ha) =	0.08	IMPERVIOUS	0.00	PERVIOUS (i)	0.00
Dep. Storage (mm) =	1.00		5.00		
Average Slope (%) =	1.00		2.00		
Length (m) =	23.00		40.00		
Mannings n =	0.013		0.250		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr

0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37

4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36
5.667	0.70	11.667	1.05	17.667	0.50	23.67	0.36
5.750	0.70	11.750	1.05				

Surface Area (ha)= 0.04 0.00
 Dep. Storage (mm)= 1.00 1.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 16.33 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39

3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36
5.667	0.70	11.667	1.05	17.667	0.50	23.67	0.36
5.750	0.70	11.750	1.05	17.750	0.50	23.75	0.36
5.833	0.70	11.833	1.05	17.833	0.50	23.83	0.36
5.917	0.70	11.917	1.05	17.917	0.50	23.92	0.36
6.000	0.70	12.000	1.05	18.000	0.50	24.00	0.36

Max.Eff.Inten.(mm/hr)= 8.37 3.12
 over (min) 5.00 10.00
 Storage Coeff. (min)= 2.32 (ii) 5.35 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.30 0.16

PEAK FLOW (cms)= 0.00 0.00 *TOTALS*
 TIME TO PEAK (hrs)= 8.42 9.00 0.001 (iii)
 RUNOFF VOLUME (mm)= 24.00 8.08 9.00
 TOTAL RAINFALL (mm)= 25.00 25.00 25.00
 RUNOFF COEFFICIENT = 0.96 0.32 0.70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 85.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0038)
 ID= 1 DT= 5.0 min

Area (ha)= 0.01
 Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.01 0.00
 Dep. Storage (mm)= 1.00 1.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 8.16 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41

2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.						

Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.33 0.24

PEAK FLOW (cms)= 0.00 0.00 0.000 (iii)
 TIME TO PEAK (hrs)= 8.33 9.00 9.00
 RUNOFF VOLUME (mm)= 24.00 8.08 12.87
 TOTAL RAINFALL (mm)= 25.00 25.00 25.00
 RUNOFF COEFFICIENT = 0.96 0.32 0.51

ID = 3 (0030): 0.58 0.005 9.00 8.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)
 3 + 2 = 1

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0030):	0.58	0.005	9.00	8.85
+ ID2= 2 (0038):	0.01	0.000	9.00	12.87
=====				
ID = 1 (0030):	0.59	0.005	9.00	8.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0031) Area (ha)= 0.09 Curve Number (CN)= 84.0
 ID= 1 DT= 5.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
 U.H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

ADD HYD (0030)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0026):	0.28	0.001	9.00	5.84
+ ID2= 2 (0028):	0.08	0.002	9.00	21.85
=====				
ID = 3 (0030):	0.36	0.003	9.00	9.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)
 3 + 2 = 1

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0030):	0.36	0.003	9.00	9.40
+ ID2= 2 (0029):	0.18	0.001	9.00	5.82
=====				
ID = 1 (0030):	0.54	0.004	9.00	8.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0030):	0.54	0.004	9.00	8.21
+ ID2= 2 (0037):	0.04	0.001	9.00	17.51
=====				

--- TRANSFORMED HYETOGRAPH ---

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44

1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36
5.667	0.70	11.667	1.05	17.667	0.50	23.67	0.36
5.750	0.70	11.750	1.05	17.750	0.50	23.75	0.36
5.833	0.70	11.833	1.05	17.833	0.50	23.83	0.36
5.917	0.70	11.917	1.05	17.917	0.50	23.92	0.36
6.000	0.70	12.000	1.05	18.000	0.50	24.00	0.36

Unit Hyd Qpeak (cms)= 0.020

PEAK FLOW (cms)= 0.001 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 5.820
 TOTAL RAINFALL (mm)= 24.996
 RUNOFF COEFFICIENT = 0.233

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0018) Area (ha)= 0.71
 ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.70	0.01
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	68.80	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44

1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36
5.667	0.70	11.667	1.05	17.667	0.50	23.67	0.36
5.750	0.70	11.750	1.05	17.750	0.50	23.75	0.36
5.833	0.70	11.833	1.05	17.833	0.50	23.83	0.36

5.917	0.70	11.917	1.05	17.917	0.50	23.92	0.36
6.000	0.70	12.000	1.05	18.000	0.50	24.00	0.36
Max.Eff.Inten.(mm/hr)=	8.37	2.27					
over (min)	5.00	10.00					
Storage Coeff. (min)=	5.51 (ii)	8.54 (ii)					
Unit Hyd. Tpeak (min)=	5.00	10.00					
Unit Hyd. peak (cms)=	0.20	0.12					
PEAK FLOW (cms)=	0.02	0.00				0.016 (iii)	
TIME TO PEAK (hrs)=	9.00	9.00				9.00	
RUNOFF VOLUME (mm)=	24.00	5.85				23.81	
TOTAL RAINFALL (mm)=	25.00	25.00				25.00	
RUNOFF COEFFICIENT =	0.96	0.23				0.95	

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0032)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0018):	0.71	0.016	9.00	23.81
+ ID2= 2 (0031):	0.09	0.001	9.00	5.82
=====				
ID = 3 (0032):	0.80	0.017	9.00	21.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0019)				
OVERFLOW IS OFF				
IN= 2---> OUT= 1				
DT= 5.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.0190	0.0279
	0.0000	0.0028	0.0200	0.0307
	0.0080	0.0056	0.0210	0.0335
	0.0100	0.0084	0.0220	0.0363
	0.0120	0.0112	0.0230	0.0391
	0.0140	0.0140	0.0240	0.0419
	0.0150	0.0168	0.0250	0.0447
	0.0160	0.0196	0.0250	0.0475
	0.0170	0.0223	0.0260	0.0499
	0.0180	0.0251	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0032)	0.800	0.017	9.00
OUTFLOW: ID= 1 (0019)	0.800	0.009	9.08

PEAK FLOW REDUCTION [Qout/Qin](%)= 52.14
 TIME SHIFT OF PEAK FLOW (min)= 5.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0067

CALIB				
NASHYD (0036)				
ID= 1 DT= 5.0 min	Area (ha)=	0.14	Curve Number (CN)=	84.0
	Ia (mm)=	5.00	# of Linear Res.(N)=	3.00
	U.H. Tp(hrs)=	0.16		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41

2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50		

RUNOFF VOLUME (mm)= 5.819
 TOTAL RAINFALL (mm)= 24.996
 RUNOFF COEFFICIENT = 0.233

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0039) Area (ha)= 0.05 Curve Number (CN)= 84.0
 ID= 1 DT= 5.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
 U.H. Tp(hrs)= 0.16

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41

2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36
5.667	0.70	11.667	1.05	17.667	0.50	23.67	0.36
5.750	0.70	11.750	1.05	17.750	0.50	23.75	0.36
5.833	0.70	11.833	1.05	17.833	0.50	23.83	0.36
5.917	0.70	11.917	1.05	17.917	0.50	23.92	0.36
6.000	0.70	12.000	1.05	18.000	0.50	24.00	0.36

Unit Hyd Qpeak (cms)= 0.012

PEAK FLOW (cms)= 0.000 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 5.810
 TOTAL RAINFALL (mm)= 24.996
 RUNOFF COEFFICIENT = 0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0001) Area (ha)= 0.27
 ID= 1 DT= 5.0 min Total Imp(%)= 64.00 Dir. Conn.(%)= 64.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.17 0.10
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 42.43 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41

2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.			

PEAK FLOW (cms)= 0.00 0.00 0.004 (iii)
 TIME TO PEAK (hrs)= 8.75 9.50 9.00
 RUNOFF VOLUME (mm)= 24.00 5.85 17.39
 TOTAL RAINFALL (mm)= 25.00 25.00 25.00
 RUNOFF COEFFICIENT = 0.96 0.23 0.70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 STANDHYD (0014) Area (ha)= 0.72
 ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.71 0.01
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 69.28 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44

1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36

5.667 0.70 | 11.667 1.05 | 17.667 0.50 | 23.67 0.36
 5.750 0.70 | 11.750 1.05 | 17.750 0.50 | 23.75 0.36
 5.833 0.70 | 11.833 1.05 | 17.833 0.50 | 23.83 0.36
 5.917 0.70 | 11.917 1.05 | 17.917 0.50 | 23.92 0.36
 6.000 0.70 | 12.000 1.05 | 18.000 0.50 | 24.00 0.36

Max.Eff.Inten.(mm/hr)= 8.37 2.27
 over (min) 5.00 10.00
 Storage Coeff. (min)= 5.53 (ii) 8.56 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.20 0.12

 TOTALS
 PEAK FLOW (cms)= 0.02 0.00 0.017 (iii)
 TIME TO PEAK (hrs)= 9.00 9.00 9.00
 RUNOFF VOLUME (mm)= 24.00 5.85 23.81
 TOTAL RAINFALL (mm)= 25.00 25.00 25.00
 RUNOFF COEFFICIENT = 0.96 0.23 0.95

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR(0020) OVERFLOW IS OFF
 IN= 2--> OUT= 1
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0255
0.0000	0.0025	0.0200	0.0280
0.0000	0.0051	0.0210	0.0306
0.0100	0.0076	0.0220	0.0331
0.0120	0.0102	0.0230	0.0357
0.0140	0.0127	0.0240	0.0382
0.0150	0.0153	0.0250	0.0407
0.0160	0.0178	0.0250	0.0433
0.0170	0.0204	0.0260	0.0455
0.0180	0.0229	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0014) 0.720 0.017 9.00 23.81
 OUTFLOW: ID= 1 (0020) 0.720 0.009 9.08 20.17

 PEAK FLOW REDUCTION [Qout/Qin](%)= 54.42
 TIME SHIFT OF PEAK FLOW (min)= 5.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0064

 CALIB
 STANDHYD (0016) Area (ha)= 0.77
 ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.76 0.01
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 71.65 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41

2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39
3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36
5.667	0.70	11.667	1.05	17.667	0.50	23.67	0.36
5.750	0.70	11.750	1.05	17.750	0.50	23.75	0.36
5.833	0.70	11.833	1.05	17.833	0.50	23.83	0.36
5.917	0.70	11.917	1.05	17.917	0.50	23.92	0.36
6.000	0.70	12.000	1.05	18.000	0.50	24.00	0.36

Max.Eff.Inten.(mm/hr)= 8.37
over (min) 5.00
Storage Coeff. (min)= 5.64 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.20

TOTALS
0.018 (iii)

PEAK FLOW (cms)= 0.02 0.00 0.018 (iii)

TIME TO PEAK (hrs)= 9.00 9.00 9.00
RUNOFF VOLUME (mm)= 24.00 5.85 23.81
TOTAL RAINFALL (mm)= 25.00 25.00 25.00
RUNOFF COEFFICIENT = 0.96 0.23 0.95

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0017)				
OVERFLOW IS OFF				
IN= 2----> OUT= 1				
DT= 5.0 min				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	0.0190	0.0279	
0.0000	0.0028	0.0200	0.0307	
0.0080	0.0056	0.0210	0.0334	
0.0100	0.0084	0.0220	0.0362	
0.0120	0.0111	0.0230	0.0390	
0.0140	0.0139	0.0240	0.0418	
0.0150	0.0167	0.0250	0.0446	
0.0160	0.0195	0.0250	0.0474	
0.0170	0.0223	0.0260	0.0498	
0.0180	0.0251	0.0000	0.0000	
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
0.770	0.018	9.00	23.81	
OUTFLOW: ID= 1 (0017)	0.770	0.009	9.08	20.09
PEAK FLOW REDUCTION [Qout/Qin](%)= 51.13				
TIME SHIFT OF PEAK FLOW (min)= 5.00				
MAXIMUM STORAGE USED (ha.m.)= 0.0071				

CALIB STANDHYD (0021)
Area (ha)= 0.44
Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.44 0.00
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 54.16 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	0.35	6.083	0.95	12.083	0.87	18.08	0.47
0.167	0.35	6.167	0.95	12.167	0.87	18.17	0.47
0.250	0.35	6.250	0.95	12.250	0.87	18.25	0.47
0.333	0.35	6.333	0.95	12.333	0.87	18.33	0.47
0.417	0.35	6.417	0.95	12.417	0.87	18.42	0.47
0.500	0.35	6.500	0.95	12.500	0.87	18.50	0.47
0.583	0.35	6.583	0.95	12.583	0.87	18.58	0.47
0.667	0.35	6.667	0.95	12.667	0.87	18.67	0.47
0.750	0.35	6.750	0.95	12.750	0.87	18.75	0.47
0.833	0.35	6.833	0.95	12.833	0.87	18.83	0.47
0.917	0.35	6.917	0.95	12.917	0.87	18.92	0.47
1.000	0.35	7.000	0.95	13.000	0.87	19.00	0.47
1.083	0.38	7.083	1.80	13.083	0.74	19.08	0.44
1.167	0.38	7.167	1.80	13.167	0.74	19.17	0.44
1.250	0.38	7.250	1.80	13.250	0.74	19.25	0.44
1.333	0.38	7.333	1.80	13.333	0.74	19.33	0.44
1.417	0.38	7.417	1.80	13.417	0.74	19.42	0.44
1.500	0.38	7.500	1.80	13.500	0.74	19.50	0.44
1.583	0.38	7.583	1.80	13.583	0.74	19.58	0.44
1.667	0.38	7.667	1.80	13.667	0.74	19.67	0.44
1.750	0.38	7.750	1.80	13.750	0.74	19.75	0.44
1.833	0.38	7.833	1.80	13.833	0.74	19.83	0.44
1.917	0.38	7.917	1.80	13.917	0.74	19.92	0.44
2.000	0.38	8.000	1.80	14.000	0.74	20.00	0.44
2.083	0.42	8.083	8.37	14.083	0.66	20.08	0.41
2.167	0.42	8.167	8.37	14.167	0.66	20.17	0.41
2.250	0.42	8.250	8.37	14.250	0.66	20.25	0.41
2.333	0.42	8.333	8.37	14.333	0.66	20.33	0.41
2.417	0.42	8.417	8.37	14.417	0.66	20.42	0.41
2.500	0.42	8.500	8.37	14.500	0.66	20.50	0.41
2.583	0.42	8.583	8.37	14.583	0.66	20.58	0.41
2.667	0.42	8.667	8.37	14.667	0.66	20.67	0.41
2.750	0.42	8.750	8.37	14.750	0.66	20.75	0.41
2.833	0.42	8.833	8.37	14.833	0.66	20.83	0.41
2.917	0.42	8.917	8.37	14.917	0.66	20.92	0.41
3.000	0.42	9.000	8.37	15.000	0.66	21.00	0.41
3.083	0.48	9.083	2.23	15.083	0.59	21.08	0.39
3.167	0.48	9.167	2.23	15.167	0.59	21.17	0.39
3.250	0.48	9.250	2.23	15.250	0.59	21.25	0.39
3.333	0.48	9.333	2.23	15.333	0.59	21.33	0.39
3.417	0.48	9.417	2.23	15.417	0.59	21.42	0.39
3.500	0.48	9.500	2.23	15.500	0.59	21.50	0.39
3.583	0.48	9.583	2.23	15.583	0.59	21.58	0.39

3.667	0.48	9.667	2.23	15.667	0.59	21.67	0.39
3.750	0.48	9.750	2.23	15.750	0.59	21.75	0.39
3.833	0.48	9.833	2.23	15.833	0.59	21.83	0.39
3.917	0.48	9.917	2.23	15.917	0.59	21.92	0.39
4.000	0.48	10.000	2.23	16.000	0.59	22.00	0.39
4.083	0.56	10.083	1.38	16.083	0.54	22.08	0.37
4.167	0.56	10.167	1.38	16.167	0.54	22.17	0.37
4.250	0.56	10.250	1.38	16.250	0.54	22.25	0.37
4.333	0.56	10.333	1.38	16.333	0.54	22.33	0.37
4.417	0.56	10.417	1.38	16.417	0.54	22.42	0.37
4.500	0.56	10.500	1.38	16.500	0.54	22.50	0.37
4.583	0.56	10.583	1.38	16.583	0.54	22.58	0.37
4.667	0.56	10.667	1.38	16.667	0.54	22.67	0.37
4.750	0.56	10.750	1.38	16.750	0.54	22.75	0.37
4.833	0.56	10.833	1.38	16.833	0.54	22.83	0.37
4.917	0.56	10.917	1.38	16.917	0.54	22.92	0.37
5.000	0.56	11.000	1.38	17.000	0.54	23.00	0.37
5.083	0.70	11.083	1.05	17.083	0.50	23.08	0.36
5.167	0.70	11.167	1.05	17.167	0.50	23.17	0.36
5.250	0.70	11.250	1.05	17.250	0.50	23.25	0.36
5.333	0.70	11.333	1.05	17.333	0.50	23.33	0.36
5.417	0.70	11.417	1.05	17.417	0.50	23.42	0.36
5.500	0.70	11.500	1.05	17.500	0.50	23.50	0.36
5.583	0.70	11.583	1.05	17.583	0.50	23.58	0.36
5.667	0.70	11.667	1.05	17.667	0.50	23.67	0.36
5.750	0.70	11.750	1.05	17.750	0.50	23.75	0.36
5.833	0.70	11.833	1.05	17.833	0.50	23.83	0.36
5.917	0.70	11.917	1.05	17.917	0.50	23.92	0.36
6.000	0.70	12.000	1.05	18.000	0.50	24.00	0.36

Max.Eff.Inten.(mm/hr)= 8.37 2.27
over (min) 5.00 10.00
Storage Coeff. (min)= 4.77 (ii) 7.80 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.22 0.13

TOTALS
PEAK FLOW (cms)= 0.01 0.00 0.010 (iii)
TIME TO PEAK (hrs)= 8.92 9.00 9.00
RUNOFF VOLUME (mm)= 24.00 5.85 23.81
TOTAL RAINFALL (mm)= 25.00 25.00 25.00
RUNOFF COEFFICIENT = 0.96 0.23 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0001):	0.27	0.004	9.00	17.39
+ ID2= 2 (0017):	0.77	0.009	9.08	20.09
=====				
ID = 3 (0022):	1.04	0.013	9.00	19.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	1.04	0.013	9.00	19.39
+ ID2= 2 (0019):	0.80	0.009	9.08	18.19
=====				
ID = 1 (0022):	1.84	0.022	9.00	18.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	1.84	0.022	9.00	18.87
+ ID2= 2 (0020):	0.72	0.009	9.08	20.17
=====				
ID = 3 (0022):	2.56	0.031	9.00	19.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	2.56	0.031	9.00	19.24
+ ID2= 2 (0021):	0.44	0.010	9.00	23.81
=====				
ID = 1 (0022):	3.00	0.041	9.00	19.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	3.00	0.041	9.00	19.91
+ ID2= 2 (0036):	0.14	0.001	9.00	5.82
=====				
ID = 3 (0022):	3.14	0.042	9.00	19.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	3.14	0.042	9.00	19.28
+ ID2= 2 (0039):	0.05	0.000	9.00	5.81
=====				
ID = 1 (0022):	3.19	0.042	9.00	19.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0002)				
IN= 2---> OUT= 1				
DT= 5.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.1400	0.0413
	0.0980	0.0206	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0022)	3.190	0.042	9.00	19.07
OUTFLOW: ID= 1 (0002)	3.190	0.032	9.25	19.05

PEAK FLOW REDUCTION [Qout/Qin](%)= 75.27
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0067

ADD HYD (0035)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0002):	3.19	0.032	9.25	19.05
+ ID2= 2 (0030):	0.59	0.005	9.00	8.92
=====				
ID = 3 (0035):	3.78	0.035	9.00	17.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

=====
V V I SSSSS U U A L (v 6.2.2016)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vo.in.dat

Output filename:
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 076e-9e21-42da-a45e-7ec173794f53\scena
 Summary filename:
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 076e-9e21-42da-a45e-7ec173794f53\scena

DATE: 10-15-2024 TIME: 11:03:44

USER:

COMMENTS: _____

***** SIMULATION : 25yr_24hr Chicago *****

CHICAGO STORM IDf curve parameters: A=1368.910
 Ptotal=105.37 mm B= 8.000
 C= 0.789

used in: INTENSITY = A / (t + B)^C

Duration of storm = 24.00 hrs
 Storm time step = 10.00 min
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	0.96	6.00	3.22	12.00	2.84	18.00	1.36
0.17	0.98	6.17	3.50	12.17	2.75	18.17	1.34
0.33	1.00	6.33	3.83	12.33	2.66	18.33	1.33
0.50	1.02	6.50	4.26	12.50	2.58	18.50	1.31
0.67	1.04	6.67	4.80	12.67	2.51	18.67	1.29
0.83	1.06	6.83	5.52	12.83	2.44	18.83	1.28
1.00	1.08	7.00	6.54	13.00	2.37	19.00	1.26
1.17	1.10	7.17	8.09	13.17	2.31	19.17	1.25
1.33	1.12	7.33	10.78	13.33	2.25	19.33	1.23
1.50	1.14	7.50	16.61	13.50	2.20	19.50	1.22
1.67	1.17	7.67	39.40	13.67	2.15	19.67	1.20
1.83	1.20	7.83	139.95	13.83	2.10	19.83	1.19
2.00	1.22	8.00	51.29	14.00	2.05	20.00	1.18
2.17	1.25	8.17	27.55	14.17	2.00	20.17	1.16
2.33	1.28	8.33	18.85	14.33	1.96	20.33	1.15
2.50	1.32	8.50	14.40	14.50	1.92	20.50	1.14
2.67	1.35	8.67	11.70	14.67	1.88	20.67	1.13
2.83	1.39	8.83	9.89	14.83	1.85	20.83	1.12
3.00	1.43	9.00	8.59	15.00	1.81	21.00	1.11
3.17	1.47	9.17	7.62	15.17	1.78	21.17	1.09
3.33	1.51	9.33	6.85	15.33	1.74	21.33	1.08
3.50	1.56	9.50	6.24	15.50	1.71	21.50	1.07
3.67	1.61	9.67	5.74	15.67	1.68	21.67	1.06
3.83	1.67	9.83	5.32	15.83	1.66	21.83	1.05
4.00	1.73	10.00	4.96	16.00	1.63	22.00	1.04
4.17	1.79	10.17	4.65	16.17	1.60	22.17	1.03
4.33	1.86	10.33	4.38	16.33	1.58	22.33	1.02
4.50	1.94	10.50	4.14	16.50	1.55	22.50	1.01
4.67	2.02	10.67	3.93	16.67	1.53	22.67	1.00
4.83	2.12	10.83	3.75	16.83	1.50	22.83	0.99
5.00	2.22	11.00	3.58	17.00	1.48	23.00	0.99
5.17	2.34	11.17	3.42	17.17	1.46	23.17	0.98
5.33	2.47	11.33	3.29	17.33	1.44	23.33	0.97
5.50	2.62	11.50	3.16	17.50	1.42	23.50	0.96
5.67	2.79	11.67	3.04	17.67	1.40	23.67	0.95
5.83	2.99	11.83	2.94	17.83	1.38	23.83	0.94

CALIB
 NASHYD (0026)
 ID= 1 DT= 5.0 min

Area (ha)= 0.28 Curve Number (CN)= 84.0
 Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
 U.H. Tp(hrs)= 0.22

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11

3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99
5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99
5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98
5.417	2.47	11.417	3.29	17.417	1.44	23.42	0.97
5.500	2.47	11.500	3.29	17.500	1.44	23.50	0.97
5.583	2.62	11.583	3.16	17.583	1.42	23.58	0.96
5.667	2.62	11.667	3.16	17.667	1.42	23.67	0.96
5.750	2.79	11.750	3.04	17.750	1.40	23.75	0.95
5.833	2.79	11.833	3.04	17.833	1.40	23.83	0.95
5.917	2.99	11.917	2.94	17.917	1.38	23.92	0.94
6.000	2.99	12.000	2.94	18.000	1.38	24.00	0.94

Unit Hyd Qpeak (cms)= 0.049

PEAK FLOW (cms)= 0.039 (i)
 TIME TO PEAK (hrs)= 8.167
 RUNOFF VOLUME (mm)= 67.638
 TOTAL RAINFALL (mm)= 105.375
 RUNOFF COEFFICIENT = 0.642

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0029)
 ID= 1 DT= 5.0 min

Area (ha)= 0.18 Curve Number (CN)= 84.0
 Ia (mm)= 5.00 # of Linear Res.(N)= 3.00

----- U.H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08

3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99
5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99
5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98

Average Slope (%)= 1.00 2.00
 (m)= 23.09 40.00
 Length = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09

3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99
5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99
5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98
5.417	2.47	11.417	3.29	17.417	1.44	23.42	0.97
5.500	2.47	11.500	3.29	17.500	1.44	23.50	0.97
5.583	2.62	11.583	3.16	17.583	1.42	23.58	0.96
5.667	2.62	11.667	3.16	17.667	1.42	23.67	0.96
5.750	2.79	11.750	3.04	17.750	1.40	23.75	0.95
5.833	2.79	11.833	3.04	17.833	1.40	23.83	0.95
5.917	2.99	11.917	2.94	17.917	1.38	23.92	0.94
6.000	2.99	12.000	2.94	18.000	1.38	24.00	0.94

Max.Eff.Inten.(mm/hr)= 139.95 90.55
 over (min) = 5.00 5.00
 Storage Coeff. (min)= 0.93 (ii) 1.91 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.32

PEAK FLOW (cms)= 0.83 0.80 *TOTALS*
 TIME TO PEAK (hrs)= 8.00 8.00 0.031 (iii)
 RUNOFF VOLUME (mm)= 104.37 67.73 104.01
 TOTAL RAINFALL (mm)= 105.37 105.37 105.37
 RUNOFF COEFFICIENT = 0.99 0.64 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0037)
 ID= 1 DT= 5.0 min

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.04	0.00
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	16.33	40.00
Mannings n =	0.013	0.250

Area (ha)= 0.04
 Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16

2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000							

TOTALS

PEAK FLOW (cms)=	0.02	0.00	0.016 (iii)
TIME TO PEAK (hrs)=	8.00	8.00	8.00
RUNOFF VOLUME (mm)=	104.37	72.56	104.06
TOTAL RAINFALL (mm)=	105.37	105.37	105.37
RUNOFF COEFFICIENT =	0.99	0.69	0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		Area (ha)= 0.01		Dir. Conn.(%)= 99.00	
STANDHYD (0038)		Total Imp(%)= 99.00		ID= 1 DT= 5.0 min	
ID= 1 DT= 5.0 min					

Surface Area (ha)=	0.01	0.00
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	8.16	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25

1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99
5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99
5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98
5.417	2.47	11.417	3.29	17.417	1.44	23.42	0.97
5.500	2.47	11.500	3.29	17.500	1.44	23.50	0.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)		AREA (ha)		QPEAK (cms)		TPEAK (hrs)		R.V. (mm)	
1 + 2 = 3									
ID1= 1 (0030):		0.54		0.082		8.00		72.98	
+ ID2= 2 (0037):		0.04		0.016		8.00		104.06	
=====		0.58		0.097		8.00		75.12	
ID = 3 (0030):		0.58		0.097		8.00		75.12	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)		AREA (ha)		QPEAK (cms)		TPEAK (hrs)		R.V. (mm)	
3 + 2 = 1									
ID1= 3 (0030):		0.58		0.097		8.00		75.12	
+ ID2= 2 (0038):		0.01		0.004		8.00		82.56	
=====		0.59		0.101		8.00		75.24	
ID = 1 (0030):		0.59		0.101		8.00		75.24	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB		Area (ha)= 0.09		Curve Number (CN)= 84.0	
NASHYD (0031)		Ia (mm)= 5.00		# of Linear Res.(N)= 3.00	
ID= 1 DT= 5.0 min		U.H. Tp(hrs)= 0.17			

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28

5.583	2.62	11.583	3.16	17.583	1.42	23.58	0.96
5.667	2.62	11.667	3.16	17.667	1.42	23.67	0.96
5.750	2.79	11.750	3.04	17.750	1.40	23.75	0.95
5.833	2.79	11.833	3.04	17.833	1.40	23.83	0.95
5.917	2.99	11.917	2.94	17.917	1.38	23.92	0.94
6.000	2.99	12.000	2.94	18.000	1.38	24.00	0.94

Max.Eff.Inten.(mm/hr)=	139.95	97.49
over (min)	5.00	5.00
Storage Coeff. (min)=	0.50 (ii)	1.48 (ii)
Unit Hyd. Tpeak (min)=	5.00	5.00
Unit Hyd. peak (cms)=	0.34	0.33

TOTALS

PEAK FLOW (cms)=	0.00	0.00	0.004 (iii)
TIME TO PEAK (hrs)=	7.92	8.00	8.00
RUNOFF VOLUME (mm)=	104.37	72.56	82.56
TOTAL RAINFALL (mm)=	105.37	105.37	105.37
RUNOFF COEFFICIENT =	0.99	0.69	0.78

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0030)		AREA (ha)		QPEAK (cms)		TPEAK (hrs)		R.V. (mm)	
1 + 2 = 3									
ID1= 1 (0026):		0.28		0.039		8.17		67.64	
+ ID2= 2 (0028):		0.08		0.031		8.00		104.01	
=====		0.36		0.058		8.00		75.72	
ID = 3 (0030):		0.36		0.058		8.00		75.72	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)		AREA (ha)		QPEAK (cms)		TPEAK (hrs)		R.V. (mm)	
3 + 2 = 1									
ID1= 3 (0030):		0.36		0.058		8.00		75.72	
+ ID2= 2 (0029):		0.18		0.028		8.08		67.48	
=====		0.54		0.082		8.00		72.98	
ID = 1 (0030):		0.54		0.082		8.00		72.98	

1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99
5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99

5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98
5.417	2.47	11.417	3.29	17.417	1.44	23.42	0.97
5.500	2.47	11.500	3.29	17.500	1.44	23.50	0.97
5.583	2.62	11.583	3.16	17.583	1.42	23.58	0.96
5.667	2.62	11.667	3.16	17.667	1.42	23.67	0.96
5.750	2.79	11.750	3.04	17.750	1.40	23.75	0.95
5.833	2.79	11.833	3.04	17.833	1.40	23.83	0.95
5.917	2.99	11.917	2.94	17.917	1.38	23.92	0.94
6.000	2.99	12.000	2.94	18.000	1.38	24.00	0.94

Unit Hyd Qpeak (cms)= 0.020

PEAK FLOW (cms)= 0.014 (i)
 TIME TO PEAK (hrs)= 8.083
 RUNOFF VOLUME (mm)= 67.481
 TOTAL RAINFALL (mm)= 105.375
 RUNOFF COEFFICIENT = 0.640

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB |
 | STANDBYD (0018) | Area (ha)= 0.71
 | ID= 1 DT= 5.0 min | Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.70	0.01
Dep. Storage	(mm)=	1.00	5.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	68.80	40.00
Mannings n	=	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

		---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.083	1.36		
0.167	0.96	6.167	3.22	12.167	2.84	18.167	1.36		
0.250	0.98	6.250	3.50	12.250	2.75	18.250	1.34		
0.333	0.98	6.333	3.50	12.333	2.75	18.333	1.34		
0.417	1.00	6.417	3.83	12.417	2.66	18.417	1.33		
0.500	1.00	6.500	3.83	12.500	2.66	18.500	1.33		
0.583	1.02	6.583	4.26	12.583	2.58	18.583	1.31		
0.667	1.02	6.667	4.26	12.667	2.58	18.667	1.31		
0.750	1.04	6.750	4.80	12.750	2.51	18.750	1.29		
0.833	1.04	6.833	4.80	12.833	2.51	18.833	1.29		

0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99

5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99
5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98
5.417	2.47	11.417	3.29	17.417	1.44	23.42	0.97
5.500	2.47	11.500	3.29	17.500	1.44	23.50	0.97
5.583	2.62	11.583	3.16	17.583	1.42	23.58	0.96
5.667	2.62	11.667	3.16	17.667	1.42	23.67	0.96
5.750	2.79	11.750	3.04	17.750	1.40	23.75	0.95
5.833	2.79	11.833	3.04	17.833	1.40	23.83	0.95
5.917	2.99	11.917	2.94	17.917	1.38	23.92	0.94
6.000	2.99	12.000	2.94	18.000	1.38	24.00	0.94

Max.Eff.Inten.(mm/hr)= 139.95 90.55
 over (min) 5.00
 Storage Coeff. (min)= 1.78 (ii) 2.77 (iii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.32 0.28

PEAK FLOW (cms)= 0.27 0.00
 TIME TO PEAK (hrs)= 8.00 8.00
 RUNOFF VOLUME (mm)= 104.37 67.73
 TOTAL RAINFALL (mm)= 105.37 105.37
 RUNOFF COEFFICIENT = 0.99 0.64

TOTALS

0.274 (iii)

(cms)		(ha.m.)		(cms)		(ha.m.)	
0.0000	0.0000	0.0190	0.0279	0.0190	0.0279	0.0190	0.0279
0.0000	0.0028	0.0200	0.0307	0.0200	0.0307	0.0200	0.0307
0.0080	0.0056	0.0210	0.0335	0.0210	0.0335	0.0210	0.0335
0.0100	0.0084	0.0220	0.0363	0.0220	0.0363	0.0220	0.0363
0.0120	0.0112	0.0230	0.0391	0.0230	0.0391	0.0230	0.0391
0.0140	0.0140	0.0240	0.0419	0.0240	0.0419	0.0240	0.0419
0.0150	0.0168	0.0250	0.0447	0.0250	0.0447	0.0250	0.0447
0.0160	0.0196	0.0250	0.0475	0.0250	0.0475	0.0250	0.0475
0.0170	0.0223	0.0260	0.0499	0.0260	0.0499	0.0260	0.0499
0.0180	0.0251	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0032)	0.800	8.00	99.90
OUTFLOW: ID= 1 (0019)	0.800	8.92	96.30

PEAK FLOW REDUCTION [Qout/Qin](%)=	8.00
TIME SHIFT OF PEAK FLOW (min)=	55.00
MAXIMUM STORAGE USED (ha.m.)=	0.0389

 | CALIB |
 | NASHYD (0036) | Area (ha)= 0.14 Curve Number (CN)= 84.0
 | ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.00

 U.H. Tp(hrs)= 0.16

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25

1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99
5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99
5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98
5.417	2.47	11.417	3.29	17.417	1.44	23.42	0.97
5.500	2.47	11.500	3.29	17.500	1.44	23.50	0.97

5.583	2.62	11.583	3.16	17.583	1.42	23.58	0.96
5.667	2.62	11.667	3.16	17.667	1.42	23.67	0.96
5.750	2.79	11.750	3.04	17.750	1.40	23.75	0.95
5.833	2.79	11.833	3.04	17.833	1.40	23.83	0.95
5.917	2.99	11.917	2.94	17.917	1.38	23.92	0.94
6.000	2.99	12.000	2.94	18.000	1.38	24.00	0.94

Unit Hyd Qpeak (cms)=	0.033
PEAK FLOW (cms)=	0.023 (i)
TIME TO PEAK (hrs)=	8.083
RUNOFF VOLUME (mm)=	67.422
TOTAL RAINFALL (mm)=	105.375
RUNOFF COEFFICIENT =	0.640

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | NASHYD (0039) | Area (ha)= 0.05 Curve Number (CN)= 84.0
 | ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.00

 U.H. Tp(hrs)= 0.16

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22

1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000							

5.917 2.99 |11.917 2.94 |17.917 1.38 |23.92 0.94
6.000 2.99 |12.000 2.94 |18.000 1.38 |24.00 0.94

Unit Hyd Qpeak (cms)= 0.012

PEAK FLOW (cms)= 0.008 (i)
TIME TO PEAK (hrs)= 8.083
RUNOFF VOLUME (mm)= 67.414
TOTAL RAINFALL (mm)= 105.375
RUNOFF COEFFICIENT = 0.640

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0001) Area (ha)= 0.27
ID= 1 DT= 5.0 min Total Imp(%)= 64.00 Dir. Conn.(%)= 64.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.17 0.10
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 42.43 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23

1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01

5.750 2.79 |11.750 3.04 |17.750 1.40 |23.75 0.95
5.833 2.79 |11.833 3.04 |17.833 1.40 |23.83 0.95
5.917 2.99 |11.917 2.94 |17.917 1.38 |23.92 0.94
6.000 2.99 |12.000 2.94 |18.000 1.38 |24.00 0.94

Max.Eff.Inten.(mm/hr)= 139.95 90.55
over (min) 5.00 10.00
Storage Coeff. (min)= 1.34 (ii) 6.24 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.07 0.02 0.084 (iii)
TIME TO PEAK (hrs)= 8.00 8.08 8.00
RUNOFF VOLUME (mm)= 104.37 67.73 91.17
TOTAL RAINFALL (mm)= 105.37 105.37 105.37
RUNOFF COEFFICIENT = 0.99 0.64 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0014) Area (ha)= 0.72
ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.71 0.01
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 69.28 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33

0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01

4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99
5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99
5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98
5.417	2.47	11.417	3.29	17.417	1.44	23.42	0.97
5.500	2.47	11.500	3.29	17.500	1.44	23.50	0.97
5.583	2.62	11.583	3.16	17.583	1.42	23.58	0.96
5.667	2.62	11.667	3.16	17.667	1.42	23.67	0.96
5.750	2.79	11.750	3.04	17.750	1.40	23.75	0.95
5.833	2.79	11.833	3.04	17.833	1.40	23.83	0.95
5.917	2.99	11.917	2.94	17.917	1.38	23.92	0.94
6.000	2.99	12.000	2.94	18.000	1.38	24.00	0.94

0.0160	0.0178	0.0250	0.0433
0.0170	0.0204	0.0260	0.0455
0.0180	0.0229	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0014)	0.720	0.278	8.00
OUTFLOW : ID= 1 (0020)	0.720	0.023	8.83

PEAK FLOW REDUCTION [Qout/Qin](%) = 8.27
 TIME SHIFT OF PEAK FLOW (min) = 50.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0357

Max.Eff.Inten.(mm/hr)=	139.95	90.55
over (min)	5.00	5.00
Storage Coeff. (min)=	1.79 (ii)	2.77 (ii)
Unit Hyd. Tpeak (min)=	5.00	5.00
Unit Hyd. peak (cms)=	0.32	0.28
PEAK FLOW (cms)=	0.28	0.00
TIME TO PEAK (hrs)=	8.00	8.00
RUNOFF VOLUME (mm)=	104.37	67.73
TOTAL RAINFALL (mm)=	105.37	105.37
RUNOFF COEFFICIENT =	0.99	0.64

TOTALS
 0.278 (iii)
 8.00
 104.01
 105.37
 0.99

CALIB	Area (ha)=	0.77
STANDHYD (0016)	Total Imp(%)=	99.00
ID= 1 DT= 5.0 min	Dir. Conn.(%)=	99.00

Surface Area	(ha)=	0.76	0.01
Dep. Storage	(mm)=	1.00	5.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	71.65	40.00
Mannings n	=	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0020)
 IN= 2--> OUT= 1
 DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0255
0.0000	0.0025	0.0200	0.0280
0.0000	0.0051	0.0210	0.0306
0.0100	0.0076	0.0220	0.0331
0.0120	0.0102	0.0230	0.0357
0.0140	0.0127	0.0240	0.0382
0.0150	0.0153	0.0250	0.0407

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23

1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15
2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99
5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99
5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98
5.417	2.47	11.417	3.29	17.417	1.44	23.42	0.97
5.500	2.47	11.500	3.29	17.500	1.44	23.50	0.97
5.583	2.62	11.583	3.16	17.583	1.42	23.58	0.96
5.667	2.62	11.667	3.16	17.667	1.42	23.67	0.96

5.750	2.79	11.750	3.04	17.750	1.40	23.75	0.95
5.833	2.79	11.833	3.04	17.833	1.40	23.83	0.95
5.917	2.99	11.917	2.94	17.917	1.38	23.92	0.94
6.000	2.99	12.000	2.94	18.000	1.38	24.00	0.94

Max.Eff.Inten.(mm/hr)=	139.95	90.55
over (min)	5.00	5.00
Storage Coeff. (min)=	1.83 (ii)	2.81 (ii)
Unit Hyd. Tpeak (min)=	5.00	5.00
Unit Hyd. peak (cms)=	0.32	0.28
PEAK FLOW (cms)=	0.30	0.00
TIME TO PEAK (hrs)=	8.00	8.00
RUNOFF VOLUME (mm)=	104.37	67.73
TOTAL RAINFALL (mm)=	105.37	105.37
RUNOFF COEFFICIENT =	0.99	0.64

TOTALS
 0.297 (iii)
 8.00
 104.01
 105.37
 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0017)
 IN= 2--> OUT= 1
 DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0279
0.0000	0.0028	0.0200	0.0307
0.0000	0.0056	0.0210	0.0334
0.0100	0.0084	0.0220	0.0362
0.0120	0.0111	0.0230	0.0390
0.0140	0.0139	0.0240	0.0418
0.0150	0.0167	0.0250	0.0446
0.0160	0.0195	0.0250	0.0474
0.0170	0.0223	0.0260	0.0498
0.0180	0.0251	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0016)	0.770	0.297	8.00
OUTFLOW : ID= 1 (0017)	0.770	0.023	8.92

PEAK FLOW REDUCTION [Qout/Qin](%) = 7.74
 TIME SHIFT OF PEAK FLOW (min) = 55.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0391

CALIB
STANDHYD (0021)
ID= 1 DT= 5.0 min

Area (ha)= 0.44
Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.44 0.00
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 54.16 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.96	6.083	3.22	12.083	2.84	18.08	1.36
0.167	0.96	6.167	3.22	12.167	2.84	18.17	1.36
0.250	0.98	6.250	3.50	12.250	2.75	18.25	1.34
0.333	0.98	6.333	3.50	12.333	2.75	18.33	1.34
0.417	1.00	6.417	3.83	12.417	2.66	18.42	1.33
0.500	1.00	6.500	3.83	12.500	2.66	18.50	1.33
0.583	1.02	6.583	4.26	12.583	2.58	18.58	1.31
0.667	1.02	6.667	4.26	12.667	2.58	18.67	1.31
0.750	1.04	6.750	4.80	12.750	2.51	18.75	1.29
0.833	1.04	6.833	4.80	12.833	2.51	18.83	1.29
0.917	1.06	6.917	5.52	12.917	2.44	18.92	1.28
1.000	1.06	7.000	5.52	13.000	2.44	19.00	1.28
1.083	1.08	7.083	6.54	13.083	2.37	19.08	1.26
1.167	1.08	7.167	6.54	13.167	2.37	19.17	1.26
1.250	1.10	7.250	8.09	13.250	2.31	19.25	1.25
1.333	1.10	7.333	8.09	13.333	2.31	19.33	1.25
1.417	1.12	7.417	10.78	13.417	2.25	19.42	1.23
1.500	1.12	7.500	10.78	13.500	2.25	19.50	1.23
1.583	1.14	7.583	16.61	13.583	2.20	19.58	1.22
1.667	1.14	7.667	16.61	13.667	2.20	19.67	1.22
1.750	1.17	7.750	39.40	13.750	2.15	19.75	1.20
1.833	1.17	7.833	39.40	13.833	2.15	19.83	1.20
1.917	1.20	7.917	139.95	13.917	2.10	19.92	1.19
2.000	1.20	8.000	139.94	14.000	2.10	20.00	1.19
2.083	1.22	8.083	51.29	14.083	2.05	20.08	1.18
2.167	1.22	8.167	51.29	14.167	2.05	20.17	1.18
2.250	1.25	8.250	27.55	14.250	2.00	20.25	1.16
2.333	1.25	8.333	27.55	14.333	2.00	20.33	1.16
2.417	1.28	8.417	18.85	14.417	1.96	20.42	1.15
2.500	1.28	8.500	18.85	14.500	1.96	20.50	1.15

2.583	1.32	8.583	14.40	14.583	1.92	20.58	1.14
2.667	1.32	8.667	14.40	14.667	1.92	20.67	1.14
2.750	1.35	8.750	11.70	14.750	1.88	20.75	1.13
2.833	1.35	8.833	11.70	14.833	1.88	20.83	1.13
2.917	1.39	8.917	9.89	14.917	1.85	20.92	1.12
3.000	1.39	9.000	9.89	15.000	1.85	21.00	1.12
3.083	1.43	9.083	8.59	15.083	1.81	21.08	1.11
3.167	1.43	9.167	8.59	15.167	1.81	21.17	1.11
3.250	1.47	9.250	7.62	15.250	1.78	21.25	1.09
3.333	1.47	9.333	7.62	15.333	1.78	21.33	1.09
3.417	1.51	9.417	6.85	15.417	1.74	21.42	1.08
3.500	1.51	9.500	6.85	15.500	1.74	21.50	1.08
3.583	1.56	9.583	6.24	15.583	1.71	21.58	1.07
3.667	1.56	9.667	6.24	15.667	1.71	21.67	1.07
3.750	1.61	9.750	5.74	15.750	1.68	21.75	1.06
3.833	1.61	9.833	5.74	15.833	1.68	21.83	1.06
3.917	1.67	9.917	5.32	15.917	1.66	21.92	1.05
4.000	1.67	10.000	5.32	16.000	1.66	22.00	1.05
4.083	1.73	10.083	4.96	16.083	1.63	22.08	1.04
4.167	1.73	10.167	4.96	16.167	1.63	22.17	1.04
4.250	1.79	10.250	4.65	16.250	1.60	22.25	1.03
4.333	1.79	10.333	4.65	16.333	1.60	22.33	1.03
4.417	1.86	10.417	4.38	16.417	1.58	22.42	1.02
4.500	1.86	10.500	4.38	16.500	1.58	22.50	1.02
4.583	1.94	10.583	4.14	16.583	1.55	22.58	1.01
4.667	1.94	10.667	4.14	16.667	1.55	22.67	1.01
4.750	2.02	10.750	3.93	16.750	1.53	22.75	1.00
4.833	2.02	10.833	3.93	16.833	1.53	22.83	1.00
4.917	2.12	10.917	3.75	16.917	1.50	22.92	0.99
5.000	2.12	11.000	3.75	17.000	1.50	23.00	0.99
5.083	2.22	11.083	3.58	17.083	1.48	23.08	0.99
5.167	2.22	11.167	3.58	17.167	1.48	23.17	0.99
5.250	2.34	11.250	3.42	17.250	1.46	23.25	0.98
5.333	2.34	11.333	3.42	17.333	1.46	23.33	0.98
5.417	2.47	11.417	3.29	17.417	1.44	23.42	0.97
5.500	2.47	11.500	3.29	17.500	1.44	23.50	0.97
5.583	2.62	11.583	3.16	17.583	1.42	23.58	0.96
5.667	2.62	11.667	3.16	17.667	1.42	23.67	0.96
5.750	2.79	11.750	3.04	17.750	1.40	23.75	0.95
5.833	2.79	11.833	3.04	17.833	1.40	23.83	0.95
5.917	2.99	11.917	2.94	17.917	1.38	23.92	0.94
6.000	2.99	12.000	2.94	18.000	1.38	24.00	0.94

Max.Eff.Inten.(mm/hr)= 139.95 90.55
over (min) = 5.00 5.00
Storage Coeff. (min)= 1.55 (ii) 2.53 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.29

PEAK FLOW (cms)= 0.17 0.00

TOTALS
0.170 (iii)

TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 104.37 67.73 104.01
TOTAL RAINFALL (mm)= 105.37 105.37 105.37
RUNOFF COEFFICIENT = 0.99 0.64 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
1 + 2 = 3

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0001):	0.27	0.084	8.00 91.17
+ ID2= 2 (0017):	0.77	0.023	8.92 100.28
ID = 3 (0022):	1.04	0.102	8.00 97.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
3 + 2 = 1

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	1.04	0.102	8.00 97.92
+ ID2= 2 (0019):	0.80	0.023	8.92 96.30
ID = 1 (0022):	1.84	0.120	8.00 97.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
1 + 2 = 3

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	1.84	0.120	8.00 97.22
+ ID2= 2 (0020):	0.72	0.023	8.83 100.37
ID = 3 (0022):	2.56	0.138	8.00 98.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
3 + 2 = 1

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	2.56	0.138	8.00 98.10
+ ID2= 2 (0021):	0.44	0.170	8.00 104.01
ID = 1 (0022):	3.00	0.308	8.00 98.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
1 + 2 = 3

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	3.00	0.308	8.00 98.97
+ ID2= 2 (0036):	0.14	0.023	8.08 67.42
ID = 3 (0022):	3.14	0.328	8.00 97.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
3 + 2 = 1

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	3.14	0.328	8.00 97.56
+ ID2= 2 (0039):	0.05	0.008	8.08 67.41
ID = 1 (0022):	3.19	0.335	8.00 97.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0002)
IN= 2---> OUT= 1
DT= 5.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	0.1440	0.0413
0.0980	0.0206	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0022)	3.190	0.335	8.00 97.09
OUTFLOW : ID= 1 (0002)	3.190	0.124	8.42 97.07

PEAK FLOW REDUCTION [Qout/Qin](%)= 36.93

TIME SHIFT OF PEAK FLOW (min)= 25.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0322

ADD HYD (0035)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0002):	3.19	0.124	8.42	97.07
+ ID2= 2 (0030):	0.59	0.101	8.00	75.24
ID = 3 (0035):	3.78	0.203	8.00	93.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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V V I SSSSS U U A L (v 6.2.2016)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vo.in.dat

Output filename:
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 d5f9-9d9e-4443-86f9-b759abcff7ed\scena
 Summary filename:
 C:\Users\schhom\AppData\Local\Civica\WH5\c4784722-8666-4e89-8a43-7d10dc9357ef\e4a0
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DATE: 10-15-2024 TIME: 11:03:50

USER:

4.33	1.08	10.33	2.41	16.33	0.92	22.33	0.61
4.50	1.12	10.50	2.29	16.50	0.91	22.50	0.60
4.67	1.17	10.67	2.18	16.67	0.89	22.67	0.60
4.83	1.22	10.83	2.08	16.83	0.88	22.83	0.59
5.00	1.27	11.00	2.00	17.00	0.87	23.00	0.59
5.17	1.34	11.17	1.92	17.17	0.86	23.17	0.58
5.33	1.41	11.33	1.84	17.33	0.84	23.33	0.58
5.50	1.49	11.50	1.78	17.50	0.83	23.50	0.57
5.67	1.58	11.67	1.72	17.67	0.82	23.67	0.57
5.83	1.69	11.83	1.66	17.83	0.81	23.83	0.56

CALIB	Area (ha)=	Curve Number (CN)=
NASHYD (0026)	0.28	84.0
ID= 1 DT= 5.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 3.00
	U.H. Tp(hrs)= 0.22	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70

COMMENTS:

 ** SIMULATION : 2yr_24hr Chicago **

CHICAGO STORM
 Ptotal= 55.78 mm

IDF curve parameters: A= 586.100
 B= 6.000
 C= 0.760

used in: INTENSITY = A / (t + B)^C

Duration of storm = 24.00 hrs
 Storm time step = 10.00 min
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.00	0.58	6.00	1.81	12.00	1.61	18.00	0.80
0.17	0.59	6.17	1.95	12.17	1.56	18.17	0.79
0.33	0.60	6.33	2.13	12.33	1.51	18.33	0.78
0.50	0.61	6.50	2.35	12.50	1.47	18.50	0.77
0.67	0.62	6.67	2.63	12.67	1.43	18.67	0.76
0.83	0.63	6.83	2.99	12.83	1.39	18.83	0.75
1.00	0.64	7.00	3.50	13.00	1.36	19.00	0.74
1.17	0.65	7.17	4.27	13.17	1.32	19.17	0.74
1.33	0.66	7.33	5.56	13.33	1.29	19.33	0.73
1.50	0.68	7.50	8.29	13.50	1.26	19.50	0.72
1.67	0.69	7.67	18.82	13.67	1.23	19.67	0.71
1.83	0.71	7.83	71.26	13.83	1.21	19.83	0.70
2.00	0.72	8.00	71.26	14.00	1.18	20.00	0.70
2.17	0.74	8.17	13.32	14.17	1.16	20.17	0.69
2.33	0.76	8.33	9.33	14.33	1.13	20.33	0.68
2.50	0.78	8.50	7.27	14.50	1.11	20.50	0.68
2.67	0.79	8.67	6.00	14.67	1.09	20.67	0.67
2.83	0.82	8.83	5.14	14.83	1.07	20.83	0.66
3.00	0.84	9.00	4.51	15.00	1.05	21.00	0.66
3.17	0.86	9.17	4.04	15.17	1.03	21.17	0.65
3.33	0.89	9.33	3.66	15.33	1.01	21.33	0.64
3.50	0.91	9.50	3.36	15.50	1.00	21.50	0.64
3.67	0.94	9.67	3.10	15.67	0.98	21.67	0.63
3.83	0.97	9.83	2.89	15.83	0.96	21.83	0.63
4.00	1.00	10.00	2.71	16.00	0.95	22.00	0.62
4.17	1.04	10.17	2.55	16.17	0.93	22.17	0.61

2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Unit Hyd Qpeak (cms)= 0.049

PEAK FLOW (cms)= 0.013 (i)
 TIME TO PEAK (hrs)= 8.167
 RUNOFF VOLUME (mm)= 25.965
 TOTAL RAINFALL (mm)= 55.777
 RUNOFF COEFFICIENT = 0.466

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0029) Area (ha)= 0.18 Curve Number (CN)= 84.0
 ID= 1 DT= 5.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.00
 U.H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68

2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Unit Hyd Qpeak (cms)= 0.040

PEAK FLOW (cms)= 0.009 (i)
 TIME TO PEAK (hrs)= 8.083
 RUNOFF VOLUME (mm)= 25.906
 TOTAL RAINFALL (mm)= 55.777

RUNOFF COEFFICIENT = 0.464

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0028) Area (ha)= 0.08
 ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.08 0.00
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 23.09 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69

2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00</				

Unit Hyd. peak (cms)= 0.33 0.29
 PEAK FLOW (cms)= 0.02 0.00
 TIME TO PEAK (hrs)= 8.00 8.00
 RUNOFF VOLUME (mm)= 54.78 26.00
 TOTAL RAINFALL (mm)= 55.78 55.78
 RUNOFF COEFFICIENT = 0.98 0.47

TOTALS
 0.016 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0037)
 ID= 1 DT= 5.0 min
 Area (ha)= 0.04
 Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.04 0.00
 Dep. Storage (mm)= 1.00 1.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 16.33 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74

1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58

5.500 1.41 11.500 1.84 17.500 0.84 23.50 0.58
 5.583 1.49 11.583 1.78 17.583 0.83 23.58 0.57
 5.667 1.49 11.667 1.78 17.667 0.83 23.67 0.57
 5.750 1.58 11.750 1.72 17.750 0.82 23.75 0.57
 5.833 1.58 11.833 1.72 17.833 0.82 23.83 0.57
 5.917 1.69 11.917 1.66 17.917 0.81 23.92 0.56
 6.000 1.69 12.000 1.66 18.000 0.81 24.00 0.56

Max.Eff.Inten.(mm/hr)= 71.26 35.80
 over (min)= 5.00 5.00
 Storage Coeff. (min)= 0.99 (ii) 2.27 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.30

PEAK FLOW (cms)= 0.01 0.00
 TIME TO PEAK (hrs)= 8.00 8.00
 RUNOFF VOLUME (mm)= 54.78 29.73
 TOTAL RAINFALL (mm)= 55.78 55.78
 RUNOFF COEFFICIENT = 0.98 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0038)
 ID= 1 DT= 5.0 min
 Area (ha)= 0.01
 Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.01 0.00
 Dep. Storage (mm)= 1.00 1.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 8.16 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79

0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61

4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Max.Eff.Inten.(mm/hr)= 71.26 35.80
over (min) 5.00 5.00
Storage Coeff. (min)= 0.65 (ii) 1.94 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.31

TOTALS
PEAK FLOW (cms)= 0.00 0.00 0.002 (iii)
TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 54.78 29.73 38.16
TOTAL RAINFALL (mm)= 55.78 55.78 55.78
RUNOFF COEFFICIENT = 0.98 0.53 0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0030)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0026):	0.28	0.013	8.17	25.97
+ ID2= 2 (0028):	0.08	0.016	8.00	54.49
=====				
ID = 3 (0030):	0.36	0.024	8.00	32.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0030):	0.36	0.024	8.00	32.30
+ ID2= 2 (0029):	0.18	0.009	8.08	25.91
=====				
ID = 1 (0030):	0.54	0.032	8.00	30.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0030):	0.54	0.032	8.00	30.17
+ ID2= 2 (0037):	0.04	0.008	8.00	49.40
=====				
ID = 3 (0030):	0.58	0.040	8.00	31.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0030):	0.58	0.040	8.00	31.50
+ ID2= 2 (0038):	0.01	0.002	8.00	38.16
=====				
ID = 1 (0030):	0.59	0.041	8.00	31.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (0031)				
ID= 1 DT= 5.0 min				
Area (ha)=	0.09	Curve Number (CN)=	84.0	
Ia (mm)=	5.00	# of Linear Res.(N)=	3.00	
U.H. Tp(hrs)=	0.17			

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----
TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN

hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62

4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Unit Hyd Tpeak (cms)= 0.020

PEAK FLOW (cms)= 0.005 (i)
TIME TO PEAK (hrs)= 8.083
RUNOFF VOLUME (mm)= 25.906
TOTAL RAINFALL (mm)= 55.777
RUNOFF COEFFICIENT = 0.464

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANDHYD (0018)				
ID= 1 DT= 5.0 min				
Area (ha)=	0.71	Dir. Conn.(%)=	99.00	
Total Imp(%)=	99.00			
IMPERVIOUS PERVIOUS (i)				
Surface Area (ha)=	0.70		0.01	
Dep. Storage (mm)=	1.00		5.00	
Average Slope (%)=	1.00		2.00	
Length (m)=	68.80		40.00	
Mannings n =	0.013		0.250	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63

4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Max.Eff.Inten.(mm/hr)=	71.26	30.03	
over (min)	5.00	5.00	
Storage Coeff. (min)=	2.34 (ii)	3.62 (ii)	
Unit Hyd. Tpeak (min)=	5.00	5.00	
Unit Hyd. peak (cms)=	0.30	0.25	
PEAK FLOW (cms)=	0.14	0.00	0.138 (iii)
TIME TO PEAK (hrs)=	8.00	8.00	8.00
RUNOFF VOLUME (mm)=	54.78	26.00	54.49
TOTAL RAINFALL (mm)=	55.78	55.78	55.78
RUNOFF COEFFICIENT =	0.98	0.47	0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (0032) |

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0018):	0.71	0.138	8.00	54.49
+ ID2= 2 (0031):	0.09	0.005	8.08	25.91
ID = 3 (0032):	0.80	0.142	8.00	51.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR(0019)	OVERFLOW IS OFF
IN= 2--> OUT= 1	
DT= 5.0 min	
OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000
0.0000	0.0028
0.0080	0.0056
0.0100	0.0084
0.0120	0.0112
0.0140	0.0140
0.0150	0.0168
0.0160	0.0196
0.0170	0.0223
0.0180	0.0251

INFLOW : ID= 2 (0032)	AREA	QPEAK	TPEAK	R.V.
OUTFLOW: ID= 1 (0019)	(ha)	(cms)	(hrs)	(mm)
	0.800	0.142	8.00	51.27
	0.800	0.016	8.67	47.68

PEAK FLOW REDUCTION [Qout/Qin](%)= 10.97
TIME SHIFT OF PEAK FLOW (min)= 40.00
MAXIMUM STORAGE USED (ha.m.)= 0.0184

CALIB	NASHYD (0036)
ID= 1 DT= 5.0 min	
Area (ha)=	0.14
Ia (mm)=	5.00
U.H. Tp(hrs)=	0.16
Curve Number (CN)=	84.0
# of Linear Res.(N)=	3.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79

0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917					

4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Unit Hyd Qpeak (cms) = 0.033

PEAK FLOW (cms) = 0.007 (i)
 TIME TO PEAK (hrs) = 8.083
 RUNOFF VOLUME (mm) = 25.882
 TOTAL RAINFALL (mm) = 55.777
 RUNOFF COEFFICIENT = 0.464

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0039) Area (ha) = 0.05 Curve Number (CN) = 84.0
 ID= 1 DT= 5.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 3.00
 U.H. Tp(hrs) = 0.16

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77

0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60

4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Unit Hyd Qpeak (cms) = 0.012

PEAK FLOW (cms) = 0.003 (i)
 TIME TO PEAK (hrs) = 8.083
 RUNOFF VOLUME (mm) = 25.873
 TOTAL RAINFALL (mm) = 55.777
 RUNOFF COEFFICIENT = 0.464

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0001) Area (ha) = 0.27
 ID= 1 DT= 5.0 min Total Imp(%) = 64.00 Dir. Conn.(%) = 64.00

IMPERVIOUS PERVIOUS (i)

Surface Area	(ha)	0.17	0.10
Dep. Storage	(mm)	1.00	5.00
Average Slope	(%)	1.00	2.00
Length	(m)	42.43	40.00
Mannings n	=	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78

0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.4					

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Max.Eff.Inten.(mm/hr)= 71.26
 over (min) 5.00
 Storage Coeff. (min)= 1.75 (ii)
 Unit Hyd. Tpeak (min)= 5.00
 Unit Hyd. peak (cms)= 0.32

TOTALS

PEAK FLOW (cms)= 0.03
 TIME TO PEAK (hrs)= 8.00
 RUNOFF VOLUME (mm)= 54.78
 TOTAL RAINFALL (mm)= 55.78
 RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0014)
 ID= 1 DT= 5.0 min
 Area (ha)= 0.72
 Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.71 0.01
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 69.28 40.00
 Mannings n = 0.013 0.250

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64

RESERVOIR (0020)
 IN= 2---> OUT= 1
 DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0255
0.0000	0.0025	0.0200	0.0280
0.0080	0.0051	0.0210	0.0306
0.0100	0.0076	0.0220	0.0331
0.0120	0.0102	0.0230	0.0357
0.0140	0.0127	0.0240	0.0382
0.0150	0.0153	0.0250	0.0407
0.0160	0.0178	0.0250	0.0433
0.0170	0.0204	0.0260	0.0455
0.0180	0.0229	0.0000	0.0000

AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 INFLOW : ID= 2 (0014) 0.720 0.140 8.00 54.49
 OUTFLOW : ID= 1 (0020) 0.720 0.016 8.58 50.85

PEAK FLOW REDUCTION [Qout/Qin](%)= 11.27
 TIME SHIFT OF PEAK FLOW (min)= 35.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0173

CALIB
 STANDHYD (0016)
 ID= 1 DT= 5.0 min
 Area (ha)= 0.77
 Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.76 0.01
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 71.65 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78

3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Max.Eff.Inten.(mm/hr)= 71.26
 over (min) 5.00
 Storage Coeff. (min)= 2.35 (ii)
 Unit Hyd. Tpeak (min)= 5.00
 Unit Hyd. peak (cms)= 0.30

TOTALS

PEAK FLOW (cms)= 0.14
 TIME TO PEAK (hrs)= 8.00
 RUNOFF VOLUME (mm)= 54.78
 TOTAL RAINFALL (mm)= 55.78
 RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73
1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60

4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.917	2.08	16.917	0.88	22.92	0.59
5.000	1.22	11.000	2.08	17.000	0.88	23.00	0.59
5.083	1.27	11.083	2.00	17.083	0.87	23.08	0.59
5.167	1.27	11.167	2.00	17.167	0.87	23.17	0.59
5.250	1.34	11.250	1.92	17.250	0.86	23.25	0.58
5.333	1.34	11.333	1.92	17.333	0.86	23.33	0.58
5.417	1.41	11.417	1.84	17.417	0.84	23.42	0.58
5.500	1.41	11.500	1.84	17.500	0.84	23.50	0.58
5.583	1.49	11.583	1.78	17.583	0.83	23.58	0.57
5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Max.Eff.Inten.(mm/hr)= 71.26 30.03
over (min) 5.00 5.00
Storage Coeff. (min)= 2.40 (ii) 3.68 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.30 0.25

PEAK FLOW (cms)= 0.15 0.00 0.150 (iii)
TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 54.78 26.00 54.49
TOTAL RAINFALL (mm)= 55.78 55.78 55.78
RUNOFF COEFFICIENT = 0.98 0.47 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0017)				OVERFLOW IS OFF			
IN= 2 ---> OUT= 1							
DT= 5.0 min							
	OUTFLOW (cms)	STORAGE (ha.m.)		OUTFLOW (cms)	STORAGE (ha.m.)		
	0.0000	0.0000		0.0190	0.0279		
	0.0000	0.0028		0.0200	0.0307		
	0.0000	0.0056		0.0210	0.0334		
	0.0100	0.0084		0.0220	0.0362		
	0.0120	0.0111		0.0230	0.0390		
	0.0140	0.0139		0.0240	0.0418		

0.0150	0.0167	0.0250	0.0446
0.0160	0.0195	0.0250	0.0474
0.0170	0.0223	0.0260	0.0498
0.0180	0.0251	0.0000	0.0000

AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

INFLOW : ID= 2 (0016) 0.770 0.150 8.00 54.49
OUTFLOW: ID= 1 (0017) 0.770 0.016 8.58 50.76

PEAK FLOW REDUCTION [Qout/Qin](%)= 10.56
TIME SHIFT OF PEAK FLOW (min)= 35.00
MAXIMUM STORAGE USED (ha.m.)= 0.0190

CALIB STANDHYD (0021)	Area (ha)= 0.44
ID= 1 DT= 5.0 min	Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00
Surface Area (ha)= 0.44	IMPERVIOUS (i) 0.00
Dep. Storage (mm)= 1.00	PERVIOUS (i) 5.00
Average Slope (%)= 1.00	0.250
Length (m)= 54.16	
Mannings n = 0.013	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.58	6.083	1.81	12.083	1.61	18.08	0.80
0.167	0.58	6.167	1.81	12.167	1.61	18.17	0.80
0.250	0.59	6.250	1.95	12.250	1.56	18.25	0.79
0.333	0.59	6.333	1.95	12.333	1.56	18.33	0.79
0.417	0.60	6.417	2.13	12.417	1.51	18.42	0.78
0.500	0.60	6.500	2.13	12.500	1.51	18.50	0.78
0.583	0.61	6.583	2.35	12.583	1.47	18.58	0.77
0.667	0.61	6.667	2.35	12.667	1.47	18.67	0.77
0.750	0.62	6.750	2.63	12.750	1.43	18.75	0.76
0.833	0.62	6.833	2.63	12.833	1.43	18.83	0.76
0.917	0.63	6.917	2.99	12.917	1.39	18.92	0.75
1.000	0.63	7.000	2.99	13.000	1.39	19.00	0.75
1.083	0.64	7.083	3.50	13.083	1.36	19.08	0.74
1.167	0.64	7.167	3.50	13.167	1.36	19.17	0.74
1.250	0.65	7.250	4.27	13.250	1.32	19.25	0.74
1.333	0.65	7.333	4.27	13.333	1.32	19.33	0.74
1.417	0.66	7.417	5.56	13.417	1.29	19.42	0.73

1.500	0.66	7.500	5.56	13.500	1.29	19.50	0.73
1.583	0.68	7.583	8.29	13.583	1.26	19.58	0.72
1.667	0.68	7.667	8.29	13.667	1.26	19.67	0.72
1.750	0.69	7.750	18.82	13.750	1.23	19.75	0.71
1.833	0.69	7.833	18.82	13.833	1.23	19.83	0.71
1.917	0.71	7.917	71.26	13.917	1.21	19.92	0.70
2.000	0.71	8.000	71.26	14.000	1.21	20.00	0.70
2.083	0.72	8.083	24.34	14.083	1.18	20.08	0.70
2.167	0.72	8.167	24.34	14.167	1.18	20.17	0.70
2.250	0.74	8.250	13.32	14.250	1.16	20.25	0.69
2.333	0.74	8.333	13.32	14.333	1.16	20.33	0.69
2.417	0.76	8.417	9.33	14.417	1.13	20.42	0.68
2.500	0.76	8.500	9.33	14.500	1.13	20.50	0.68
2.583	0.78	8.583	7.27	14.583	1.11	20.58	0.68
2.667	0.78	8.667	7.27	14.667	1.11	20.67	0.68
2.750	0.79	8.750	6.00	14.750	1.09	20.75	0.67
2.833	0.79	8.833	6.00	14.833	1.09	20.83	0.67
2.917	0.82	8.917	5.14	14.917	1.07	20.92	0.66
3.000	0.82	9.000	5.14	15.000	1.07	21.00	0.66
3.083	0.84	9.083	4.51	15.083	1.05	21.08	0.66
3.167	0.84	9.167	4.51	15.167	1.05	21.17	0.66
3.250	0.86	9.250	4.04	15.250	1.03	21.25	0.65
3.333	0.86	9.333	4.04	15.333	1.03	21.33	0.65
3.417	0.89	9.417	3.66	15.417	1.01	21.42	0.64
3.500	0.89	9.500	3.66	15.500	1.01	21.50	0.64
3.583	0.91	9.583	3.36	15.583	1.00	21.58	0.64
3.667	0.91	9.667	3.36	15.667	1.00	21.67	0.64
3.750	0.94	9.750	3.10	15.750	0.98	21.75	0.63
3.833	0.94	9.833	3.10	15.833	0.98	21.83	0.63
3.917	0.97	9.917	2.89	15.917	0.96	21.92	0.63
4.000	0.97	10.000	2.89	16.000	0.96	22.00	0.63
4.083	1.00	10.083	2.71	16.083	0.95	22.08	0.62
4.167	1.00	10.167	2.71	16.167	0.95	22.17	0.62
4.250	1.04	10.250	2.55	16.250	0.93	22.25	0.61
4.333	1.04	10.333	2.55	16.333	0.93	22.33	0.61
4.417	1.08	10.417	2.41	16.417	0.92	22.42	0.61
4.500	1.08	10.500	2.41	16.500	0.92	22.50	0.61
4.583	1.12	10.583	2.29	16.583	0.91	22.58	0.60
4.667	1.12	10.667	2.29	16.667	0.91	22.67	0.60
4.750	1.17	10.750	2.18	16.750	0.89	22.75	0.60
4.833	1.17	10.833	2.18	16.833	0.89	22.83	0.60
4.917	1.22	10.					

5.667	1.49	11.667	1.78	17.667	0.83	23.67	0.57
5.750	1.58	11.750	1.72	17.750	0.82	23.75	0.57
5.833	1.58	11.833	1.72	17.833	0.82	23.83	0.57
5.917	1.69	11.917	1.66	17.917	0.81	23.92	0.56
6.000	1.69	12.000	1.66	18.000	0.81	24.00	0.56

Max.Eff.Inten.(mm/hr)= 71.26 30.03
over (min) = 5.00 5.00
Storage Coeff. (min)= 2.02 (ii) 3.31 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.31 0.26

PEAK FLOW (cms)= 0.09 0.00 *TOTALS*
TIME TO PEAK (hrs)= 8.00 8.00 0.086 (iii)
RUNOFF VOLUME (mm)= 54.78 26.00 54.48
TOTAL RAINFALL (mm)= 55.78 55.78 55.78
RUNOFF COEFFICIENT = 0.98 0.47 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0001):	0.27	0.037	8.00	44.38
+ ID2= 2 (0017):	0.77	0.016	8.58	50.76
=====				
ID = 3 (0022):	1.04	0.050	8.00	49.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	1.04	0.050	8.00	49.11
+ ID2= 2 (0019):	0.80	0.016	8.67	47.68
=====				
ID = 1 (0022):	1.84	0.063	8.00	48.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	1.84	0.063	8.00	48.49
+ ID2= 2 (0020):	0.72	0.016	8.58	50.85
=====				
ID = 3 (0022):	2.56	0.076	8.00	49.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	2.56	0.076	8.00	49.15
+ ID2= 2 (0021):	0.44	0.086	8.00	54.48
=====				
ID = 1 (0022):	3.00	0.163	8.00	49.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0001):	3.00	0.163	8.00	49.93
+ ID2= 2 (0036):	0.14	0.007	8.08	25.88
=====				
ID = 3 (0022):	3.14	0.169	8.00	48.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	3.14	0.169	8.00	48.86
+ ID2= 2 (0039):	0.05	0.003	8.08	25.87
=====				
ID = 1 (0022):	3.19	0.171	8.00	48.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR(0002)				
IN= 2---> OUT= 1				
DT= 5.0 min				
OVERFLOW IS OFF				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.1440	0.0413
	0.0980	0.0206	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0022)	3.190	0.171	8.00	48.50
OUTFLOW: ID= 1 (0002)	3.190	0.075	8.42	48.49

PEAK FLOW REDUCTION [Qout/Qin](%)= 43.79
TIME SHIFT OF PEAK FLOW (min)= 25.00
MAXIMUM STORAGE USED (ha.m.)= 0.0158

ADD HYD (0035)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0002):	3.19	0.075	8.42	48.49
+ ID2= 2 (0030):	0.59	0.041	8.00	31.61
=====				
ID = 3 (0035):	3.78	0.102	8.17	45.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L (v 6.2.2016)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voain.dat

Output filename:
C:\Users\schhom\AppData\Local\Civica\VH5\c4784722-8666-4e89-8a43-7d10dc9357ef\3b614f6e-db96-48e1-8b7c-5a6d3e4f9d95\scena
Summary filename:
C:\Users\schhom\AppData\Local\Civica\VH5\c4784722-8666-4e89-8a43-7d10dc9357ef\3b614f6e-db96-48e1-8b7c-5a6d3e4f9d95\scena

DATE: 10-15-2024 TIME: 11:03:46

USER:

COMMENTS:

**** SIMULATION : 50yr_24hr Chicago ****

| CHICAGO STORM | IDF curve parameters: A=1622.450
| Ptotal=117.76 mm | B= 9.000
C= 0.797

used in: INTENSITY = A / (t + B)^C

Duration of storm = 24.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	1.04	6.00	3.55	12.00	3.12	18.00	1.48
0.17	1.06	6.17	3.86	12.17	3.02	18.17	1.46
0.33	1.08	6.33	4.24	12.33	2.92	18.33	1.44
0.50	1.10	6.50	4.72	12.50	2.83	18.50	1.42
0.67	1.12	6.67	5.33	12.67	2.75	18.67	1.40
0.83	1.14	6.83	6.16	12.83	2.67	18.83	1.38
1.00	1.16	7.00	7.33	13.00	2.60	19.00	1.37
1.17	1.19	7.17	9.12	13.17	2.53	19.17	1.35
1.33	1.21	7.33	12.24	13.33	2.46	19.33	1.34
1.50	1.24	7.50	19.05	13.50	2.40	19.50	1.32
1.67	1.27	7.67	45.51	13.67	2.34	19.67	1.30
1.83	1.30	7.83	155.24	13.83	2.29	19.83	1.29
2.00	1.33	8.00	59.23	14.00	2.24	20.00	1.27

2.17	1.36	8.17	31.85	14.17	2.19	20.17	1.26
2.33	1.39	8.33	21.68	14.33	2.14	20.33	1.25
2.50	1.43	8.50	16.46	14.50	2.10	20.50	1.23
2.67	1.47	8.67	13.31	14.67	2.05	20.67	1.22
2.83	1.51	8.83	11.20	14.83	2.01	20.83	1.21
3.00	1.55	9.00	9.70	15.00	1.97	21.00	1.20
3.17	1.60	9.17	8.57	15.17	1.94	21.17	1.18
3.33	1.64	9.33	7.69	15.33	1.90	21.33	1.17
3.50	1.70	9.50	6.99	15.50	1.87	21.50	1.16
3.67	1.75	9.67	6.41	15.67	1.83	21.67	1.15
3.83	1.81	9.83	5.93	15.83	1.80	21.83	1.14
4.00	1.88	10.00	5.52	16.00	1.77	22.00	1.13
4.17	1.95	10.17	5.16	16.17	1.74	22.17	1.11
4.33	2.03	10.33	4.86	16.33	1.71	22.33	1.10
4.50	2.12	10.50	4.59	16.50	1.69	22.50	1.09
4.67	2.21	10.67	4.35	16.67	1.66	22.67	1.08
4.83	2.31	10.83	4.14	16.83	1.63	22.83	1.07
5.00	2.43	11.00	3.95	17.00	1.61	23.00	1.06
5.17	2.56	11.17	3.78	17.17	1.59	23.17	1.06
5.33	2.71	11.33	3.62	17.33	1.56	23.33	1.05
5.50	2.87	11.50	3.48	17.50	1.54	23.50	1.04
5.67	3.06	11.67	3.35	17.67	1.52	23.67	1.03
5.83	3.29	11.83	3.23	17.83	1.50	23.83	1.02

CALIB
 NASHYD (0026)
 ID= 1 DT= 5.0 min

Area (ha)= 0.28 Curve Number (CN)= 84.0
 Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
 U.H. Tp(hrs)= 0.22

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38

Unit Hyd Qpeak (cms)= 0.049
 PEAK FLOW (cms)= 0.046 (i)
 TIME TO PEAK (hrs)= 8.167
 RUNOFF VOLUME (mm)= 78.803
 TOTAL RAINFALL (mm)= 117.763
 RUNOFF COEFFICIENT = 0.669

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0029)
 ID= 1 DT= 5.0 min

Area (ha)= 0.18 Curve Number (CN)= 84.0
 Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
 U.H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38
1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37
1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35

1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37
1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35
1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17
3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06

1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17
3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03						

5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Unit Hyd Qpeak (cms)= 0.040

PEAK FLOW (cms)= 0.033 (i)
 TIME TO PEAK (hrs)= 8.083
 RUNOFF VOLUME (mm)= 78.624
 TOTAL RAINFALL (mm)= 117.763
 RUNOFF COEFFICIENT = 0.668

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0028)
 ID= 1 DT= 5.0 min

Area (ha)= 0.08
 Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.08 0.00
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 23.09 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38
1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37

1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35
1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17
3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06
5.333	2.56	11.333	3.78	17.333	1.59	23.33	1.06

5.417	2.71	11.417	3.62	17.417	1.56	23.42	1.05
5.500	2.71	11.500	3.62	17.500	1.56	23.50	1.05
5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Max.Eff.Inten.(mm/hr)= 155.24 105.93
 over (min)= 5.00 5.00
 Storage Coeff. (min)= 0.89 (ii) 1.83 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.32

PEAK FLOW (cms)= 0.03 0.00 0.034 (iii)
 TIME TO PEAK (hrs)= 8.00 8.00
 RUNOFF VOLUME (mm)= 116.76 78.91 116.38
 TOTAL RAINFALL (mm)= 117.76 117.76 117.76
 RUNOFF COEFFICIENT = 0.99 0.67 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0037)
 ID= 1 DT= 5.0 min

Area (ha)= 0.04
 Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.04 0.00
 Dep. Storage (mm)= 1.00 1.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 16.33 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48

0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38
1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37
1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35
1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17
3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083			

4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06
5.333	2.56	11.333	3.78	17.333	1.59	23.33	1.06
5.417	2.71	11.417	3.62	17.417	1.56	23.42	1.05
5.500	2.71	11.500	3.62	17.500	1.56	23.50	1.05
5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Max.Eff.Inten.(mm/hr)= 155.24 112.85
over (min) = 5.00 5.00
Storage Coeff. (min)= 0.72 (ii) 1.66 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.32

PEAK FLOW (cms)= 0.02 0.00
TIME TO PEAK (hrs)= 8.00 8.00
RUNOFF VOLUME (mm)= 116.76 83.91
TOTAL RAINFALL (mm)= 117.76 117.76
RUNOFF COEFFICIENT = 0.99 0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB							
STANDHYD (0038)	Area (ha)=	0.01					
ID= 1 DT= 5.0 min	Total Imp(%)=	99.00	Dir. Conn.(%)=	99.00			
	IMPERVIOUS	PERVIOUS (i)					
Surface Area (ha)=	0.01	0.00					
Dep. Storage (mm)=	1.00	1.50					

Average Slope (%)= 1.00 2.00
Length (m)= 8.16 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38
1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37
1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35
1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18

- THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0030)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0026):	0.28	0.046	8.17	78.80
+ ID2= 2 (0028):	0.08	0.034	8.00	116.38
=====				
ID = 3 (0030):	0.36	0.067	8.00	87.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0030):	0.36	0.067	8.00	87.15
+ ID2= 2 (0029):	0.18	0.033	8.00	78.62
=====				
ID = 1 (0030):	0.54	0.095	8.00	84.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0030):	0.54	0.095	8.00	84.31
+ ID2= 2 (0037):	0.04	0.017	8.00	116.43
=====				
ID = 3 (0030):	0.58	0.112	8.00	86.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0030):	0.58	0.112	8.00	86.53
+ ID2= 2 (0038):	0.01	0.004	8.00	94.89
=====				
ID = 1 (0030):	0.59	0.116	8.00	86.67

3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17
3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06
5.333	2.56	11.333	3.78	17.333	1.59	23.33	1.06
5.417	2.71	11.417	3.62	17.417	1.56	23.42	1.05
5.500	2.71	11.500	3.62	17.500	1.56	23.50	1.05
5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Max.Eff.Inten.(mm/hr)= 155.24 112.85
over (min) = 5.00 5.00
Storage Coeff. (min)= 0.48 (ii) 1.42 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.33

PEAK FLOW (cms)= 0.00 0.00
TIME TO PEAK (hrs)= 7.92 8.00
RUNOFF VOLUME (mm)= 116.76 83.91
TOTAL RAINFALL (mm)= 117.76 117.76
RUNOFF COEFFICIENT = 0.99 0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0031) Area (ha)= 0.09 Curve Number (CN)= 84.0
 ID= 1 DT= 5.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
 U.H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38
1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37
1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35
1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21

3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17
3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06
5.333	2.56	11.333	3.78	17.333	1.59	23.33	1.06
5.417	2.71	11.417	3.62	17.417	1.56	23.42	1.05
5.500	2.71	11.500	3.62	17.500	1.56	23.50	1.05
5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Unit Hyd Qpeak (cms)= 0.020

PEAK FLOW (cms)= 0.017 (i)
 TIME TO PEAK (hrs)= 8.083
 RUNOFF VOLUME (mm)= 78.622
 TOTAL RAINFALL (mm)= 117.763
 RUNOFF COEFFICIENT = 0.668

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

STANDHYD (0018) Area (ha)= 0.71
 ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS Pervious (i)
 Surface Area (ha)= 0.70 0.01
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 68.80 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38
1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37
1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35
1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22

2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17
3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06

3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06
5.333	2.56	11.333	3.78	17.333	1.59	23.33	1.06
5.417	2.71	11.417	3.62	17.417	1.56	23.42	1.05
5.500	2.71	11.500	3.62	17.500	1.56	23.50	1.05
5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Unit Hyd Qpeak (cms) = 0.012

PEAK FLOW (cms) = 0.010 (i)
 TIME TO PEAK (hrs) = 8.083
 RUNOFF VOLUME (mm) = 78.541
 TOTAL RAINFALL (mm) = 117.763
 RUNOFF COEFFICIENT = 0.667

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		STANDHYD (0001)		Area (ha) = 0.27		Total Imp(%) = 64.00		Dir. Conn.(%) = 64.00	
ID= 1 DT= 5.0 min		IMPERVIOUS		PERVIOUS (i)					
Surface Area (ha) = 0.17		0.17		0.10					
Dep. Storage (mm) = 1.00		1.00		5.00					
Average Slope (%) = 1.00		1.00		2.00					
Length (m) = 42.43		42.43		40.00					

Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48				
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48				
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46				
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46				
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44				
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44				
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42				
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42				
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40				
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40				
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38				
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38				
1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37				
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37				
1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35				
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35				
1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34				
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34				
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32				
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32				
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30				
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30				
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29				
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29				
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27				
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27				
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26				
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26				
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25				
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25				
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23				
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23				
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22				
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22				
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21				
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21				
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20				
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20				
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18				
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18				
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17				
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17				

3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06
5.333	2.56	11.333	3.78	17.333	1.59	23.33	1.06
5.417	2.71	11.417	3.62	17.417	1.56	23.42	1.05
5.500	2.71	11.500	3.62	17.500	1.56	23.50	1.05
5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Max.Eff.Inten.(mm/hr) = 155.24 over (min) = 5.00
 Storage Coeff. (min) = 1.28 (ii) 5.98 (iii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.33

PEAK FLOW (cms) = 0.07 0.02
 TIME TO PEAK (hrs) = 8.00 8.08
 RUNOFF VOLUME (mm) = 116.76 78.91
 TOTAL RAINFALL (mm) = 117.76 117.76
 RUNOFF COEFFICIENT = 0.99 0.67

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		STANDHYD (0014)		Area (ha) = 0.72		Total Imp(%) = 99.00		Dir. Conn.(%) = 99.00	
ID= 1 DT= 5.0 min		IMPERVIOUS		PERVIOUS (i)					
Surface Area (ha) = 0.71		0.71		0.01					
Dep. Storage (mm) = 1.00		1.00		5.00					
Average Slope (%) = 1.00		1.00		2.00					
Length (m) = 69.28		69.28		40.00					
Mannings n = 0.013		0.013		0.250					

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48				
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48				
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46				
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46				
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44				
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44				
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42				
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42				
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40				
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40				
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38				
1.000	1.14										

2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17
3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06
5.333	2.56	11.333	3.78	17.333	1.59	23.33	1.06
5.417	2.71	11.417	3.62	17.417	1.56	23.42	1.05
5.500	2.71	11.500	3.62	17.500	1.56	23.50	1.05
5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Max.Eff.Inten.(mm/hr)= 155.24
over (min) 5.00
Storage Coeff. (min)= 1.72 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.32

PEAK FLOW (cms)= 0.31

TOTALS
0.309 (iii)

TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 116.76 78.91 116.38
TOTAL RAINFALL (mm)= 117.76 117.76 117.76
RUNOFF COEFFICIENT = 0.99 0.67 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OVERFLOW IS OFF				
RESERVOIR(0020)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2----> OUT= 1	0.0000	0.0000	0.0190	0.0255
DT= 5.0 min	0.0000	0.0025	0.0210	0.0280
	0.0080	0.0051	0.0210	0.0306
	0.0100	0.0076	0.0220	0.0331
	0.0120	0.0102	0.0230	0.0357
	0.0140	0.0127	0.0240	0.0382
	0.0150	0.0153	0.0250	0.0407
	0.0160	0.0178	0.0250	0.0433
	0.0170	0.0204	0.0260	0.0455
	0.0180	0.0229	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW : ID= 2 (0014)	0.720	0.309	8.00	116.38
OUTFLOW: ID= 1 (0020)	0.720	0.825	8.83	112.75

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.09
TIME SHIFT OF PEAK FLOW (min)= 50.00
MAXIMUM STORAGE USED (ha.m.)= 0.0407

CALIB	Area (ha)	IMPERVIOUS	PERVIOUS (i)
STANDHYD (0016)	0.77		
ID= 1 DT= 5.0 min	Total Imp(%)= 99.00	Dir. Conn.(%)= 99.00	
Surface Area (ha)	0.76	0.01	
Dep. Storage (mm)	1.00	5.00	
Average Slope (%)	1.00	2.00	
Length (m)	71.65	40.00	

Mannings n = 0.013 0.250
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46
0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38
1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37
1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35
1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17

3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10
4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06
5.333	2.56	11.333	3.78	17.333	1.59	23.33	1.06
5.417	2.71	11.417	3.62	17.417	1.56	23.42	1.05
5.500	2.71	11.500	3.62	17.500	1.56	23.50	1.05
5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Max.Eff.Inten.(mm/hr)= 155.24
over (min) 5.00
Storage Coeff. (min)= 1.75 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.32

PEAK FLOW (cms)= 0.33
TIME TO PEAK (hrs)= 8.00
RUNOFF VOLUME (mm)= 116.76
TOTAL RAINFALL (mm)= 117.76
RUNOFF COEFFICIENT = 0.99

TOTALS
0.330 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0017)
IN= 2--> OUT= 1
DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0279
0.0000	0.0028	0.0200	0.0307
0.0000	0.0056	0.0210	0.0334
0.0100	0.0084	0.0220	0.0362
0.0120	0.0111	0.0230	0.0390
0.0140	0.0139	0.0240	0.0418
0.0150	0.0167	0.0250	0.0446
0.0160	0.0195	0.0250	0.0474
0.0170	0.0223	0.0260	0.0498
0.0180	0.0251	0.0000	0.0000

INFLOW : ID= 2 (0016)
OUTFLOW: ID= 1 (0017)

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
0.770	0.330	8.00	116.38
0.770	0.025	8.92	112.66

PEAK FLOW REDUCTION [Qout/Qin](%)= 7.56
TIME SHIFT OF PEAK FLOW (min)= 55.00
MAXIMUM STORAGE USED (ha.m.)= 0.0446

CALIB
STANDHYD (0021)
ID= 1 DT= 5.0 min

Area (ha)= 0.44
Total Imp(%)= 99.00
Dir. Conn.(%)= 99.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	0.44	0.00
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	54.16	40.00
Mannings n	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	1.04	6.083	3.55	12.083	3.12	18.08	1.48
0.167	1.04	6.167	3.55	12.167	3.12	18.17	1.48
0.250	1.06	6.250	3.86	12.250	3.02	18.25	1.46
0.333	1.06	6.333	3.86	12.333	3.02	18.33	1.46

0.417	1.08	6.417	4.24	12.417	2.92	18.42	1.44
0.500	1.08	6.500	4.24	12.500	2.92	18.50	1.44
0.583	1.10	6.583	4.72	12.583	2.83	18.58	1.42
0.667	1.10	6.667	4.72	12.667	2.83	18.67	1.42
0.750	1.12	6.750	5.33	12.750	2.75	18.75	1.40
0.833	1.12	6.833	5.33	12.833	2.75	18.83	1.40
0.917	1.14	6.917	6.16	12.917	2.67	18.92	1.38
1.000	1.14	7.000	6.16	13.000	2.67	19.00	1.38
1.083	1.16	7.083	7.33	13.083	2.60	19.08	1.37
1.167	1.16	7.167	7.33	13.167	2.60	19.17	1.37
1.250	1.19	7.250	9.12	13.250	2.53	19.25	1.35
1.333	1.19	7.333	9.12	13.333	2.53	19.33	1.35
1.417	1.21	7.417	12.24	13.417	2.46	19.42	1.34
1.500	1.21	7.500	12.24	13.500	2.46	19.50	1.34
1.583	1.24	7.583	19.05	13.583	2.40	19.58	1.32
1.667	1.24	7.667	19.05	13.667	2.40	19.67	1.32
1.750	1.27	7.750	45.51	13.750	2.34	19.75	1.30
1.833	1.27	7.833	45.52	13.833	2.34	19.83	1.30
1.917	1.30	7.917	155.24	13.917	2.29	19.92	1.29
2.000	1.30	8.000	155.23	14.000	2.29	20.00	1.29
2.083	1.33	8.083	59.23	14.083	2.24	20.08	1.27
2.167	1.33	8.167	59.23	14.167	2.24	20.17	1.27
2.250	1.36	8.250	31.85	14.250	2.19	20.25	1.26
2.333	1.36	8.333	31.85	14.333	2.19	20.33	1.26
2.417	1.39	8.417	21.68	14.417	2.14	20.42	1.25
2.500	1.39	8.500	21.68	14.500	2.14	20.50	1.25
2.583	1.43	8.583	16.46	14.583	2.10	20.58	1.23
2.667	1.43	8.667	16.46	14.667	2.10	20.67	1.23
2.750	1.47	8.750	13.31	14.750	2.05	20.75	1.22
2.833	1.47	8.833	13.31	14.833	2.05	20.83	1.22
2.917	1.51	8.917	11.20	14.917	2.01	20.92	1.21
3.000	1.51	9.000	11.20	15.000	2.01	21.00	1.21
3.083	1.55	9.083	9.70	15.083	1.97	21.08	1.20
3.167	1.55	9.167	9.70	15.167	1.97	21.17	1.20
3.250	1.60	9.250	8.57	15.250	1.94	21.25	1.18
3.333	1.60	9.333	8.57	15.333	1.94	21.33	1.18
3.417	1.64	9.417	7.69	15.417	1.90	21.42	1.17
3.500	1.64	9.500	7.69	15.500	1.90	21.50	1.17
3.583	1.70	9.583	6.99	15.583	1.87	21.58	1.16
3.667	1.70	9.667	6.99	15.667	1.87	21.67	1.16
3.750	1.75	9.750	6.41	15.750	1.83	21.75	1.15
3.833	1.75	9.833	6.41	15.833	1.83	21.83	1.15
3.917	1.81	9.917	5.93	15.917	1.80	21.92	1.14
4.000	1.81	10.000	5.93	16.000	1.80	22.00	1.14
4.083	1.88	10.083	5.52	16.083	1.77	22.08	1.13
4.167	1.88	10.167	5.52	16.167	1.77	22.17	1.13
4.250	1.95	10.250	5.16	16.250	1.74	22.25	1.11
4.333	1.95	10.333	5.16	16.333	1.74	22.33	1.11
4.417	2.03	10.417	4.86	16.417	1.71	22.42	1.10
4.500	2.03	10.500	4.86	16.500	1.71	22.50	1.10

4.583	2.12	10.583	4.59	16.583	1.69	22.58	1.09
4.667	2.12	10.667	4.59	16.667	1.69	22.67	1.09
4.750	2.21	10.750	4.35	16.750	1.66	22.75	1.08
4.833	2.21	10.833	4.35	16.833	1.66	22.83	1.08
4.917	2.31	10.917	4.14	16.917	1.63	22.92	1.07
5.000	2.31	11.000	4.14	17.000	1.63	23.00	1.07
5.083	2.43	11.083	3.95	17.083	1.61	23.08	1.06
5.167	2.43	11.167	3.95	17.167	1.61	23.17	1.06
5.250	2.56	11.250	3.78	17.250	1.59	23.25	1.06
5.333	2.56	11.333	3.78	17.333	1.59	23.33	1.06
5.417	2.71	11.417	3.62	17.417	1.56	23.42	1.05
5.500	2.71	11.500	3.62	17.500	1.56	23.50	1.05
5.583	2.87	11.583	3.48	17.583	1.54	23.58	1.04
5.667	2.87	11.667	3.48	17.667	1.54	23.67	1.04
5.750	3.06	11.750	3.35	17.750	1.52	23.75	1.03
5.833	3.06	11.833	3.35	17.833	1.52	23.83	1.03
5.917	3.29	11.917	3.23	17.917	1.50	23.92	1.02
6.000	3.29	12.000	3.23	18.000	1.50	24.00	1.02

Max.Eff.Inten.(mm/hr)= 155.24
over (min)= 5.00
Storage Coeff. (min)= 1.48 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.33

TOTALS
PEAK FLOW (cms)= 0.19
TIME TO PEAK (hrs)= 8.00
RUNOFF VOLUME (mm)= 116.76
TOTAL RAINFALL (mm)= 117.76
RUNOFF COEFFICIENT = 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0022)
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0001):	0.27	0.095	8.00
+ ID2= 2 (0017):	0.77	0.025	8.92
-----	-----	-----	-----
ID = 3 (0022):	1.04	0.114	8.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
3 + 2 = 1

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0022):	1.04	0.114	8.00
+ ID2= 2 (0019):	0.80	0.025	9.00
-----	-----	-----	-----
ID = 1 (0022):	1.84	0.133	8.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0022):	1.84	0.133	8.00
+ ID2= 2 (0020):	0.72	0.025	8.83
-----	-----	-----	-----
ID = 3 (0022):	2.56	0.152	8.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
3 + 2 = 1

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0022):	2.56	0.152	8.00
+ ID2= 2 (0021):	0.44	0.189	8.00
-----	-----	-----	-----
ID = 1 (0022):	3.00	0.341	8.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0022):	3.00	0.341	8.00
+ ID2= 2 (0036):	0.14	0.027	8.08
-----	-----	-----	-----
ID = 3 (0022):	3.14	0.364	8.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)
3 + 2 = 1

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0022):	3.14	0.364	8.00	109.81
+ ID2= 2 (0039):	0.05	0.010	8.08	78.54

ID = 1 (0022):	3.19	0.373	8.00	109.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR(0002)
IN= 2---> OUT= 1
DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1440	0.0413
0.0980	0.0206	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0022)	3.190	0.373	8.00	109.32
OUTFLOW: ID= 1 (0002)	3.190	0.134	8.42	109.31

PEAK FLOW REDUCTION [Qout/Qin](%)= 36.04
TIME SHIFT OF PEAK FLOW (min)= 25.00
MAXIMUM STORAGE USED (ha.m.)= 0.0370

ADD HYD (0035)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0002):	3.19	0.134	8.42	109.31
+ ID2= 2 (0030):	0.59	0.116	8.00	86.67

ID = 3 (0035):	3.78	0.226	8.17	105.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L (v 6.2.2016)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

0.00	0.67	6.00	2.23	12.00	1.97	18.00	0.95
0.17	0.68	6.17	2.42	12.17	1.91	18.17	0.94
0.33	0.70	6.33	2.65	12.33	1.85	18.33	0.93
0.50	0.71	6.50	2.94	12.50	1.79	18.50	0.91
0.67	0.72	6.67	3.31	12.67	1.74	18.67	0.90
0.83	0.74	6.83	3.80	12.83	1.69	18.83	0.89
1.00	0.75	7.00	4.50	13.00	1.65	19.00	0.88
1.17	0.77	7.17	5.55	13.17	1.61	19.17	0.87
1.33	0.78	7.33	7.36	13.33	1.57	19.33	0.86
1.50	0.80	7.50	11.29	13.50	1.53	19.50	0.85
1.67	0.82	7.67	26.88	13.67	1.49	19.67	0.84
1.83	0.83	7.83	101.51	13.83	1.46	19.83	0.83
2.00	0.85	8.00	35.11	14.00	1.43	20.00	0.82
2.17	0.87	8.17	18.70	14.17	1.39	20.17	0.81
2.33	0.90	8.33	12.80	14.33	1.37	20.33	0.80
2.50	0.92	8.50	9.80	14.50	1.34	20.50	0.80
2.67	0.94	8.67	7.98	14.67	1.31	20.67	0.79
2.83	0.97	8.83	6.76	14.83	1.29	20.83	0.78
3.00	0.99	9.00	5.89	15.00	1.26	21.00	0.77
3.17	1.02	9.17	5.23	15.17	1.24	21.17	0.76
3.33	1.05	9.33	4.71	15.33	1.21	21.33	0.76
3.50	1.09	9.50	4.29	15.50	1.19	21.50	0.75
3.67	1.12	9.67	3.95	15.67	1.17	21.67	0.74
3.83	1.16	9.83	3.66	15.83	1.15	21.83	0.73
4.00	1.20	10.00	3.42	16.00	1.13	22.00	0.73
4.17	1.25	10.17	3.21	16.17	1.12	22.17	0.72
4.33	1.30	10.33	3.03	16.33	1.10	22.33	0.71
4.50	1.35	10.50	2.86	16.50	1.08	22.50	0.71
4.67	1.41	10.67	2.72	16.67	1.06	22.67	0.70
4.83	1.47	10.83	2.59	16.83	1.05	22.83	0.69
5.00	1.54	11.00	2.48	17.00	1.03	23.00	0.69
5.17	1.62	11.17	2.37	17.17	1.02	23.17	0.68
5.33	1.72	11.33	2.28	17.33	1.00	23.33	0.68
5.50	1.82	11.50	2.19	17.50	0.99	23.50	0.67
5.67	1.93	11.67	2.11	17.67	0.98	23.67	0.67
5.83	2.07	11.83	2.04	17.83	0.96	23.83	0.66

CALIB
NASHYD (0026)
ID= 1 DT= 5.0 min

Area (ha)= 0.28 Curve Number (CN)= 84.0
Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.22

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----
TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\W02\Woin.dat

Output filename:
C:\Users\schhom\AppData\Local\Civica\XH5\c4784722-8666-4e89-8a43-7d10dc9357ef\cad8
9aaf-c1c1-441f-951a-9d53e10ce3fb\scena
Summary filename:
C:\Users\schhom\AppData\Local\Civica\XH5\c4784722-8666-4e89-8a43-7d10dc9357ef\cad8
9aaf-c1c1-441f-951a-9d53e10ce3fb\scena

DATE: 10-15-2024 TIME: 11:03:49

USER:

COMMENTS:

** SIMULATION : Syr_24hr Chicago **

CHICAGO STORM
Ptotal= 73.43 mm

IDF curve parameters: A= 946.460
B= 7.000
C= 0.788
used in: INTENSITY = A / (t + B)^C

Duration of storm = 24.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
-------------	---------------	-------------	---------------	-------------	---------------	-------------	---------------

0.083	0.67	6.083	2.23	12.083	1.97	18.083	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.167	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.250	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.333	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.417	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.500	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.583	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.667	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.750	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.833	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.917	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.000	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.083	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.167	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.250	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.333	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.417	0.86
1.500	0.78	7.500	7.36	13.500	1.57	19.500	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.583	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.667	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.750	0.84
1.833	0.82	7.833	26.88	13.833	1.49	19.833	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.917	0.83
2.000	0.83	8.000	101.51	14.000	1.46	20.000	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.083	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.167	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.250	0.81
2.333	0.87	8.333	18.70	14.333	1.39	20.333	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.417	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.500	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.583	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.667	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.750	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.833	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.917	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.000	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.083	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.167	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.250	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.333	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.417	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.500	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.583	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.667	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.750	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.833	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.917	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.000	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.083	0.73

4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Unit Hyd Qpeak (cms) = 0.049

PEAK FLOW (cms) = 0.022 (i)
 TIME TO PEAK (hrs) = 8.167
 RUNOFF VOLUME (mm) = 40.032
 TOTAL RAINFALL (mm) = 73.428
 RUNOFF COEFFICIENT = 0.545

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0029) Area (ha) = 0.18 Curve Number (CN) = 84.0
 ID= 1 DT= 5.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 3.00
 U.H. Tp(hrs) = 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.08	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.17	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.25	0.94

0.333	0.68	6.333	2.42	12.333	1.91	18.33	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.42	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.50	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.17	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.25	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.33	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.42	0.86
1.500	0.78	7.500	7.36	13.500	1.57	19.50	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.58	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.67	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.75	0.84
1.833	0.82	7.833	26.89	13.833	1.49	19.83	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.92	0.83
2.000	0.83	8.000	101.51	14.000	1.46	20.00	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.08	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.17	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.25	0.81
2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71

4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Unit Hyd Qpeak (cms) = 0.040

PEAK FLOW (cms) = 0.016 (i)
 TIME TO PEAK (hrs) = 8.083
 RUNOFF VOLUME (mm) = 39.941
 TOTAL RAINFALL (mm) = 73.428
 RUNOFF COEFFICIENT = 0.544

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDYD (0028) Area (ha) = 0.08
 ID= 1 DT= 5.0 min Total Imp(%) = 99.00 Dir. Conn.(%) = 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.08 0.00
 Dep. Storage (mm) = 1.00 5.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 23.09 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.08	0.95

0.167	0.67	6.167	2.23	12.167	1.97	18.17	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.25	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.33	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.42	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.50	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.17	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.25	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.33	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.42	0.86
1.500	0.78	7.500	7.36	13.500	1.57	19.50	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.58	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.67	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.75	0.84
1.833	0.82	7.833	26.89	13.833	1.49	19.83	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.92	0.83
2.000	0.83	8.000	101.51	14.000	1.46	20.00	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.08	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.17	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.25	0.81
2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99</						

4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Max.Eff.Inten.(mm/hr)= 101.51 53.13
over (min) 5.00 5.00
Storage Coeff. (min)= 1.05 (ii) 2.17 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.31

PEAK FLOW (cms)= 0.02 0.00 0.022 (iii)
TIME TO PEAK (hrs)= 8.00 8.00
RUNOFF VOLUME (mm)= 72.43 40.09 72.10
TOTAL RAINFALL (mm)= 73.43 73.43 73.43
RUNOFF COEFFICIENT = 0.99 0.55 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| STANDHYD (0037) | Area (ha)= 0.04
| ID= 1 DT= 5.0 min | Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.04 0.00

Dep. Storage (mm)= 1.00 1.50
Average Slope (%)= 1.00 2.00
Length (m)= 16.33 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.08	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.17	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.25	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.33	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.42	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.50	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.17	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.25	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.33	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.42	0.86
1.500	0.78	7.500	7.36	13.500	1.57	19.50	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.58	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.67	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.75	0.84
1.833	0.82	7.833	26.89	13.833	1.49	19.83	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.92	0.83
2.000	0.83	8.000	101.51	14.000	1.46	20.00	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.08	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.17	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.25	0.81
2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| STANDHYD (0038) | Area (ha)= 0.01
| ID= 1 DT= 5.0 min | Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.01 0.00
Dep. Storage (mm)= 1.00 1.50
Average Slope (%)= 1.00 2.00
Length (m)= 8.16 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.08	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.17	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.25	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.33	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.42	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.50	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.17	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.25	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.33	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.42	0.86
1.500	0.78	7.500	7.36	13.500	1.57	19.50	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.58	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.67	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.75	0.84
1.833	0.82	7.833	26.89	13.833	1.49	19.83	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.92	0.83
2.000	0.83	8.000	101.51	14.000	1.46	20.00	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.08	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.17	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.25	0.81

3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Max.Eff.Inten.(mm/hr)= 101.51 59.91
over (min) 5.00 5.00
Storage Coeff. (min)= 0.86 (ii) 1.97 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.31

PEAK FLOW (cms)= 0.01 0.00 0.011 (iii)
TIME TO PEAK (hrs)= 8.00 8.00
RUNOFF VOLUME (mm)= 72.43 44.31 68.21
TOTAL RAINFALL (mm)= 73.43 73.43 73.43
RUNOFF COEFFICIENT = 0.99 0.60 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)

2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Max.Eff.Inten.(mm/hr)= 101.51 59.91
over (min) = 5.00 5.00
Storage Coeff. (min)= 0.56 (ii) 1.68 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00

Unit Hyd. peak (cms)= 0.34 0.32
PEAK FLOW (cms)= 0.00 0.00 *TOTALS*
TIME TO PEAK (hrs)= 8.00 8.00 0.003 (iii)
RUNOFF VOLUME (mm)= 72.43 44.31 54.23
TOTAL RAINFALL (mm)= 73.43 73.43 73.43
RUNOFF COEFFICIENT = 0.99 0.60 0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0030)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0026):	0.28	0.022	8.17	40.03
+ ID2= 2 (0028):	0.08	0.022	8.00	72.10
=====				
ID = 3 (0030):	0.36	0.038	8.00	47.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0030):	0.36	0.038	8.00	47.16
+ ID2= 2 (0029):	0.18	0.016	8.00	39.94
=====				
ID = 1 (0030):	0.54	0.051	8.00	44.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0030):	0.54	0.051	8.00	44.75
+ ID2= 2 (0037):	0.04	0.011	8.00	68.21
=====				
ID = 3 (0030):	0.58	0.062	8.00	46.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0030):	0.58	0.062	8.00	46.37
+ ID2= 2 (0038):	0.01	0.003	8.00	54.23
=====				
ID = 1 (0030):	0.59	0.065	8.00	46.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0031) Area (ha)= 0.09 Curve Number (CN)= 84.0
ID= 1 DT= 5.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.008	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.117	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.225	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.333	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.442	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.550	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.17	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.25	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.33	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.42	0.86
1.500	0.78	7.500	7.36	13.500	1.57	19.50	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.58	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.67	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.75	0.84
1.833	0.82	7.833	26.89	13.833	1.49	19.83	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.92	0.83

2.000	0.83	8.000	101.51	14.000	1.46	20.00	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.08	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.17	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.25	0.81
2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Unit Hyd Qpeak (cms) = 0.020

PEAK FLOW (cms) = 0.008 (i)
TIME TO PEAK (hrs) = 8.083
RUNOFF VOLUME (mm) = 39.939
TOTAL RAINFALL (mm) = 73.428
RUNOFF COEFFICIENT = 0.544

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0018) Area (ha) = 0.71
ID= 1 DT= 5.0 min Total Imp(%) = 99.00 Dir. Conn.(%) = 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.70 0.01
Dep. Storage (mm) = 1.00 5.00
Average Slope (%) = 1.00 2.00
Length (m) = 68.80 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. It shows transformed hyetograph data for various time intervals.

Table with 10 columns showing hydrograph data including flow rates (cms) and cumulative volumes (mm) over time.

6.000 2.07 |12.000 2.04 |18.000 0.96 | 24.00 0.66

Max.Eff.Inten.(mm/hr)= 101.51 53.13
Storage Coeff. (min)= 2.03 (ii) 3.15 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.31 0.27
PEAK FLOW (cms)= 0.20 0.00 0.198 (iii)
TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 72.43 40.09 72.10
TOTAL RAINFALL (mm)= 73.43 73.43 73.43
RUNOFF COEFFICIENT = 0.99 0.55 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 84.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0032)
1 + 2 = 3
ID1= 1 (0018): 0.71 0.198 8.00 72.10
+ ID2= 2 (0031): 0.09 0.008 8.08 39.94
ID = 3 (0032): 0.80 0.205 8.00 68.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 4 columns: OUTFLOW (cms), STORAGE (ha.m.), OUTFLOW (cms), STORAGE (ha.m.). Shows overflow characteristics for different flow rates.

AREA QPEAK TPEAK R.V.
INFLOW : ID= 2 (0032) 0.800 0.205 8.00 68.49
OUTFLOW: ID= 1 (0019) 0.800 0.018 8.75 64.89

PEAK FLOW REDUCTION [Qout/Qin](%) = 8.99
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.0264

CALIB
NASHYD (0036) Area (ha) = 0.14 Curve Number (CN) = 84.0
Ia (mm) = 5.00 # of Linear Res. (N) = 3.00
ID= 1 DT= 5.0 min U.H. Tp(hrs) = 0.16

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows another transformed hyetograph data set.

2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Unit Hyd Qpeak (cms)= 0.033

PEAK FLOW (cms)= 0.013 (i)
 TIME TO PEAK (hrs)= 8.083

RUNOFF VOLUME (mm)= 39.902
 TOTAL RAINFALL (mm)= 73.428
 RUNOFF COEFFICIENT = 0.543

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)=	0.05	Curve Number (CN)=	84.0
NASHYD (0039)	Ia (mm)=	5.00	# of Linear Res.(N)=	3.00
ID= 1 DT= 5.0 min	U.H. Tp(hrs)=	0.16		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.08	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.17	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.25	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.33	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.42	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.50	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.17	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.25	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.33	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.42	0.86
1.500	0.78	7.500	7.36	13.500	1.57	19.50	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.58	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.67	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.75	0.84
1.833	0.82	7.833	26.89	13.833	1.49	19.83	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.92	0.83
2.000	0.83	8.000	101.51	14.000	1.46	20.00	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.08	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.17	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.25	0.81
2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80

2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Unit Hyd Qpeak (cms)= 0.012

PEAK FLOW (cms)= 0.005 (i)
 TIME TO PEAK (hrs)= 8.083
 RUNOFF VOLUME (mm)= 39.898
 TOTAL RAINFALL (mm)= 73.428
 RUNOFF COEFFICIENT = 0.543

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)=	0.27	Dir. Conn.(%)=	64.00
STANDHYD (0001)	Total Imp(%)=	64.00		
ID= 1 DT= 5.0 min				

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.17	0.10
Dep. Storage	(mm)=	1.00	5.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	42.43	40.00
Mannings n	=	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.08	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.17	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.25	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.33	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.42	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.50	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88

2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Max.Eff.Inten.(mm/hr)= 101.51 over (min)= 5.00
Storage Coeff. (min)= 1.52 (ii) 10.61 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.33

TOTALS

PEAK FLOW (cms)= 0.05 0.01 0.054 (iii)
TIME TO PEAK (hrs)= 8.00 8.17 8.00
RUNOFF VOLUME (mm)= 72.43 40.09 60.76
TOTAL RAINFALL (mm)= 73.43 73.43 73.43
RUNOFF COEFFICIENT = 0.99 0.55 0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | STANDHYD (0014) | Area (ha)= 0.72
ID= 1 DT= 5.0 min | Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.71 0.01
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 69.28 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.08	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.17	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.25	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.33	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.42	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.50	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.17	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.25	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.33	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.42	0.86

1.500	0.78	7.500	7.36	13.500	1.57	19.50	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.58	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.67	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.75	0.84
1.833	0.82	7.833	26.89	13.833	1.49	19.83	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.92	0.83
2.000	0.83	8.000	101.51	14.000	1.46	20.00	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.08	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.17	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.25	0.81
2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67

5.667 1.82 | 11.667 2.19 | 17.667 0.99 | 23.67 0.67
5.750 1.93 | 11.750 2.11 | 17.750 0.98 | 23.75 0.67
5.833 1.93 | 11.833 2.11 | 17.833 0.98 | 23.83 0.67
5.917 2.07 | 11.917 2.04 | 17.917 0.96 | 23.92 0.66
6.000 2.07 | 12.000 2.04 | 18.000 0.96 | 24.00 0.66

Max.Eff.Inten.(mm/hr)= 101.51 over (min)= 5.00
Storage Coeff. (min)= 2.04 (ii) 3.15 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.31

PEAK FLOW (cms)= 0.20 0.00 0.201 (iii)
TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 72.43 40.09 72.10
TOTAL RAINFALL (mm)= 73.43 73.43 73.43
RUNOFF COEFFICIENT = 0.99 0.55 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0020) | OVERFLOW IS OFF
IN= 2---> OUT= 1 |
DT= 5.0 min |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0255
0.0000	0.0025	0.0200	0.0280
0.0000	0.0051	0.0210	0.0306
0.0100	0.0076	0.0220	0.0331
0.0120	0.0102	0.0230	0.0357
0.0140	0.0127	0.0240	0.0382
0.0150	0.0153	0.0250	0.0407
0.0160	0.0178	0.0250	0.0433
0.0170	0.0204	0.0260	0.0455
0.0180	0.0229	0.0000	0.0000

AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
INFLOW : ID= 2 (0014) 0.720 0.201 8.00 72.10
OUTFLOW: ID= 1 (0020) 0.720 0.019 8.67 68.46

PEAK FLOW REDUCTION [Qout/Qin](%)= 9.28
TIME SHIFT OF PEAK FLOW (min)= 40.00

MAXIMUM STORAGE USED (ha.m.)= 0.0246

CALIB
STANDHYD (0016)
ID= 1 DT= 5.0 min

Area (ha)= 0.77
Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.76 0.01
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 71.65 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.08	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.17	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.25	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.33	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.42	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.50	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.17	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.25	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.33	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.42	0.86
1.500	0.78	7.500	7.36	13.500	1.57	19.50	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.58	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.67	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.75	0.84
1.833	0.82	7.833	26.89	13.833	1.49	19.83	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.92	0.83
2.000	0.83	8.000	101.51	14.000	1.46	20.00	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.08	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.17	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.25	0.81
2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80

2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76
3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Max.Eff.Inten.(mm/hr)= 101.51 53.13
over (min)= 5.00 5.00
Storage Coeff. (min)= 2.08 (ii) 3.20 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.31 0.27

TOTALS

PEAK FLOW (cms)= 0.21 0.00 0.215 (iii)
TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 72.43 40.09 72.10
TOTAL RAINFALL (mm)= 73.43 73.43 73.43
RUNOFF COEFFICIENT = 0.99 0.55 0.98

Length (m)= 54.16 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0017)
IN= 2--> OUT= 1
DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0279
0.0000	0.0028	0.0200	0.0307
0.0080	0.0056	0.0210	0.0334
0.0100	0.0084	0.0220	0.0362
0.0120	0.0111	0.0230	0.0390
0.0140	0.0139	0.0240	0.0418
0.0150	0.0167	0.0250	0.0446
0.0160	0.0195	0.0250	0.0474
0.0170	0.0223	0.0260	0.0498
0.0180	0.0251	0.0000	0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (0016) 0.770 0.215 8.00 72.10
OUTFLOW: ID= 1 (0017) 0.770 0.019 8.75 68.38

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.68
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 0.0269

CALIB
STANDHYD (0021)
ID= 1 DT= 5.0 min

Area (ha)= 0.44
Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.44 0.00
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.67	6.083	2.23	12.083	1.97	18.08	0.95
0.167	0.67	6.167	2.23	12.167	1.97	18.17	0.95
0.250	0.68	6.250	2.42	12.250	1.91	18.25	0.94
0.333	0.68	6.333	2.42	12.333	1.91	18.33	0.94
0.417	0.70	6.417	2.65	12.417	1.85	18.42	0.93
0.500	0.70	6.500	2.65	12.500	1.85	18.50	0.93
0.583	0.71	6.583	2.94	12.583	1.79	18.58	0.91
0.667	0.71	6.667	2.94	12.667	1.79	18.67	0.91
0.750	0.72	6.750	3.31	12.750	1.74	18.75	0.90
0.833	0.72	6.833	3.31	12.833	1.74	18.83	0.90
0.917	0.74	6.917	3.80	12.917	1.69	18.92	0.89
1.000	0.74	7.000	3.80	13.000	1.69	19.00	0.89
1.083	0.75	7.083	4.50	13.083	1.65	19.08	0.88
1.167	0.75	7.167	4.50	13.167	1.65	19.17	0.88
1.250	0.77	7.250	5.55	13.250	1.61	19.25	0.87
1.333	0.77	7.333	5.55	13.333	1.61	19.33	0.87
1.417	0.78	7.417	7.36	13.417	1.57	19.42	0.86
1.500	0.78	7.500	7.36	13.500	1.57	19.50	0.86
1.583	0.80	7.583	11.29	13.583	1.53	19.58	0.85
1.667	0.80	7.667	11.29	13.667	1.53	19.67	0.85
1.750	0.82	7.750	26.88	13.750	1.49	19.75	0.84
1.833	0.82	7.833	26.89	13.833	1.49	19.83	0.84
1.917	0.83	7.917	101.51	13.917	1.46	19.92	0.83
2.000	0.83	8.000	101.51	14.000	1.46	20.00	0.83
2.083	0.85	8.083	35.11	14.083	1.43	20.08	0.82
2.167	0.85	8.167	35.11	14.167	1.43	20.17	0.82
2.250	0.87	8.250	18.70	14.250	1.39	20.25	0.81
2.333	0.87	8.333	18.70	14.333	1.39	20.33	0.81
2.417	0.90	8.417	12.80	14.417	1.37	20.42	0.80
2.500	0.90	8.500	12.80	14.500	1.37	20.50	0.80
2.583	0.92	8.583	9.80	14.583	1.34	20.58	0.80
2.667	0.92	8.667	9.80	14.667	1.34	20.67	0.80
2.750	0.94	8.750	7.98	14.750	1.31	20.75	0.79
2.833	0.94	8.833	7.98	14.833	1.31	20.83	0.79
2.917	0.97	8.917	6.76	14.917	1.29	20.92	0.78
3.000	0.97	9.000	6.76	15.000	1.29	21.00	0.78
3.083	0.99	9.083	5.89	15.083	1.26	21.08	0.77
3.167	0.99	9.167	5.89	15.167	1.26	21.17	0.77
3.250	1.02	9.250	5.23	15.250	1.24	21.25	0.76
3.333	1.02	9.333	5.23	15.333	1.24	21.33	0.76
3.417	1.05	9.417	4.71	15.417	1.21	21.42	0.76

3.500	1.05	9.500	4.71	15.500	1.21	21.50	0.76
3.583	1.09	9.583	4.29	15.583	1.19	21.58	0.75
3.667	1.09	9.667	4.29	15.667	1.19	21.67	0.75
3.750	1.12	9.750	3.95	15.750	1.17	21.75	0.74
3.833	1.12	9.833	3.95	15.833	1.17	21.83	0.74
3.917	1.16	9.917	3.66	15.917	1.15	21.92	0.73
4.000	1.16	10.000	3.66	16.000	1.15	22.00	0.73
4.083	1.20	10.083	3.42	16.083	1.13	22.08	0.73
4.167	1.20	10.167	3.42	16.167	1.13	22.17	0.73
4.250	1.25	10.250	3.21	16.250	1.12	22.25	0.72
4.333	1.25	10.333	3.21	16.333	1.12	22.33	0.72
4.417	1.30	10.417	3.03	16.417	1.10	22.42	0.71
4.500	1.30	10.500	3.03	16.500	1.10	22.50	0.71
4.583	1.35	10.583	2.86	16.583	1.08	22.58	0.71
4.667	1.35	10.667	2.86	16.667	1.08	22.67	0.71
4.750	1.41	10.750	2.72	16.750	1.06	22.75	0.70
4.833	1.41	10.833	2.72	16.833	1.06	22.83	0.70
4.917	1.47	10.917	2.59	16.917	1.05	22.92	0.69
5.000	1.47	11.000	2.59	17.000	1.05	23.00	0.69
5.083	1.54	11.083	2.48	17.083	1.03	23.08	0.69
5.167	1.54	11.167	2.48	17.167	1.03	23.17	0.69
5.250	1.62	11.250	2.37	17.250	1.02	23.25	0.68
5.333	1.62	11.333	2.37	17.333	1.02	23.33	0.68
5.417	1.72	11.417	2.28	17.417	1.00	23.42	0.68
5.500	1.72	11.500	2.28	17.500	1.00	23.50	0.68
5.583	1.82	11.583	2.19	17.583	0.99	23.58	0.67
5.667	1.82	11.667	2.19	17.667	0.99	23.67	0.67
5.750	1.93	11.750	2.11	17.750	0.98	23.75	0.67
5.833	1.93	11.833	2.11	17.833	0.98	23.83	0.67
5.917	2.07	11.917	2.04	17.917	0.96	23.92	0.66
6.000	2.07	12.000	2.04	18.000	0.96	24.00	0.66

Max.Eff.Inten.(mm/hr)= 101.51 53.13
over (min) 5.00 5.00
Storage Coeff. (min)= 1.76 (ii) 2.87 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.28

TOTALS
PEAK FLOW (cms)= 0.12 0.00 0.123 (iii)
TIME TO PEAK (hrs)= 8.00 8.00 8.00
RUNOFF VOLUME (mm)= 72.43 40.09 72.10
TOTAL RAINFALL (mm)= 73.43 73.43 73.43
RUNOFF COEFFICIENT = 0.99 0.55 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0001):	0.27	0.054	8.00	60.76
+ ID2= 2 (0017):	0.77	0.019	8.75	68.38
=====				
ID = 3 (0022):	1.04	0.069	8.00	66.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	1.04	0.069	8.00	66.40
+ ID2= 2 (0019):	0.80	0.018	8.75	64.89
=====				
ID = 1 (0022):	1.84	0.084	8.00	65.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	1.84	0.084	8.00	65.75
+ ID2= 2 (0020):	0.72	0.019	8.67	68.46
=====				
ID = 3 (0022):	2.56	0.100	8.00	66.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	2.56	0.100	8.00	66.51
+ ID2= 2 (0021):	0.44	0.123	8.00	72.10
=====				
ID = 1 (0022):	3.00	0.223	8.00	67.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

=====				
ID = 3 (0035):	3.78	0.148	8.17	62.69
=====				

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 6.2.2016)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
V I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat

Output filename:
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DATE: 10-15-2024

TIME: 11:03:51

USER:

COMMENTS:

** SIMULATION : Regional Storm **

ADD HYD (0022)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	3.00	0.223	8.00	67.33
+ ID2= 2 (0036):	0.14	0.013	8.08	39.90
=====				
ID = 3 (0022):	3.14	0.234	8.00	66.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)				
3 + 2 = 1				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	3.14	0.234	8.00	66.11
+ ID2= 2 (0039):	0.05	0.005	8.08	39.90
=====				
ID = 1 (0022):	3.19	0.238	8.00	65.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR(0002)				
IN= 2--> OUT= 1				
DT= 5.0 min				
OVERFLOW IS OFF				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.1440	0.0413
	0.0900	0.0206	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0022)	3.190	0.238	8.00	65.70
OUTFLOW: ID= 1 (0002)	3.190	0.100	8.33	65.68

PEAK FLOW REDUCTION [Qout/Qin](%)= 41.88
TIME SHIFT OF PEAK FLOW (min)= 20.00
MAXIMUM STORAGE USED (ha.m.)= 0.0215

ADD HYD (0035)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0002):	3.19	0.100	8.33	65.68
+ ID2= 2 (0030):	0.59	0.065	8.00	46.50

READ STORM
Ptotal=212.00 mm

Filename: C:\Users\schhom\AppData\Local\Temp\9e61c6e4-ab79-4906-8111-09011bd8104d\4c95d2fd
Comments: HAZEL

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	6.00	3.00	13.00	6.00	23.00	9.00	53.00
1.00	4.00	4.00	17.00	7.00	13.00	10.00	38.00
2.00	6.00	5.00	13.00	8.00	13.00	11.00	13.00

CALIB
NASHYD (0026)
ID= 1 DT= 5.0 min

Area (ha)= 0.28 Curve Number (CN)= 84.0
Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.22

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00

1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Unit Hyd Qpeak (cms)= 0.040

PEAK FLOW (cms)= 0.025 (i)
TIME TO PEAK (hrs)= 10.000
RUNOFF VOLUME (mm)= 167.184
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.789

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0028)
ID= 1 DT= 5.0 min

Area (ha)= 0.08
Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.08	0.00
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	23.09	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Unit Hyd Qpeak (cms)= 0.049

PEAK FLOW (cms)= 0.038 (i)
TIME TO PEAK (hrs)= 10.000
RUNOFF VOLUME (mm)= 167.563
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.790

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0029)
ID= 1 DT= 5.0 min

Area (ha)= 0.18 Curve Number (CN)= 84.0
Ia (mm)= 5.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Max.Eff.Inten.(mm/hr)= 53.00 49.90
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.37 (ii) 2.81 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.28

PEAK FLOW (cms)= 0.01 0.00 *TOTALS*
TIME TO PEAK (hrs)= 9.33 10.00 0.012 (iii)
RUNOFF VOLUME (mm)= 211.00 167.78 210.54

TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 1.00 0.79 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0037) Area (ha)= 0.04
 ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.04 0.00
 Dep. Storage (mm)= 1.00 1.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 16.33 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Max.Eff.Inten.(mm/hr)= 53.00 50.33
 over (min)= 5.00 5.00
 Storage Coeff. (min)= 0.73 (ii) 2.18 (iii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.31

TOTALS
 PEAK FLOW (cms)= 0.00 0.00 0.001 (iii)

1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Max.Eff.Inten.(mm/hr)= 53.00 50.33
 over (min)= 5.00 5.00
 Storage Coeff. (min)= 1.11 (ii) 2.56 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.29

PEAK FLOW (cms)= 0.01 0.00 *TOTALS*
 TIME TO PEAK (hrs)= 9.33 10.00 0.006 (iii)
 RUNOFF VOLUME (mm)= 211.00 173.55 210.61
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 1.00 0.82 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0038) Area (ha)= 0.01
 ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.01 0.00
 Dep. Storage (mm)= 1.00 1.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 8.16 40.00
 Mannings n = 0.013 0.250

TIME TO PEAK (hrs)= 9.25 10.00 10.00
 RUNOFF VOLUME (mm)= 211.00 173.55 210.62
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 1.00 0.82 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 85.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0030)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0026): 0.28 0.038 10.00 167.56
 + ID2= 2 (0028): 0.08 0.012 10.00 210.54
 ID = 3 (0030): 0.36 0.050 10.00 177.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)
 3 + 2 = 1
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 3 (0030): 0.36 0.050 10.00 177.11
 + ID2= 2 (0029): 0.18 0.025 10.00 167.18
 ID = 1 (0030): 0.54 0.075 10.00 173.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0030): 0.54 0.075 10.00 173.80
 + ID2= 2 (0037): 0.04 0.006 10.00 210.61
 ID = 3 (0030): 0.58 0.081 10.00 176.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0030)
3 + 2 = 1

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 (0030):	0.58	0.081	10.00	176.34
+ ID2= 2 (0038):	0.01	0.001	10.00	210.62

ID = 1 (0030):	0.59	0.082	10.00	176.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0031)
ID= 1 DT= 5.0 min

Area (ha)=	0.09	Curve Number (CN)=	84.0
Ia (mm)=	5.00	# of Linear Res.(N)=	3.00
U.H. Tp(hrs)=	0.17		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00

0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00

Max. Eff. Inten. (mm/hr)=	53.00	49.90
over (min)	5.00	5.00
Storage Coeff. (min)=	2.63 (ii)	4.08 (ii)
Unit Hyd. Tpeak (min)=	5.00	5.00
Unit Hyd. peak (cms)=	0.29	0.24

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	0.10	0.00	211.00	212.00	1.00
	0.00	10.00	167.78	212.00	0.79
	0.104 (iii)	10.00	210.56	212.00	0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Unit Hyd Qpeak (cms)= 0.020

PEAK FLOW (cms)=	0.012 (i)
TIME TO PEAK (hrs)=	10.000
RUNOFF VOLUME (mm)=	167.180
TOTAL RAINFALL (mm)=	212.000
RUNOFF COEFFICIENT	= 0.789

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0018)
ID= 1 DT= 5.0 min

Area (ha)=	0.71
Total Imp(%)=	99.00
Dir. Conn.(%)=	99.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	0.70	0.01
Dep. Storage	1.00	5.00
Average Slope (%)	1.00	2.00
Length (m)	68.80	40.00
Mannings n	= 0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00

ADD HYD (0032)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0018):	0.71	0.104	10.00	210.56
+ ID2= 2 (0031):	0.09	0.012	10.00	167.18

ID = 3 (0032):	0.80	0.117	10.00	205.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0019)
IN= 2---> OUT= 1
DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0190	0.0279
0.0000	0.0028	0.0200	0.0307
0.0080	0.0056	0.0210	0.0335
0.0100	0.0084	0.0220	0.0363
0.0120	0.0112	0.0230	0.0391
0.0140	0.0140	0.0240	0.0419
0.0150	0.0168	0.0250	0.0447
0.0160	0.0196	0.0250	0.0475
0.0170	0.0224	0.0260	0.0499
0.0180	0.0251	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0032)	0.800	0.117	10.00	205.68
OUTFLOW: ID= 1 (0019)	0.800	0.041	11.08	202.09

PEAK FLOW REDUCTION [Qout/Qin](%)= 35.35
TIME SHIFT OF PEAK FLOW (min)= 65.00
MAXIMUM STORAGE USED (ha.m.)= 0.0869

CALIB
NASHYD (0036)
ID= 1 DT= 5.0 min

Area (ha)=	0.14	Curve Number (CN)=	84.0
Ia (mm)=	5.00	# of Linear Res.(N)=	3.00
U.H. Tp(hrs)=	0.16		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00

0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Unit Hyd Qpeak (cms)= 0.033

PEAK FLOW (cms)= 0.019 (i)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 167.026
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.788

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0039) Area (ha)= 0.05 Curve Number (CN)= 84.0
 ID= 1 DT= 5.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.00

TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.788

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0001) Area (ha)= 0.27
 ID= 1 DT= 5.0 min Total Imp(%)= 64.00 Dir. Conn.(%)= 64.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.17 0.10
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 42.43 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00

U.H. Tp(hrs)= 0.16

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Unit Hyd Qpeak (cms)= 0.012

PEAK FLOW (cms)= 0.007 (i)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 167.018

2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Max.Eff.Inten.(mm/hr)= 53.00 49.90
 over (min) 5.00 15.00
 Storage Coeff. (min)= 1.97 (ii) 11.29 (ii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.31 0.09

TOTALS
 PEAK FLOW (cms)= 0.03 0.01 0.039 (iii)
 TIME TO PEAK (hrs)= 9.50 10.00 10.00
 RUNOFF VOLUME (mm)= 211.00 167.78 195.42
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 1.00 0.79 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0014) Area (ha)= 0.72
 ID= 1 DT= 5.0 min Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.71 0.01
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 69.28 40.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr

0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Max.Eff.Inten.(mm/hr)= 53.00 49.90
over (min) = 5.00 5.00
Storage Coeff. (min)= 2.64 (ii) 4.09 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.29 0.24

PEAK FLOW (cms)= 0.10 0.00
TIME TO PEAK (hrs)= 9.67 10.00
RUNOFF VOLUME (mm)= 211.00 167.78
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 1.00 0.79

TOTALS
0.106 (iii)
10.00
210.56
212.00
0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0020)					OVERFLOW IS OFF				
IN= 2----> OUT= 1									
DT= 5.0 min									
OUTFLOW (cms)		STORAGE (ha.m.)		OUTFLOW (cms)		STORAGE (ha.m.)			
0.0000		0.0000		0.0190		0.0255			
0.0000		0.0025		0.0200		0.0280			
0.0080		0.0051		0.0210		0.0306			
0.0100		0.0076		0.0220		0.0331			
0.0120		0.0102		0.0230		0.0357			
0.0140		0.0127		0.0240		0.0382			
0.0150		0.0153		0.0250		0.0407			
0.0160		0.0178		0.0250		0.0433			
0.0170		0.0204		0.0260		0.0455			
0.0180		0.0229		0.0000		0.0000			
AREA (ha)		QPEAK (cms)		TPEAK (hrs)		R.V. (mm)			
INFLOW : ID= 2 (0014)		0.720		0.106		10.00		210.56	
OUTFLOW: ID= 1 (0020)		0.720		0.040		11.08		206.93	
PEAK FLOW REDUCTION [Qout/Qin](%)= 37.59					TIME SHIFT OF PEAK FLOW (min)= 65.00				
MAXIMUM STORAGE USED (ha.m.)= 0.0761									

CALIB				STANDHYD (0016)			
Area (ha)= 0.77				Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00			
ID= 1 DT= 5.0 min							
Surface Area (ha)= 0.76		IMPERVIOUS (mm)= 0.01		PERVIOUS (i)			
Dep. Storage (mm)= 1.00		5.00					
Average Slope (%)= 1.00		2.00					
Length (m)= 71.65		40.00					
Mannings n = 0.013		0.250					

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Max.Eff.Inten.(mm/hr)= 53.00 49.90
over (min) = 5.00 5.00
Storage Coeff. (min)= 2.70 (ii) 4.14 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.29 0.24

PEAK FLOW (cms)= 0.11 0.00
TIME TO PEAK (hrs)= 9.75 10.00
RUNOFF VOLUME (mm)= 211.00 167.78
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 1.00 0.79

TOTALS
0.113 (iii)
10.00
210.57
212.00
0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0017)					OVERFLOW IS OFF				
IN= 2----> OUT= 1									
DT= 5.0 min									
OUTFLOW (cms)		STORAGE (ha.m.)		OUTFLOW (cms)		STORAGE (ha.m.)			
0.0000		0.0000		0.0190		0.0279			
0.0000		0.0028		0.0200		0.0307			
0.0080		0.0056		0.0210		0.0334			
0.0100		0.0084		0.0220		0.0362			
0.0120		0.0111		0.0230		0.0390			
0.0140		0.0139		0.0240		0.0418			
0.0150		0.0167		0.0250		0.0446			
0.0160		0.0195		0.0250		0.0474			
0.0170		0.0223		0.0260		0.0498			
0.0180		0.0251		0.0000		0.0000			
AREA (ha)		QPEAK (cms)		TPEAK (hrs)		R.V. (mm)			
INFLOW : ID= 2 (0016)		0.770		0.113		10.00		210.57	
OUTFLOW: ID= 1 (0017)		0.770		0.041		11.08		206.84	
PEAK FLOW REDUCTION [Qout/Qin](%)= 35.77					TIME SHIFT OF PEAK FLOW (min)= 65.00				
MAXIMUM STORAGE USED (ha.m.)= 0.0850									

CALIB				STANDHYD (0021)			
Area (ha)= 0.44				Total Imp(%)= 99.00 Dir. Conn.(%)= 99.00			
ID= 1 DT= 5.0 min							
Surface Area (ha)= 0.44		IMPERVIOUS (mm)= 0.00		PERVIOUS (i)			
Dep. Storage (mm)= 1.00		5.00					
Average Slope (%)= 1.00		2.00					
Length (m)= 54.16		40.00					
Mannings n = 0.013		0.250					

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Max. Eff. Inten. (mm/hr)=	53.00	49.00	
over (min)	5.00	5.00	
Storage Coeff. (min)=	2.28 (ii)	3.73 (ii)	
Unit Hyd. Tpeak (min)=	5.00	5.00	
Unit Hyd. peak (cms)=	0.30	0.25	

PEAK FLOW (cms)=	0.06	0.00	0.065 (iii)
TIME TO PEAK (hrs)=	9.58	10.00	10.00
RUNOFF VOLUME (mm)=	211.00	167.78	210.57

TOTALS

TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 1.00 0.79 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0022)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0001):	0.27	0.039	10.00	195.42
+ ID2= 2 (0017):	0.77	0.041	11.08	206.84
=====				
ID = 3 (0022):	1.04	0.072	10.00	203.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)	AREA	QPEAK	TPEAK	R.V.
3 + 2 = 1	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	1.04	0.072	10.00	203.87
+ ID2= 2 (0019):	0.80	0.041	11.08	202.09
=====				
ID = 1 (0022):	1.84	0.109	11.00	203.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	1.84	0.109	11.00	203.10
+ ID2= 2 (0020):	0.72	0.040	11.08	206.93
=====				
ID = 3 (0022):	2.56	0.149	11.00	204.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)	AREA	QPEAK	TPEAK	R.V.
3 + 2 = 1	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	2.56	0.149	11.00	204.18
+ ID2= 2 (0021):	0.44	0.065	10.00	210.57
=====				
ID = 1 (0022):	3.00	0.204	10.00	205.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0022):	3.00	0.204	10.00	205.11
+ ID2= 2 (0036):	0.14	0.019	10.00	167.03
=====				
ID = 3 (0022):	3.14	0.223	10.00	203.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0022)	AREA	QPEAK	TPEAK	R.V.
3 + 2 = 1	(ha)	(cms)	(hrs)	(mm)
ID1= 3 (0022):	3.14	0.223	10.00	203.42
+ ID2= 2 (0039):	0.05	0.007	10.00	167.02
=====				
ID = 1 (0022):	3.19	0.230	10.00	202.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR(0002)	OVERFLOW IS OFF			
IN= 2----> OUT= 1	OUTFLOW	STORAGE	OUTFLOW	STORAGE
DT= 5.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.1440	0.0413
	0.0980	0.0206	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0022)	3.190	0.230	10.00	202.84
OUTFLOW: ID= 1 (0002)	3.190	0.187	11.08	202.83

PEAK FLOW REDUCTION [Qout/Qin](%)= 81.25
 TIME SHIFT OF PEAK FLOW (min)= 65.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0606

ADD HYD (0035)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0002):	3.19	0.187	11.08	202.83
+ ID2= 2 (0030):	0.59	0.082	10.00	176.92
=====				
ID = 3 (0035):	3.78	0.246	11.00	198.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

APPENDIX C2 - SANITARY DEMAND CALCULATIONS

WASTEWATER PROPOSED CONDITIONS - POPULATION ESTIMATE

Project Name: 130 Mountainview Road North
Municipality: Town of Halton Hills
Project No.: 23-763

Prepared by: KC
Checked by: BM
Date: 15-Nov-24

Persons Per Unit (Note 2)

Apartments - Less Than 2-Bed	1.355	persons per unit	
Apartments - 2+ Bedrooms	1.831	persons per unit	
Multiples - 3+ Bedrooms	3.044	persons per unit	(Townhomes)

Retail (Note 3)

Square Foot per Employee	403 sq. ft. / employee
	37 sq. m / employee

Site Population - Residential / Employment

By Parcels	Less Than 2-Bed	2+ Bed	Townhomes	Retail Space (m ²)	Population	
					Residential	Employment
Parcel A	347	307	16	525	1081	14
Parcel B	270	123	20	0	652	0
Parcel C	176	196	26	0	677	0
TOTAL	793	626	62	525	2,409	14

Notes:

- Proposed Unit Count per Site Statistics prepared by Arcadis Architects (November 15, 2024 Site Statistics)
- Persons Per Unit from Halton Region 2022 DC Background Study Table A-4 (Housing Occupancy Rates) - Built Boundary
- Retail population is from Halton Region 2022 DC Background Study Table A-8 (Sq. Ft per Employee Assumption 2022-2031), Commercial

WASTEWATER DEMAND CALCULATIONS - PROPOSED CONDITIONS

Project Name: 130 Mountainview Road North
Municipality: Town of Halton Hills
Project No.: 23-763

Prepared by: KC
Checked by: BM
Date: 15-Nov-24

Wastewater Generation Calculations**Average Dry Weather Flow**

Population = 2409 persons, from Site Statistics
Per Capita WW Generation Rate (Res) = 275 L/person/day (Halton Linear Design Manual)
Average Dry Weather Flow = 7.67 L/s

Employment = 14 employees, from Site Statistics
Per Capita WW Generation Rate (Emp) = 275 L/employee/day (Halton Linear Design Manual)
Average Dry Weather Flow = 0.04 L/s

Total Average Dry Weather Flow = 7.71 L/s

Peaking Factor (Modified Harmon)

Kav = 1.0 (Halton Linear Design Manual)
Peaking Factor = 3.52 (Halton Linear Design Manual)

Inflow / Infiltration

Factor = 0.286 L/ha/s (Halton Linear Design Manual)
I/I = 1.073 L/s

Design Flow = 28.22 L/s

Notes:

1. Wastewater generation per Region of Halton Water and Wastewater Linear Design Manual (2019)

APPENDIX D

Water Hydraulic Modelling Report (MES)



October 3, 2024

Project No. 17003-65

Whitestone Georgetown Developments General Partnership
Attn: Annette Gilgan, Principal, Gilbach Real Estate Development
c/o Ms. Kate Connell
Urbantech Consulting
2030 Bristol Circle, Suite 105
Oakville ON L6H 0H2

**Subject: 130 Mountainview Development
Water Distribution Modeling
Community of Georgetown, Region of Halton**

Dear Ms. Connell,

We are pleased to submit our report entitled “130 Mountainview Development Watermain Analysis” outlining the results of our water distribution analysis for the proposed residential development in the Community of Georgetown, Region of Halton.

This development layout was incorporated into the Region of Halton’s existing Infowater water models dated August 2022 and modeled utilizing the design information provided to Municipal Engineering Solutions. The findings of our analysis are summarized in the following report.

We trust you will find this report satisfactory. Should you have any questions or require further clarification, please call.

Yours truly,

Municipal Engineering Solutions

A handwritten signature in black ink that reads "John C. Bourrie".

Per: John C. Bourrie, P.Eng.

/LMC

File Location: D:\Projects\2024\24-007 Mountainview Halton Urbantech 17003-65\3.0 Report\Draft Report\17003-65 Mountainview Watermain Analysis_20240905.docx

130 MOUNTAINVIEW DEVELOPMENT

WATER ANALYSIS

PREPARED BY:

MUNICIPAL ENGINEERING SOLUTIONS



FOR:

WHITESTONE GEORGETOWN DEVELOPMENTS

October 2024

Project Number: 17003-65

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APPENDICES

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Section 1 – INTRODUCTION

Municipal Engineering Solutions (“MES”) was retained by Whitestone Georgetown Developments to conduct a hydraulic water analysis for the proposed 130 Mountainview development located in the Community of Georgetown in the Region of Halton. As part of this hydraulic assessment MES was requested to undertake the following:

1. Calculate/verify water demands for the proposed development using Region of Halton, provincial and industry design standards;
2. Add the subject watermain/development to the Region’s existing water model;
3. Run the model to size the subject mains to achieve service criteria during Average Day, Peak Hour, and fire flow during Maximum Day demand; and
4. Prepare a Report summarizing the modeling results for agency review and design purposes.

1.1 Development Background

The 130 Mountainview Development will consist of 1,482 apartments and some commercial on the west side of Mountainview Road North, south of River Drive in the Community of Georgetown. The proposed development is shown below on **Figure 1**.

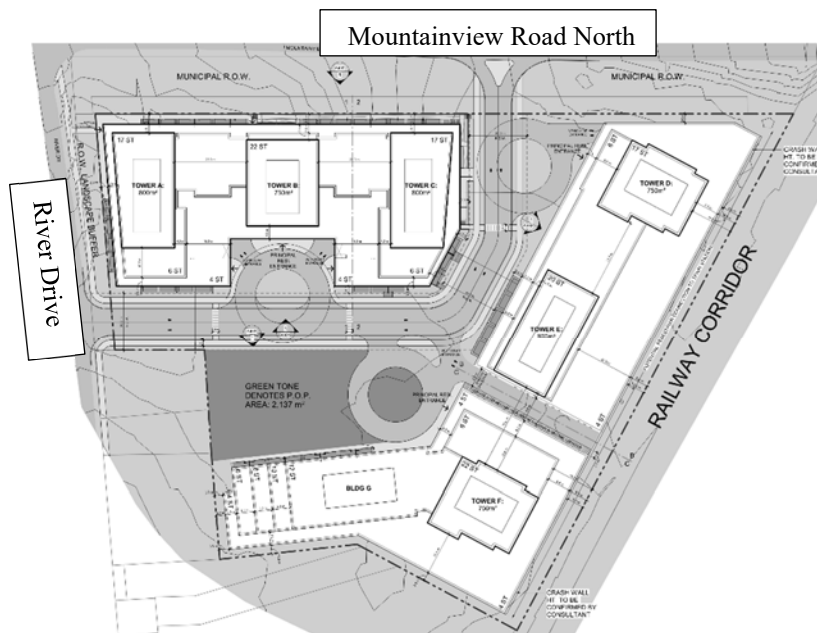


Figure 1 - Proposed 130 Mountainview Development

Section 2 – WATERMAIN DESIGN CRITERIA

The design criteria utilized to estimate the water demands for the hydraulic water model follows general industry standards and is calculated using the design criteria and guidelines outlined in the Region of Halton’s October 2019 Water and Wastewater Linear Design Manual, the Ministry of the Environment, Conservation and Parks (MECP) Watermain Design Criteria, and the Fire Underwriters Survey.

The following sections summarize the specific design criteria used to carry out the hydraulic watermain assessment for this development.

2.1 Equivalent Population Densities & Water Design Factors

To calculate the equivalent population and water design factors for this development MES used Region of Halton criteria as noted in the “*Region of Halton Water and Wastewater Linear Design Manual, October 2019*”. **Table 1** summarizes the population densities and **Table 2** summarizes the average daily demand and peaking factors used for this analysis.

Table 1 – Equivalent Population Density

Type of Development	Equivalent Population (Persons/Ha)	Equivalent Population (Persons/Unit)
Single Family	55	3.772
Semi-Detached	100	3.772
Townhouse	135	2.536
Apartments – 1 bedroom	285	1.356
Apartments – 2 or more bedrooms	285	1.831
Light Commercial	90	37.2 m ² /emp (400 sq ft/emp)
Community Services	40	55 m ² /emp (592 sq ft/emp)

Source: Region of Halton Water and Wastewater Linear Design Manual, October 2019, 2022 Development Charges Update December 2021

Table 2 - Water Design Factors

Type of Development	Average Daily Demand (m ³ per capita)	Maximum Daily Demand Peaking Factor	Peak Hourly Demand Peaking Factor
Residential	0.275	2.25	4.00
Industrial	0.275	2.25	2.25
Commercial	0.275	2.25	2.25
Community Services	0.275	2.25	2.25

Source: Region of Halton Water and Wastewater Linear Design Manual, October 2019

Section 3 –FLOW DEMANDS

Utilizing the equivalent population data and the corresponding Average Day, Maximum Day, and Peak Hour data from **Table 1** the water demands for this development were calculated.

3.1 Equivalent Population Flow Demands

The calculated demands for the development are summarized in **Table 3**. For additional details on the development water demands and assigned demand nodes used in the water model see **Appendix A**.

Table 3 – Water Demand for Mountainview Development

Development	Average Day Demand (L/S)	Maximum Day Demand (L/S)	Peak Hour Demand (L/S)
130 Mountainview	7.60	17.10	30.36

3.2 Fire Flow Demands

The fire demands for this development were based on typical flows calculated using the Fire Underwriters Survey (“FUS”) formula outlined in the “*Water Supply For Public Fire Protection Guideline*”, dated 2020. Once the building designs/configurations are known for the proposed development the fire flows for each unit/building must be confirmed

using the FUS criteria to determine the actual fire flow required. Building construction and sprinkler systems may need to be designed to suit the available flow and pressure. The fire flows used are shown in **Table 4**.

Table 4 - Fire Flow Requirements

Building	Fire Flow (L/S)
Parcel A Buildings A, B, C	167
Parcel B Buildings D & E	183
Parcel C Buildings F & G	200

Source: Fire Underwriters Survey

As noted, the fire flows in **Table 4** above are calculated using the FUS formula using estimated values. **Table 5** below summarizes the criteria utilized to develop the fire flow requirements as well as the assumptions made.

Table 5 – FUS Criteria/Assumptions

	Type of Development
	<u>Apartment Buildings</u>
Type of Construction	Fire Resistive Construction (All structural elements are Non-Combustible with a minimum 2-hour fire resistance rating)
Occupancy Type	Limited Combustible
Fire Protection (Sprinkler/Firewalls)	Sprinklered
Area Considered	Parcel A Buildings A, B & C –Total GCA 78,426 m ² Area Considered 27,631 m ² Parcel B Buildings D & E –Total GCA 48,889 m ² Area Considered 22,352.5 m ² Parcel C Buildings F & G –Total GCA 51,242 m ² Area Considered 22,833 m ²

Note: For Additional Information on FUS Criteria Refer to Water Supply for Public Protection Guide, Fire Underwriters Survey, 2020

3.3 External Demands

The Region of Halton InfoWater model that was provided by the Region to MES included water demands for existing and known future developments within the Region.

Section 4 – OTHER SYSTEM REQUIREMENTS

4.1 System Pressure Requirements

In addition to meeting the various flow requirements, the system must also satisfy minimum and maximum pressure requirements as outlined by the Region of Halton. The Region's pressure requirements are outlined in the Water and Wastewater Linear Design Manual and stipulate the following:

1. The water system shall be designed to maintain as close as possible to a maximum working pressure of 690 kPa (100 psi) as a best management practice.
2. The minimum system pressure shall not be less than 140 kPa (20 psi) at any point in the water system under fire flow conditions.
3. Under normal operating conditions, the water system shall have a target minimum static pressure of 345 kPa (50 psi). Under no operating conditions shall the static pressure within a distribution main fall below 275 kPa (40 psi).
4. The normal method of reduction of pressures to comply with the Ontario Building Code (reduction of pressures to 550 kPa, 80 psi) is by pressure reducing valves to be installed on individual services.

4.2 Watermain Sizing

The Region of Halton also stipulates minimum pipe sizes and requires that all watermains are adequately sized to maintain demand flows at the required pressures without causing excessive energy loss or result in water quality decay. The watermain system must therefore be designed to accommodate the greater of the following:

- Maximum day plus fire demand
- Peak hour demand

The minimum pipe size for commercial and industrial areas shall be 300 mm diameter and for residential areas the minimum pipe size shall be 150 mm diameter. For distribution systems providing fire protection the minimum pipe size shall be 150 mm diameter in accordance with Ministry of the Environment, Conservation and Parks (MECP) and NFPA requirements.

To provide appropriate fire protection, reliable supply and pressures the water distribution system should be looped wherever possible to improve supply security and water quality.

4.3 Watermain C-Factor

In designing and modeling of the pipes the Coefficient of Roughness (C-Factor) factors from the MECP were utilized. It's our understanding that the Region's 2019 design criteria are undergoing an update to the C-Factors to the MECP values. The Coefficient of Roughness assigned to each pipe size in summarized in **Table 6** below.

Table 6 - Hazen-Williams Coefficient of Roughness (C-Factors)

Size of Pipe (Diameter in mm)	Coefficient of Roughness (C)
150 mm	100
200 mm to 250 mm	110
300 mm to 600 mm	120
Greater than 600 mm	130

Source: MECP

Section 5 – ANALYSIS & MODELING RESULTS

To conduct the hydraulic water analysis for the proposed development the water demands were estimated by MES using the design criteria previously discussed and incorporated the demands into the existing Region of Halton InfoWater model which was provided by the Region and confirmed as most recent. The following sections discusses the model setup and results.

5.1 Model Setup

The Mountainview development is located within the Region's Zone G6G. As Georgetown builds out, the Region will be switching areas south of Credit River to a lake-based water supply. The Mountainview site will remain in the G6G zone. The development was modeled under 2026 and 2031 conditions in the Region's water model.

The site will be provided with connections to the existing Zone G6G 250 mm watermains along River Drive and Mountainview Road North.

New nodes were created to add the flow demands and service elevation information from the development to the Region of Halton's existing Infowater hydraulic water distribution model system and the system analysis was conducted. Friction factor for the pipes were assigned according to **Table 6**.

5.2 Watermain Sizing and System Pressures

The analysis was conducted under 2026 and 2031 servicing conditions for Average Day, Maximum Day, Peak Hour and Maximum day plus Fire demands to size the watermains and meet the pressure requirements. The pipe size and layout are shown in **Appendix B**.

The watermains were sized at 200 mm according to the results of average day, maximum day, maximum day plus fire, and peak hour scenarios.

The site is adequately supplied from Zone G6G according to the model output. Both 2026 and 2031 scenarios considered that the Georgetown groundwater/lake water zone split has been completed. There is a neighbouring development site, 1 Rosetta Street, that will increase the water demand in the area. The Rosetta Street site will add about 12 L/s peak hour demand (1 Rosetta Street Georgetown FSR, Table 3.1 by Arcadis, June 2023). The Mountainview site will achieve adequate pressures and flows with the estimated Rosetta demands considered.

Modeled service pressures for the development are summarized in **Table 7**. All pressures lie within the required operating range under average day, maximum day, and peak hour demands.

Detailed pipe and node tables for the various scenarios modelled are attached to this report in **Appendix B**.

Table 7 - Modeled Service Pressures

Scenario	Average Day	Maximum Day	Peak Hour	Max. Day + Fire
2026	63.2 – 76.4 psi (436 to 527 kPa)	62.1 – 75.3 psi (428 to 519 kPa)	59.7 to 73.0 psi (411 to 504 kPa)	260 to 450 L/s @ 20 psi
2031	63.0 – 76.2 psi (435 to 525 kPa)	61.8 – 75.0 psi (426 to 517 kPa)	59.1 to 72.4 psi (408 to 499 kPa)	257 to 444 L/s @ 20 psi

Section 6 – CONCLUSIONS

The results are summarized below.

- The service pressures are expected to range between 59.7 psi to 76.4 psi (411 kPa to 527 kPa) in 2026 and between 59.1 psi to 76.2 psi (408 kPa to 525 kPa) in 2031.
- The available fire flow meets the preliminary fire flow demands at the minimum pressure of 20 psi (140 kPa).
- Pressures must be confirmed in the field after the watermains are constructed.
- The available fire flow meets or exceeds the preliminary fire flow demands utilized for this assessment at the minimum pressure of 140 kPa based on the proposed watermain supply and assumptions made within this report but should be confirmed when additional information becomes available. Once building designs/configurations are known, the fire flows must be confirmed using the FUS formula. Building construction, firewall locations and sprinkler systems may need to be designed to suit the available flow and pressure.
- This report, including all modeling assumptions used, is to be submitted to and reviewed by the water operating authority (municipality) to confirm that the modeling parameters used are acceptable to the operating authority and/or confirm if modified domestic or fire flow requirements are required or should be implemented for this particular development.

Appendix A

Demands

Halton Design Criteria

Water & Wastewater Linear Design Manual, October 2019



Equivalent Population by Unit

(2022 Development Charges Update, Table A-4)

Type of Development	Equivalent Population Density
	(Person/Unit)
Semi-Detached	3.772
Townhouse	2.536
Apartment Less than 2 Bdrm	1.356
Apartment 2 or more Bdrms	1.831

Equivalent Population by Area

Type of Development	Equivalent Population Density	Average Day Demands
	(Person/Hectare)	(m3/ha/day)
Single Family	55	15.13
Semi-detached duplex and 4-plex	100	27.50
Townhouse, Maisonette (<6 stories)	135	37.13
Apartments (>6 stories)	285	78.38
Light Commercial Areas	90	24.75
Community Services	40	11.00
Light Industrial Areas	125	34.38
Hospitals (persons/bed)	4	

Water Design Factors

Average Daily Demand (m3/capita)	0.275
Maximum Daily Demand P.F.	2.25
Maximum Hourly Demand P.F.	
<i>Residential</i>	4
<i>I/C/I</i>	2.25

Coefficient of Roughness

Size of Pipe (mm Dia.)	Material	Coefficient of Roughness (C)
50	Copper	120
100-400	PVC/HDPE	130
Over 400	Concrete Lined	110

Minimum Pipe Size

Type of Development	Size of Pipe (mm Dia.)
Residential	150
Commercial/Industrial/Community	300

Working Pressures

Parameter	Pressure
Normal Condition	
Minimum Pressure	275 kPa (40 psi)
Target Pressure	350 kPa (50 psi)
Maximum (Building Code)	550 kPa (80 psi)
Maximum (Halton)	690 kPa (100 psi)
Fire Flow Conditions	
Minimum Pressure	140 kPa (20 psi)

Water Demand
Mountainview, Georgetown ON
October 3, 2024



Node	Exist. Zone	Future Zone	Elevation (m)	Type of Development					Equivalent Population		Demands			Fire Flow Demands
				Single Family (units)	Townhomes (units)	Apts < 2 bdrm (units)	Apts ≥ 2 bdrm (units)	Commercial (ha)	Total Population (Residential)	Total Population (ICI)	ADD (L/s)	MDD (L/s)	PHD (L/s)	(L/s)
J-700	G6G	G6G	256.40						0	0	0.00	0.00	0.00	
J-701	G6G	G6G	254.00						0	0	0.00	0.00	0.00	
J-702	G6G	G6G	257.40			274	140		628	0	2.00	4.50	7.99	183
J-703	G6G	G6G	256.44						0	0	0.00	0.00	0.00	
J-704	G6G	G6G	254.16		37	341	288	0.0615	1084	6	3.47	7.80	13.83	167
J-705	G6G	G6G	255.22						0	0	0.00	0.00	0.00	
J-706	G6G	G6G	255.60		11	155	236		670	0	2.13	4.80	8.53	200
J-707	G6G	G6G	251.14						0	0	0.00	0.00	0.00	
Total				0	48	770	664	0.062	2382	6	7.60	17.10	30.36	

FUS CALCULATION

Project: Mountainview	Building Type/Block # Towers A, B & C
Project Number: 17003-65	Firewalls/Sprinkler: Sprinklered
Project Location: Halton	Number of Units/Unit #'s 666 Units
Date: 27-Sep-24	

1.0 FUS Formula

$RFF = 220C\sqrt{A}$ where: RFF = required fire flow in litres per minute;
C = the Coefficient related to the type of construction; and
A = the Total Effective Floor Area (m²) excluding basements at least 50% below grade)^a

NBC Occupancy	Group C
Type of Construction ^b	Fire-Resistive Construction Type I
Protection (for C below 1.0)	Unprotected Openings
Footprint area	7143.0 sq. metres
Storeys	17-22
C =	0.6
A =	27631.0 Total Effective Area ^a
F =	22000 L/min (rounded)

2.0 Occupancy Adjustment

Type of Occupancy ^c	Limited Combustible
Hazard Allowance	-0.15
	-3300 L/min
Adjusted Fire Flow	18700 L/min

3.0 Sprinkler Adjustment

	Credit	Total
NFPA 13 sprinkler standard	YES	30%
Standard Water Supply	YES	10%
Fully Supervised system	YES	10%
		50%

Sprinkler Credit 9350 L/min

4.0 Exposure Adjustment

Construction Type of the Exposed Building Face: Type I-II (unprotected)

Side	Distance to Building (m)	Length (ft) by height in storeys	Percent	Total*
North Side				4%
	over 30	over 100	0%	
South Side				
	20.1 to 30	over 100	4%	
East Side				
	over 30	over 100	0%	
West Side				
	over 30	over 100	0%	

*max 75%

Exposures Surcharge 750 L/min

Total Required Fire Flow	10000 L/min
(rounded)	167 L/sec

a) For buildings with a construction coefficient from 1.0 to 1.5, consider 100% of all floor areas. For buildings with a construction coefficient below 1.0 (vertical openings are inadequately protected), consider the two largest adjoining floors plus 50% of each of any floors immediately above them up to a maximum of eight. If the vertical openings and exterior vertical communications are properly protected, consider only the area of the largest floor plus 25% of each of the two immediately adjoining floors.

b) Wood Frame=1.5, Mass Timber= 0.8 to 1.5, Ordinary=1.0, Noncombustible=0.8, Fire-Resistive=0.6

c) Noncombustible=-25%, Limited Combustible=-15%, Combustible=0%, Free Burning=+15%, Rapid Burning=+25%

FUS CALCULATION

Project: Mountainview	Building Type/Block #	Towers D & E
Project Number: 17003-65	Firewalls/Sprinkler:	Sprinklered
Project Location: Halton	Number of Units/Unit #'s	414 Units
Date: 27-Sep-24		

1.0 FUS Formula

$RFF = 220C\sqrt{A}$ where: RFF = required fire flow in litres per minute;
C = the Coefficient related to the type of construction; and
A = the Total Effective Floor Area (m²) excluding basements at least 50% below grade)^a

NBC Occupancy	Group C
Type of Construction ^b	Fire-Resistive Construction Type I
Protection (for C below 1.0)	Unprotected Openings
Footprint area	5657.0 sq. metres
Storeys	33
C =	0.6
A =	22352.5 Total Effective Area ^a
F =	20000 L/min (rounded)

2.0 Occupancy Adjustment

Type of Occupancy ^c	Limited Combustible
Hazard Allowance	-0.15
	-3000 L/min
Adjusted Fire Flow	17000 L/min

3.0 Sprinkler Adjustment

	Credit	Total
NFPA 13 sprinkler standard	YES	30%
Standard Water Supply	YES	10%
Fully Supervised system	YES	10%
		50%

Sprinkler Credit 8500 L/min

4.0 Exposure Adjustment

Construction Type of the Exposed Building Face: Type I-II (unprotected)

North Side	Percent	Total*
Distance to Building (m)	20.1 to 30	4%
Length (ft) by height in storeys	over 100	
South Side		
Distance to Building (m)	over 30	0%
Length (ft) by height in storeys	over 100	
East Side		
Distance to Building (m)	over 30	0%
Length (ft) by height in storeys	over 100	
West Side		
Distance to Building (m)	3.1 to 10	11%
Length (ft) by height in storeys	over 100	

*max 75%

Exposures Surcharge 2550 L/min

Total Required Fire Flow 11000 L/min
(rounded) 183 L/sec

a) For buildings with a construction coefficient from 1.0 to 1.5, consider 100% of all floor areas. For buildings with a construction coefficient below 1.0 (vertical openings are inadequately protected), consider the two largest adjoining floors plus 50% of each of any floors immediately above them up to a maximum of eight. If the vertical openings and exterior vertical communications are properly protected, consider only the area of the largest floor plus 25% of each of the two immediately adjoining floors.
b) Wood Frame=1.5, Mass Timber= 0.8 to 1.5, Ordinary=1.0, Noncombustible=0.8, Fire-Resistive=0.6
c) Noncombustible=-25%, Limited Combustible=-15%, Combustible=0%, Free Burning=+15%, Rapid Burning=+25%

FUS CALCULATION

Project: Mountainview	Building Type/Block #	Towers F & G
Project Number: 17003-65	Firewalls/Sprinkler:	Sprinklered
Project Location: Halton	Number of Units/Unit #'s	402 Units
Date: 27-Sep-24		

1.0 FUS Formula

$RFF = 220C\sqrt{A}$ where: RFF = required fire flow in litres per minute;
C = the Coefficient related to the type of construction; and
A = the Total Effective Floor Area (m²) excluding basements at least 50% below grade)^a

NBC Occupancy	Group C
Type of Construction ^b	Fire-Resistive Construction Type I
Protection (for C below 1.0)	Unprotected Openings
Footprint area	5672.0 sq. metres
Storeys	22
C =	0.6
A =	22833.0 Total Effective Area ^a
F =	20000 L/min (rounded)

2.0 Occupancy Adjustment

Type of Occupancy ^c	Limited Combustible
Hazard Allowance	-0.15
	-3000 L/min
Adjusted Fire Flow	17000 L/min

3.0 Sprinkler Adjustment

	Credit	Total
NFPA 13 sprinkler standard	YES	30%
Standard Water Supply	YES	10%
Fully Supervised system	YES	10%
		50%

Sprinkler Credit 8500 L/min

4.0 Exposure Adjustment

Construction Type of the Exposed Building Face: Type I-II (unprotected)

Side	Distance to Building (m)	Length (ft) by height in storeys	Percent	Total*	
North Side					
	over 30	over 100	0%	19%	
South Side					
	over 30	over 100	0%		
East Side					
	3.1 to 10	over 100	11%		
West Side					
	10.1 to 20	over 100	8%		

*max 75%

Exposures Surcharge 3230 L/min

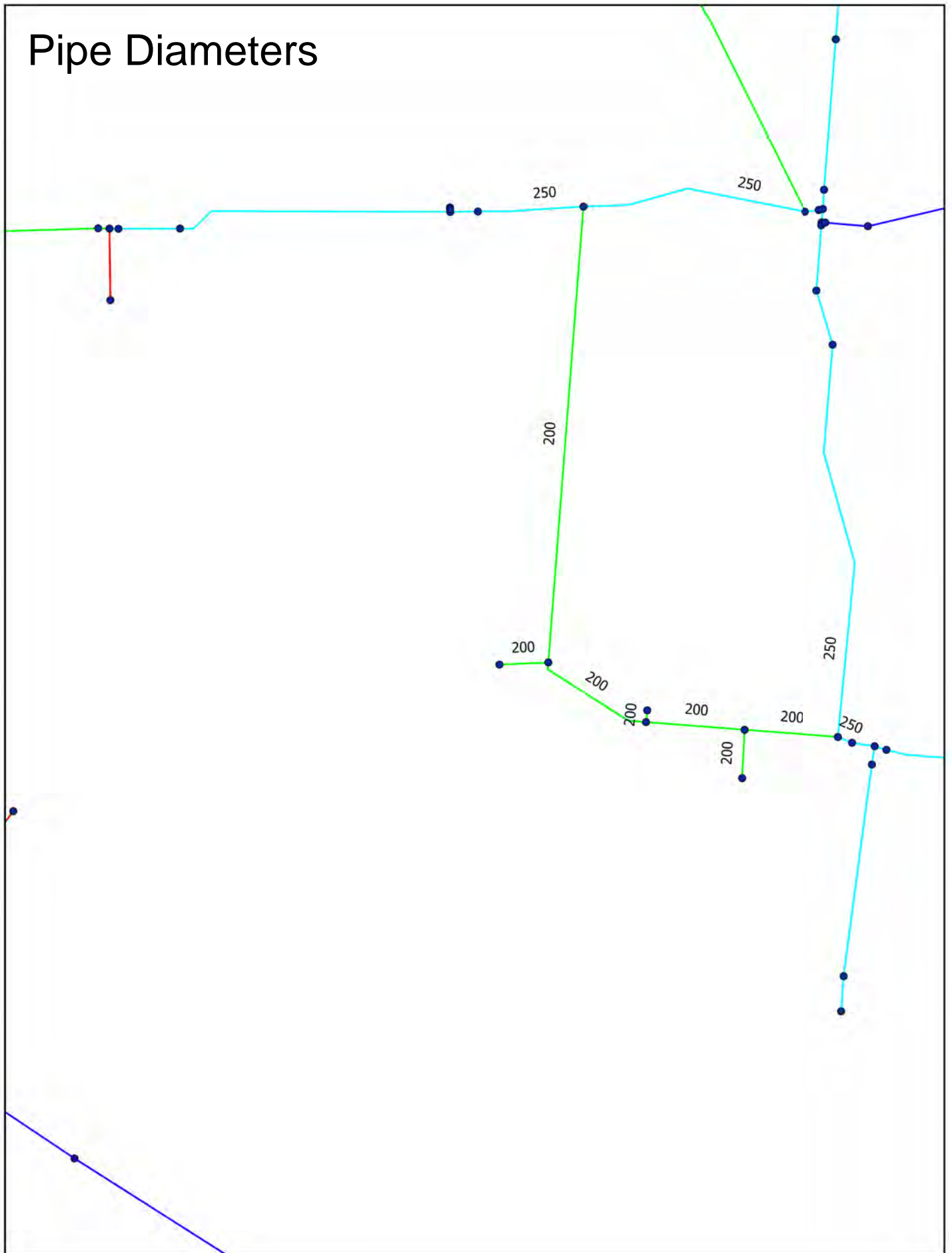
Total Required Fire Flow 12000 L/min (rounded)	200 L/sec
---	------------------

a) For buildings with a construction coefficient from 1.0 to 1.5, consider 100% of all floor areas. For buildings with a construction coefficient below 1.0 (vertical openings are inadequately protected), consider the two largest adjoining floors plus 50% of each of any floors immediately above them up to a maximum of eight. If the vertical openings and exterior vertical communications are properly protected, consider only the area of the largest floor plus 25% of each of the two immediately adjoining floors.
b) Wood Frame=1.5, Mass Timber= 0.8 to 1.5, Ordinary=1.0, Noncombustible=0.8, Fire-Resistive=0.6
c) Noncombustible=-25%, Limited Combustible=-15%, Combustible=0%, Free Burning=+15%, Rapid Burning=+25%

Appendix B

Model Results

Pipe Diameters



2026 Fire Flows



2031 Fire Flows



APPENDIX E

Watermark Hydrant Flow Test Report



Hydrant Flow Test Report

Residual Hydrant Number _____

Date: 16-Sep-24 Time: 10:10 AM Operator: Colin Powell
 Witness: Halton Region

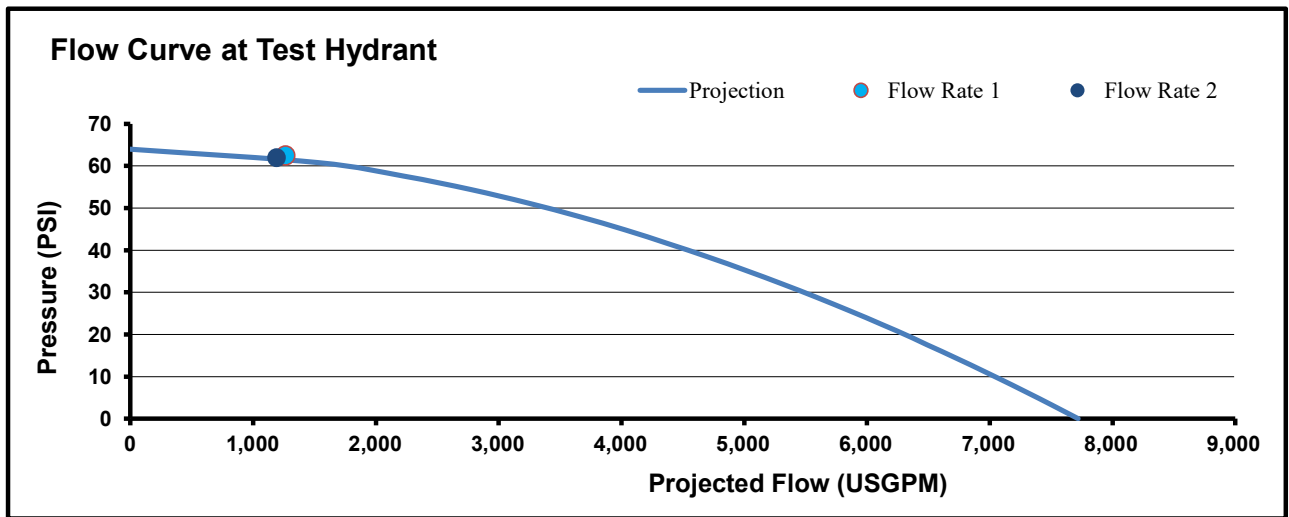
Residual Test Hydrant:	5 Daniela Court	
Hydrant Number:	NFPA Colour Code:	CLASS AA - BLUE
Owner:		

STATIC PRESSURE:	64 psi	441 kPa	Pressure Drop
RESIDUAL PRESSURE 1:	62.5 psi	kPa	
RESIDUAL PRESSURE 2:	62 psi	427 kPa	3.1%

Flow Hydrants:		Hydrant Number
A	7 River Drive	
B		
C		

Hydrant No.	Flow Device	Outlet Dia. (in.)	Flow Rate 1		Flow Rate 2	
			Reading (psi)	(USGPM)	Reading (psi)	(USGPM)
A	Pitot	2.5	20	698	14	584
A	Pitot	2.5	13	562	15	604
A	HoseMonster	4"			-	
B	Pitot	2.5		0		0
B	Pitot	2.5		0		0
Total Flow (USGPM)			1260		1188	
Total Flow (L/second)			80		75	
Available Flow At Test Hydrant at 20 ps			7,812 USGPM		6,305 USGPM	
			493 L/second		398 L/second	

Average Projection at 20 PSI **7,059 USGPM**



Comments/Discrepancies/Diagram:



Hydrant Flow Test Report

Residual Hydrant Number _____

Date: 16-Sep-24 Time: 10:40 AM

Operator: Colin Powell

Witness: Halton Region

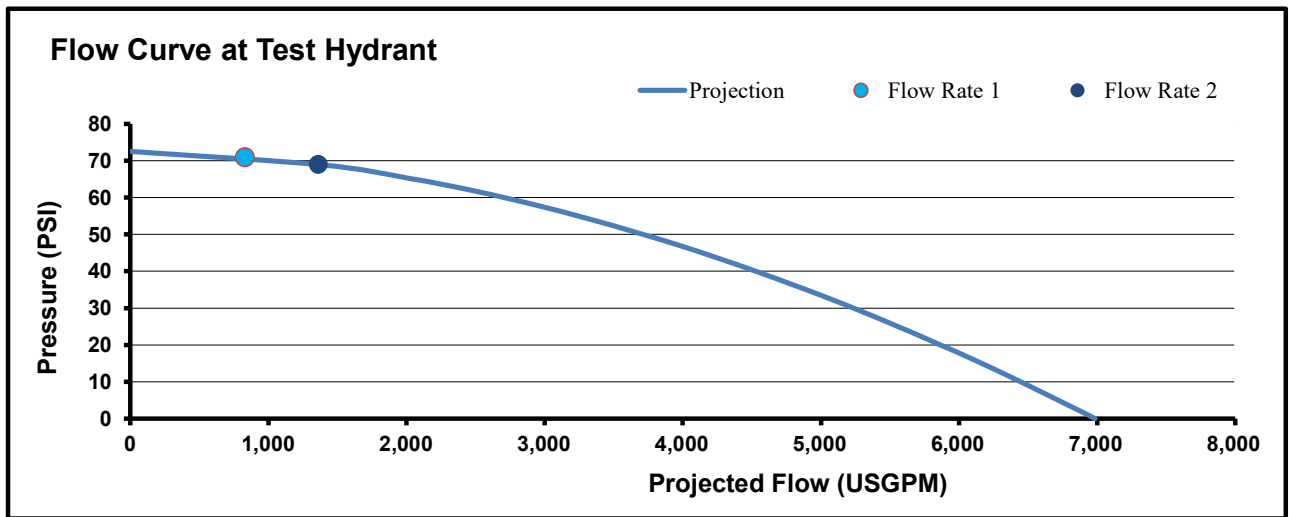
Residual Test Hydrant:	Mountainview at River Drive		
Hydrant Number:		NFPA Colour Code:	CLASS AA - BLUE
Owner:			

STATIC PRESSURE:	72.5 psi	500 kPa	Pressure Drop
RESIDUAL PRESSURE 1:	71 psi	490 kPa	
RESIDUAL PRESSURE 2:	69 psi	476 kPa	4.8%

Flow Hydrants:		Hydrant Number
A		
B		
C		

Hydrant No.	Flow Device	Outlet Dia. (in.)	Flow Rate 1		Flow Rate 2	
			Reading (psi)	(USGPM)	Reading (psi)	(USGPM)
A	Pitot	2.5	28	825	18	662
A	Pitot	2.5		0	20	698
A	HoseMonster	4"			-	
B	Pitot	2.5		0		0
B	Pitot	2.5		0		0
Total Flow (USGPM)			825		1360	
Total Flow (L/second)			52		86	
Available Flow At Test Hydrant at 20 ps			5,630 USGPM		5,868 USGPM	
			355 L/second		370 L/second	

Average Projection at 20 PSI **5,749 USGPM**



Comments/Discrepancies/Diagram: